Preliminary Geotechnical and GeoHazards Report

Big Rock Solar SiteSEC and NEC of Liebert Road and Wixom Road

El Centro, California

Prepared for:

92JT 8ME, LLC 211 Sutter Street, 6th Floor San Francisco, CA 94108





Prepared by:

Landmark Consultants, Inc. 780 N. 4th Street El Centro, CA 92243 (760) 337-1100

September 2017



September 1, 2017

780 N. 4th Street El Centro, CA 92243 (760) 370-3000 (760) 337-8900 fax

77-948 Wildc at Drive Palm Desert, CA 92211 (760) 360-0665 (760) 360-0521 fax

Mr. Daniel Kolta 92JT 8ME, LLC 211 Sutter Street, 6th Floor San Francisco, CA 94108

Preliminary Geological and Geotechnical Hazard Evaluation Big Rock Solar Site SEC and NEC Liebert Road and Wixom Road El Centro, California LCI Project No. LE17083

Dear Mr. Kolta:

This preliminary geotechnical report and geologic hazards study is provided for preliminary site evaluation and permitting of the photo-voltaic solar farm at the approximately 342-acre project area located at the southeast and northeast corners of Liebert Road and Wixom Road approximately 8 miles west of El Centro, California.

Scope of Work

The scope of work consisted of a geologic and geotechnical hazards evaluation of the project site which addresses the following items:

- 1. General site geology.
- 2. Site location in relation to mapped earthquake faults and seismic zones.
- 3. Intensity of ground shaking at the site.
- 4. Potential for liquefaction, ground failure, and landslides at the site. No drilling was conducted to determine potential for liquefaction settlement or soil analysis.
- 5. Soil corrosivity.
- 6. Plant growth suitability of the site soils.
- 7. Preliminary pavement structural sections.
- 8. Potential for flooding at the site from manmade facilities (dams, canals, etc.) and from natural storms.
- 9. Other potential geologic and geotechnical hazards.

Site Description

The project site is located at the southeast and northeast corners of Liebert Road and Wixom Road approximately 8 miles west of El Centro, California. The project site consists of 342-acres comprised of seven (7) agricultural fields currently in crop production. Dirt field roads bisect the site. The Westside Main Canal (earthen) forms the southern property boundary. Liebert Road (unpaved) forms the western property boundary. Wixom Road (paved) forms the northern property boundary except for the 60-acre parcel located at the northeast corner of Wixom Road and Liebert Road. The Fig Drain, an approximately 10 foot deep irrigation drainage ditch, forms the eastern property boundary. There is an approximately 3 to 8 foot elevation difference between the northern and southwestern fields (lower) and the fields to the east (higher).

Agricultural fields are located around the perimeter of the project site. Dirt field roads are located along the margins and also cross the parcels. A rural residence and farm shop are located near the center of the northeast portion of the site. The adjacent properties are approximately the same elevation as the project sites.

Site Geological Conditions

Site Geology: The project site is located in the Imperial Valley portion of the Salton Trough physiographic province. The Salton Trough is a topographic and geologic structural depression resulting from large scale regional faulting. The trough is bounded on the northeast by the San Andreas Fault and Chocolate Mountains and the southwest by the Peninsular Range and faults of the San Jacinto Fault Zone. The Salton Trough represents the northward extension of the Gulf of California, containing both marine and non-marine sediments since the Miocene Epoch. Tectonic activity that formed the trough continues at a high rate as evidenced by deformed young sedimentary deposits and high levels of seismicity. Figure 1 shows the location of the site in relation to regional faults and physiographic features.

The Imperial Valley is directly underlain by lacustrine deposits, which consist of interbedded lenticular and tabular silt, sand, and clay. Based on Unified Soil Classification System, the permeability of these soils is expected to be low to moderate. The Late Pleistocene to Holocene lake deposits are probably less than 100 feet thick and derived from periodic flooding of the Colorado River which intermittently formed a fresh water lake (Lake Cahuilla).

Older deposits consist of Miocene to Pleistocene non-marine and marine sediments deposited during intrusions of the Gulf of California. Basement rock consisting of Mesozoic granite and Paleozoic metamorphic rocks are estimated to exist at depths between 15,000 - 20,000 feet.

Groundwater: The groundwater in the site area is brackish and typically encountered at a depth of between 5 to 10 feet below ground surface in the vicinity of the project site. There is uncertainty in the accuracy of short-term water level measurements, particularly in fine-grained soil. Groundwater levels may fluctuate with water elevation in the Westside Main Canal, precipitation, irrigation of adjacent properties, drainage, and site grading. The groundwater level noted should not be interpreted to represent an accurate or permanent condition.

Onsite Wastewater Disposal: The near surface soils at the project site generally consist of silts and silty sands having a moderate infiltration rate. The near surface soils are considered good in supporting onsite septic systems and leach fields for wastewater disposal. Site specific studies will be required to determine that County Environmental Health standards are met in regard to soil percolation rates and separation of leach fields from groundwater.

Geological Hazards

Landsliding: No ancient landslides are shown on geologic maps of the region and no indications of landslides were observed during our site investigation. The hazard of landsliding is unlikely due to the relatively planar topography of the project site.

Volcanic hazards: The site is not located proximal to any known volcanically active area and the risk of volcanic hazards is considered very low.

Tsunamis, seiches, and flooding: The site does not lie near any large bodies of water, so the threat of tsunami, seiches, or other seismically-induced flooding is considered unlikely. The project site is located in FEMA Flood Zone X, an area determined to be outside the 0.2% annual chance floodplain (FIRM Panel 06025C2050C). The Westside Main Canal forms a portion of the southern property boundary. The earthen canal supplies irrigation water to the western portion of the Imperial Valley. The canal embankments are approximately 3 to 4 feet above the elevation of the west side of the project site and the water level is slightly above the elevation of the project site.

Expansive soil: In general, much of the near surface soils within the project site consist of silts and silty sands having a low expansion potential. Clay soils may also be encountered within the near surface soils of the project site. A site specific geotechnical investigation will be required at this site to determine the extent and effect of expansive soils.

Corrosive Soils: The lacustrine site soils (lake bed deposits) are known to be corrosive. Typical remediation for the corrosive soil conditions consists of using concrete mixed with higher cement contents (6 sacks Type V Portland Cement) and low water-cement ratios (0.45 w/c ratio). Additionally, steel post corrosion protection is required, consisting of zinc coatings (galvanizing) or increased structural sections to compensate for metal loss due to corrosion.

Liquefaction/Seismic Settlements: Liquefaction is a potential design consideration because of possible saturated sandy substrata underlying the site. Liquefaction occurs when granular soil below the water table is subjected to vibratory motions, such as produced by earthquakes. With strong ground shaking, an increase in pore water pressure develops as the soil tends to reduce in volume. If the increase in pore water pressure is sufficient to reduce the vertical effective stress (suspending the soil particles in water), the soil strength decreases and the soil behaves as a liquid (similar to quicksand). Liquefaction can produce excessive settlement, ground rupture, lateral spreading, or failure of shallow bearing foundations.

Four conditions are generally required for liquefaction to occur:

- (1) the soil must be saturated (relatively shallow groundwater);
- (2) the soil must be loosely packed (low to medium relative density);
- (3) the soil must be relatively cohesionless (not clayey); and
- (4) groundshaking of sufficient intensity must occur to function as a trigger mechanism.

All of these conditions may exist to some degree at this site. Liquefaction settlement and ground fissures were noted along the Westside Main Canal in the area of the project site after the April 4, 2010 magnitude 7.2M_w El Mayor-Cucapah Earthquake. McCrink and others (2011) reported several liquefaction related failures to the embankment of the Westside Main Canal along the southern margin of the project site. Ground fissures and sand boils were noted along the embankments of the Westside Main Canal.

Seismic Hazards

The project site is located in the seismically active Imperial Valley of southern California with numerous mapped faults of the San Andreas Fault System traversing the region. The San Andreas Fault System is comprised of the San Andreas, San Jacinto, and Elsinore Fault Zones in southern California. The Imperial fault represents a transition from the more continuous San Andreas fault to a more nearly echelon pattern characteristic of the faults under the Gulf of California (USGS, 1990). We have performed a computer-aided search of known faults or seismic zones that lie within a 62 mile (100 kilometer) radius of the project site (Table 1).

A fault map illustrating known active faults relative to the site is presented on Figure 1, Regional Fault Map. A legend for the regional fault map is presented on Figure 2. The criterion for fault classification adopted by the California Geological Survey defines Earthquake Fault Zones along active or potentially active faults. An active fault is one that has ruptured during Holocene time (roughly within the last 11,000 years). A fault that has ruptured during the last 1.8 million years (Quaternary time), but has not been proven by direct evidence to have not moved within Holocene time is considered to be potentially active. A fault that has not moved during both Pleistocene and Holocene time (that is no movement within the last 1.8 million years) is considered to be inactive.

Review of the current Alquist-Priolo Earthquake Fault Zone maps (CGS, 2000a) indicates that the nearest mapped Earthquake Fault Zone is an unnamed fault located approximately 2.1 miles west of the substation project site. Geologic mapping by the USGS (Rymer and others, 2011) of the Imperial Valley after the April 4, 2010 magnitude 7.2M_w El Mayor-Cucapah Earthquake indicates movement along several known and unknown faults west of the project site. Surface rupture on these faults is possible from future seismic events in the area.

The nearest mapped major Earthquake Fault Zone is the Laguna Salada fault located approximately 9.4 miles southwest of the site and the Superstition Hills fault located approximately 8.5 miles northeast of the substation site.

Groundshaking. The primary seismic hazard at the project site is the potential for strong groundshaking during earthquakes along the Brawley Seismic Zone, Imperial, Brawley, East Highline, and Elmore Ranch Faults (Figure 2).

<u>Site Acceleration</u>: The project site is considered likely to be subjected to moderate to strong ground motion from earthquakes in the region. Ground motions are dependent primarily on the earthquake magnitude and distance to the seismogenic (rupture) zone. Accelerations also are dependent upon attenuation by rock and soil deposits, direction of rupture and type of fault; therefore, ground motions may vary considerably in the same general area.

<u>CBC General Ground Motion Parameters:</u> The 2016 CBC general ground motion parameters are based on the Risk-Targeted Maximum Considered Earthquake (MCE_R). The U.S. Geological Survey "U.S. Seismic Design Maps Web Application" (USGS, 2017) was used to obtain the site coefficients and adjusted maximum considered earthquake spectral response acceleration parameters. The site soils have been classified as Site Class D (stiff soil profile).

Design spectral response acceleration parameters are defined as the earthquake ground motions that are two-thirds (2/3) of the corresponding MCE_R ground motions. Design earthquake ground motion parameters are provided in Table 2. A Risk Category II was determined using Table 1604.5 and the Seismic Design Category is D since S_1 is less than 0.75.

The Maximum Considered Earthquake Geometric Mean (MCE_G) peak ground acceleration (PGA_M) value was determined from the "U.S. Seismic Design Maps Web Application" (USGS, 2017) for liquefaction and seismic settlement analysis in accordance with 2016 CBC Section 1803.5.12 and CGS Note 48 (PGA_M = $F_{PGA}*PGA$). A PGA_M value of 0.50g has been determined for this project site.

Surface Rupture: The project site does not lie within a State of California, Alquist-Priolo Earthquake Fault Zone. Surface fault rupture at the project site is considered to be low.

Other Hazards

Hazardous Materials: The site is not located in proximity to any known hazardous materials (methane gas, tar seeps, hydrogen sulfide gas), and the risk of hazardous materials is considered very low.

Radon 222 Gas: Radon gas is not believed to be a potential hazard at the site. A report titled "California Statewide Radon Survey-Screening Results", dated November 1990 and published by the California State Department of Health Services, notes that Southern California showed a low risk of elevated radon levels, based on 2-day tests conducted from January through April 1990. Some of the reported testing was performed in Imperial County; however, no data was observed as being at or near the project site.

Naturally occurring asbestos: The site is not located in proximity to any known naturally occurring asbestos, and the risk of naturally occurring asbestos is considered very low.

Hydrocollapse: The site is dominantly underlain by clays that are not expected to collapse with the addition of water to the site. The risk of hydrocollapse is considered very low.

Regional Subsidence: Regional subsidence due to geothermal resource activities has not been documented in the area west of the New River; therefore, the risk of regional subsidence is considered low.

Conclusion

This preliminary report was prepared according to the generally accepted *geotechnical* engineering standards of practice that existed in Imperial County at the time the report was prepared. No express or implied warranties are made in connection with our services.

Our research did not reveal conditions that would preclude implementation of the proposed project provided site specific geotechnical investigations are conducted prior to site development to provide geotechnical criteria for the design and construction of this project.

We appreciate the opportunity to provide our findings and professional opinions regarding geologic and geotechnical hazards at the site. If you have any questions or comments regarding our findings, please call our office at (760) 370-3000.

CERTIFIED ENGINEERING GEOLOGIST

No. 31921 EXPIRES 12-31-18

Sincerely Yours;

Landmark Consultants, Inc.

Steven K Williams, PG, CEG

Senior Engineering Geologist

Jeffrey O. Lyon, PE

President

Table 1
Summary of Characteristics of Closest Known Active Faults

Fault Name	Approximate Distance (miles)	Approximate Distance (km)	Maximum Moment Magnitude (Mw)	Fault Length (km)	Slip Rate (mm/yr)
Unnamed 1*	2.1	3.3			
Yuha*	3.8	6.1			
Unnamed 2*	4.1	6.5			
Shell Beds	7.3	11.6			
Yuha Well *	7.8	12.5			
Superstition Hills	8.5	13.7	6.6	23 ± 2	4 ± 2
Laguna Salada	9.4	15.0	7	67 ± 7	3.5 ± 1.5
Superstition Mountain	10.4	16.6	6.6	24 ± 2	5 ± 3
Vista de Anza*	10.7	17.1			
Borrego (Mexico)*	12.7	20.4			
Imperial	13.5	21.7	7	62 ± 6	20 ± 5
Painted Gorge Wash*	14.3	22.9			
Brawley *	15.1	24.1			
Ocotillo*	15.4	24.6			
Rico *	18.3	29.3			
Elsinore - Coyote Mountain	19.1	30.6	6.8	39 ± 4	4 ± 2
Pescadores (Mexico)*	20.5	32.8			
Cerro Prieto *	21.8	34.9			
Elmore Ranch	22.0	35.2	6.6	29 ± 3	1 ± 0.5
Cucapah (Mexico)*	22.8	36.5			
San Jacinto - Borrego	25.5	40.7	6.6	29 ± 3	4 ± 2
San Andreas - Coachella	42.5	68.0	7.2	96 ± 10	25 ± 5

^{*} Note: Faults not included in CGS database.

Table 2 2016 California Building Code (CBC) and ASCE 7-10 Seismic Parameters

CBC Reference

Soil Site Class:

D

Table 20.3-1

Latitude:

32.7360 N

Longitude: -115.7075 W

Risk Category:

Seismic Design Category:

D

Maximum Considered Earthquake (MCE) Ground Motion

Manual MCE Chart David Construct Description	6	1.500 ~	Elaura 1612 2 1(1)	
Mapped MCE _R Short Period Spectral Response	$\mathbf{S_s}$	1.500 g	Figure 1613.3.1(1)	
Mapped MCE _R 1 second Spectral Response	S_1	0.600 g	Figure 1613.3.1(2)	
Short Period (0.2 s) Site Coefficient	$\mathbf{F_a}$	1.00	Table 1613.3.3(1)	
Long Period (1.0 s) Site Coefficient	$\mathbf{F_v}$	1.50	Table 1613.3.3(2)	

MCE_R Spectral Response Acceleration Parameter (0.2 s) Equation 16-37 S_{MS} 1.500 g $= F_a * S_s$ MCE_R Spectral Response Acceleration Parameter (1.0 s) 0.900 g $= F_{v} * S_{1}$ Equation 16-38 S_{M1}

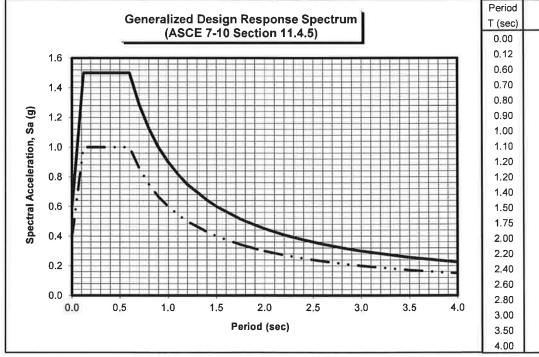
Design Earthquake Ground Motion

 $= 2/3*S_{MS}$ Design Spectral Response Acceleration Parameter (0.2 s) 1.000 g Equation 16-39 S_{DS} $= 2/3*S_{MI}$ Equation 16-40 Design Spectral Response Acceleration Parameter (1.0 s) 0.600 g S_{D1} ASCE Figure 22-12 8.00 sec T_{L}

 $0.12 \text{ sec} = 0.2 \text{ sec} / S_{D1} / S_{DS}$ T_{0}

 T_{S} $0.60 \text{ sec} = S_{D1}/S_{DS}$

0.50 g ASCE Equation 11.8-1 Peak Ground Acceleration **PGA_M**



Period	Sa	MCE _R Sa
T (sec)	(g)	(g)
0.00	0.40	0.60
0.12	1.00	1.50
0.60	1.00	1.50
0.70	0.86	1.29
0.80	0.75	1.13
0.90	0.67	1.00
1.00	0.60	0.90
1.10	0.55	0.82
1.20	0.50	0.75
1.20	0.50	0.75
1.40	0.43	0.64
1.50	0.40	0.60
1.75	0.34	0.51
2.00	0.30	0.45
2.20	0.27	0.41
2.40	0.25	0.38
2.60	0.23	0.35
2.80	0.21	0.32
3.00	0.20	0.30
3.50	0.17	0.26
4.00	0.15	0.23

Design Response Spectra MCE_R Response Spectra

Regional Fault Map

Figure 1



Map of Local Faults

Project No.: LE17083 Geo-Engineers and Geologists

Figure 2

EXPLANATION

Fault traces on land are indicated by solid lines where well located, by dashed lines where approximately located or inferred, and by dotted lines where concealed by younger rocks or by lakes or bays. Fault traces are queried where continuation or existence is uncertain. Concealed faults in the Great Valley are based on maps of selected subsurface horizons, so locations shown are approximate and may indicate structural trend only. All offshore faults based on seismic reflection profile records are shown as solid lines where well defined, dashed where Inferred, queried where uncertain.

FAULT CLASSIFICATION COLOR CODE (Indicating Recency of Movement)

Fault along which historic (last 200 years) displacement has occurred and is associated with one or more of the following:

(a) a recorded earthquake with surface rupture. (Also included are some well-defined surface breaks caused by ground shaking during earthquakes, e.g. extensive ground breakage, not on the White Wolf fault, caused by the Arvin-Tehachapi earthquake of 1952). The date of the associated earthquake is indicated. Where repeated surface ruptures on the same fault have occurred, only the date of the latest movement may be indicated, especially if earlier reports are not well documented as to location of ground breaks.

- (b) fault creep slippage slow ground displacement usually without accompanying earthquakes.
- (c) displaced survey lines.

1906 ► ■ 1906

1838 D ■ 1839

► 1951 ■

1992

CREEP

A triangle to the right or left of the date indicates termination point of observed surface displacement. Solid red triangle indicates known location of rupture termination point. Open black triangle indicates uncertain or estimated location of rupture termination point.

Date bracketed by triangles indicates local fault break.

No triangle by date indicates an intermediate point along fault break.

Fault that exhibits fault creep slippage. Hachures indicate linear extent of fault creep. Annotation (creep with leader) indicates representative locations where fault creep has been observed and recorded.

Square on fault indicates where fault creep slippage has occured that has been triggered by an earthquake on some other fault. Date of causative earthquake indicated. Squares to right and left of date indicate terminal points between which triggered creep slippage has occurred (creep either continuous or intermittent between these end points).

Holocene fault displacement (during past 11,700 years) without historic record. Geomorphic evidence for Holocene faulting includes sag ponds, scarps showing little erosion, or the following features in Holocene age deposits: offset stream courses, linear scarps, shutter ridges, and triangular faceted spurs. Recency of faulting offshore is based on the interpreted age of the youngest strata displaced by faulting.

_____?·

Late Quaternary fault displacement (during past 700,000 years). Geomorphic evidence similar to that described for Holocene faults except features are less distinct. Faulting may be younger, but lack of younger overlying deposits precludes more accurate age classification.

Quaternary fault (age undifferentiated). Most faults of this category show evidence of displacement sometime during the past 1.6 million years; possible exceptions are faults which displace rocks of undifferentiated Plio-Pleistocene age. Unnumbered Quaternary faults were based on Fault Map of California, 1975. See Bulletin 201, Appendix D for source data.

Pre-Quaternary fault (older that 1.6 million years) or fault without recognized Quaternary displacement. Some faults are shown in this category because the source of mapping used was of reconnaissnce nature, or was not done with the object of dating fault displacements. Faults in this category are not necessarily inactive.

LANDMARK
Geo-Engineers and Geologists
Project No.: LE17083

Fault Map Legend

Figure 3a

ADDITIONAL FAULT SYMBOLS

	bal and ball on downtinown side (relative or apparent).
=	Arrows along fault indicate relative or apparent direction of lateral movement.
	Arrow on fault indicates direction of dip.
	Low angle fault (barbs on upper plate). Fault surface generally dips less than 45° but locally may have been subsequently steepened. On offshore faults, barbs simply indicate a reverse fault regardless of steepness of dip.
	OTHER SYMBOLS
	Numbers refer to annotations listed in the appendices of the accompanying report. Annotations include fault name, age of fault displacement, and pertinent references including Earthquake Fault Zone maps where a fault has been zoned by the Alquist-Priolo Earthquake Fault Zoning Act. This Act requires the State Geologist to delineate zones to encompass faults with Holocene displacement.
	Structural discontinuity (offshore) separating differing Neogene structural domains. May indicate discontinuities between basement rocks.
MAMAMAMAMA	Brawley Seismic Zone, a linear zone of seismicity locally up to 10 km wide associated with the releasing step between the Imperial and San Andreas faults.

		Fault	Recency	DESCRIPTION						
ne ile	Present (Approx.)	Symbol	of Movement	ON LAND	OFFSHORE					
y Historic	200	~		Displacement during historic time (or Includes areas of known fault creep						
Naternar Holocene		-	- 3	Displacement during Holocetis time.	Fault offsets seafoot sediments or strata of Holocenic age					
	11,700 —		-2	Faults showing evidence of displacement during links Quaternary time.	Facilitatin strate of Lince Predictorin age					
Early Quatemary Pleistocen	700,000			Undwided Quoternary faults most faults in this category show evidence of draplacetimes during the fast 1,600,000 years; potistic exceptions are faults which displace rocks of undifferentiated Pilo-Pinstocens age.	Fault cuts strata of Quaternory age					
	1,600,000			Faults without recognized Quaternary displacement or showing evidence of no displacement during Quaternary time. Not necessarily inactive.	Faull cuts strata of Pliocene or older age,:					
	Late Quaternary Poence Historic Bistoric	Present (Approx.) June (Approx.) 11,700 11,700 700,000 700,000	Present (Approx.) Symbol Present (Approx.) Present (Approx.) 11,700 700,000 700,000 1,600,000	Pleistocene (Approx.) Symbol of Movement 11,700 11,700 700,000 1,600,000	Before Present (Approx.) Symbol of Movement ON LAND Displacement during historic time (Includes areas of known fault creep limb. 11,700 Faults showing evidence of displacement during limb most faults in this category show elvidence of displacement during limb which displace recks of undifferentiated Plan-Planstations are faults which displacement or showing evidence of no displacement during Quaternary lime. Not necessarily inactive.					

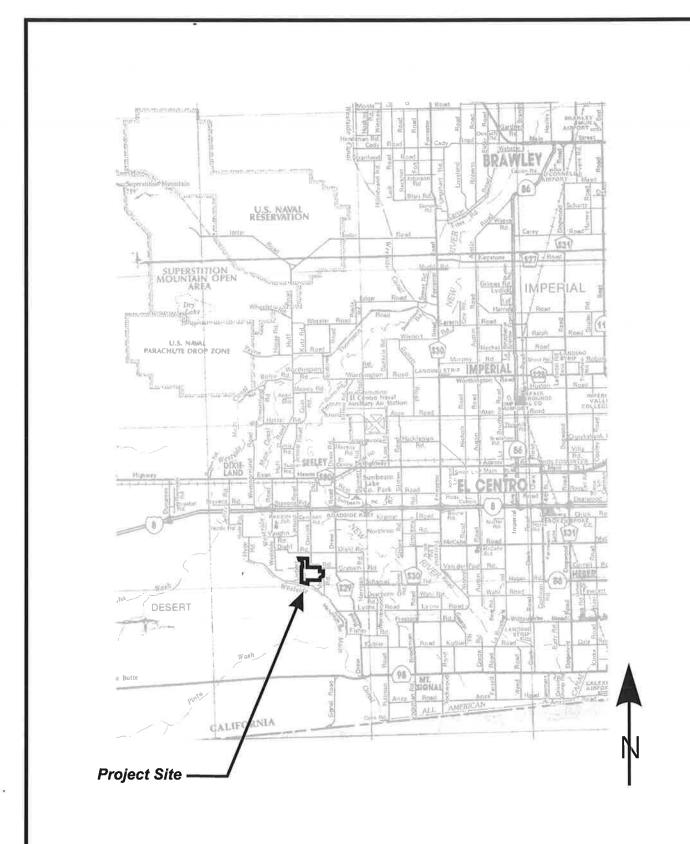
^{*} Quaternary now recognized as extending to 2.6 Ma (Walker and Geissman, 2009). Quaternary faults in this map were established using the previous 1.6 Ma criterion.



Fault Map Legend

Figure 3b

APPENDIX A



LANDMARK
Geo-Engineers and Geologists
Project No.: LE17083

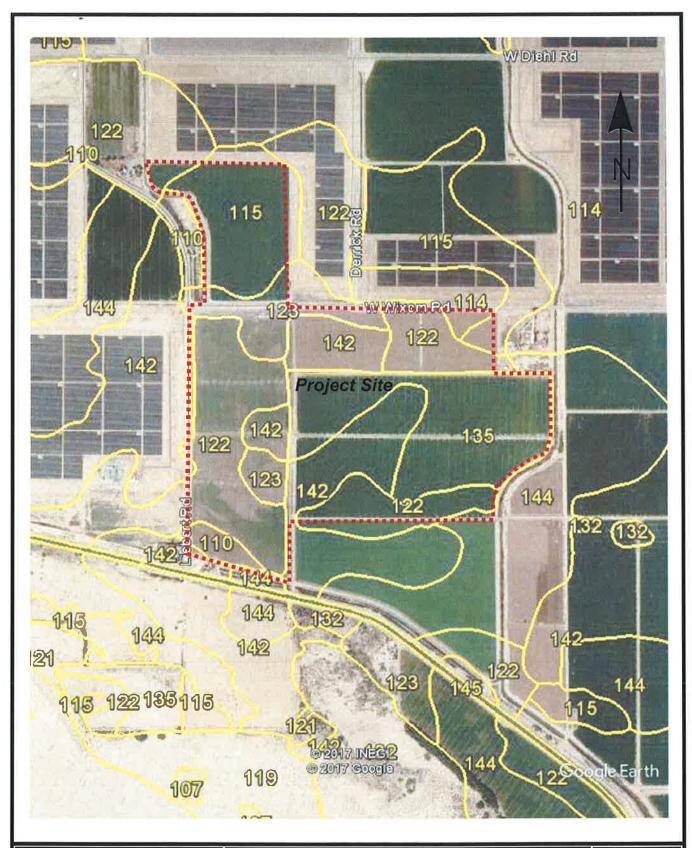
Vicinity Map



LANDMARK
Geo-Engineers and Geologists

Project No.: LE17083

Site Map

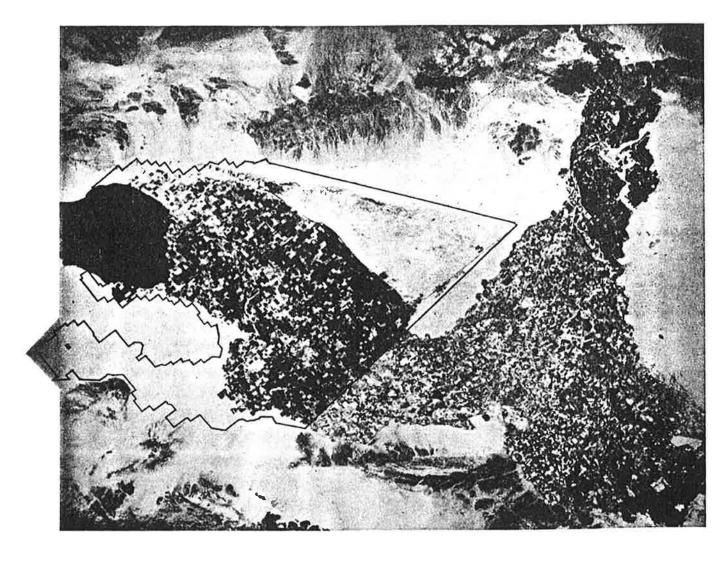


LANDMARK
Geo-Engineers and Geologists
Project No.: LE17083

Soil Survey Map

Soil Survey of

IMPERIAL COUNTY CALIFORNIA IMPERIAL VALLEY AREA



United States Department of Agriculture Soil Conservation Service
in cooperation with
University of California Agricultural Experiment Station
and
Imperial Irrigation District

TABLE 11.--ENGINEERING INDEX PROPERTIES

[The symbol > means more than. Absence of an entry indicates that data were not estimated]

Soil name and	Depth	USDA texture	Classif	1	Frag-	P	ercenta; sieve :	ge passi number-		Liquid	Plas-
map symbol			Unified		> 3 inches	4	10	40	200	limit	ticity index
	<u>In</u>				Pct		!			Pot	
100 Antho		Loamy fine sand Sandy loam, fine sandy loam.		A-2 A-2, A-4	0 0		100 75-95	75-85 50-60		==	N P N P
101*: Antho		li annu Gian annu	GW.		0	100	100	75 05	10.20		NP
Antho	8-60	Sandy loam, fine sand sandy loam.	SM	A-2, A-2, A-4			100 75 - 95				NP NP
Superstition		Fine sand Loamy fine sand, fine sand, sand.		A-2 A-2	0 0		95-100 95-100				N P N P
102*. Badland	1										
103 Carsitas		Gravelly sand Gravelly sand, gravelly coarse sand, sand.	SP, SP-SM				50 - 85 50-85		0-10 0-10	===	NP NP
104 * Fluvaquents	1 1 1						1				
105 Glenbar	13-60	Clay loam. silty clay loam.	CL CL	A-6 A-6	0	100 100		90-100 90-100		35-45 35-45	15-30 15-30
106 Glenbar	13-60	Clay loam Clay loam, silty clay loam.	CL CL	A-6, A-7 A-6, A-7		100 100		90-100 90-100		35-45 35-45	15-25 15-25
107* Glenbar	0-13	Loam	ML, CL-ML, CL	A – 4	0	100	100	100	70-80	20-30	NP-10
		Clay loam, silty clay loam.		A-6, A-7	0	100	100	95-100	75-95	35-45	15-30
108 Holtville	14-22 22 - 60	Loam	CL, CH	A = 4 A = 7 A = 4	0 0 0	100 100 100	1 100	85-100 95-100 95-100	85-95	40-65	NP-10 20-35 NP-10
	17-24 24-35	Clay, silty clay Silt loam, very fine sandy	CL, CH	A-7 A-7 A-4	0 0 0	100 100 100	100	95-100 95-100 95-100	85-95	40-65	20-35 20-35 NP-10
	35-60	loam. Loamy very fine sand, loamy fine sand.	SM, ML	A-2, A-4	0	100	100	75-100	20-55		ΝP
110 Holtville	17-24 24-35	Silty clay Clay, silty clay Silt loam, very fine sandy	CH, CL	A-7 A-7 A-4	0 0 0	100 100 100	100	95-100 95-100 95-100	85-95	40-65 40-65 25-35	20-35 20-35 NP-10
	35-60	loam. Loamy very fine sand, loamy fine sand.	SM, ML	A-2, A-4	0	100	100	75-100	20-55		ΝP

See footnote at end of table.

TABLE 11.--ENGINEERING INDEX PROPERTIES--Continued

Sad1 ma== -=d	Darth	USDA texture	Classifi		Frag-			centage passing			Plas-
Soil name and map symbol	Depth	USDA texture	Unified	AASHTO	> 3			1	200	Liquid limit	ticity index
	In				inches Pct	-4	10	40	200	Pot	index
111*: Holtville	0-10 10-22	Silty clay loam Clay, silty clay Silt loam, very fine sandy loam.	CL, CH	A-7 A-7 A-4	0 0 0	100 100 100	100	95-100 95-100 95-100	85-95	40-65 40-65 25-35	20-35 20-35 NP-10
Imperial	0-12 12-60	Silty clay loam Silty clay loam, silty clay, clay.	CL CH	A-7 A-7	0 0	100 100	100 100		85 - 95 85 - 95	40-50 50-70	10-20 25-45
112 Imperial	12-60	Silty clay Silty clay loam, silty clay, clay.		A-7 A-7	0	100 100	100 100		85-95 85-95	50 - 70 50 -7 0	25-45 25-45
113Imperial		Silty clay Silty clay, clay, silty clay loam.		A-7 A-7	0	100 100	100 100	100 100	85 - 95 85 - 95	50-70 50-70	25-45 25-45
114Imperial		Silty clay Silty clay loam, silty clay, clay,		A - 7 A - 7	0	100 100	100 100		85-95 85-95	50-70 50-70	25-45 25-45
115*: Imperial	0-12	Silty clay loam Silty clay loam, silty clay, clay.	CL CH	A-7 A-7	0	100 100	100 100		 85-95 85-95	40-50 50-70	10-20 25-45
Glenbar	0-13 13-60	Silty clay loam Clay loam, silty clay loam.	CL	A-6, A-7		100 100	100 100	90-100 90-100		35-45 35-45	15-25 15-25
116*: Imperial		Silty clay loam Silty clay loam, silty clay, clay.		A-7 A-7	0 0	100 100	100 100	100 100	85-95 85 - 95	40-50 50-70	10-20 25-45
Glenbar		Silty clay loam Clay loam, silty clay loam.		A-6, A-7 A-6	0	100 100	100 100	90-100 90-100			15-25 15-30
117, 118 Indio	112-72	Loam	ML	A – 4 A – 4	0	95-100 95-100	95-100 95-100	85-100 85-100	75-90 75 - 90	20-30 20-30	NP-5 NP-5
119*: Indio		Loam		A – 4 A – 4	0	95-100 95-100	95-100 95-100	85-100 85-100	75-90 75-90	20-30 20-30	NP-5 NP-5
Vint		Loamy fine sand Loamy sand, loamy fine sand.	SM SM	A-2 A-2	0	95-100 95-100	95-100 95-100	70-80 70-80	25-35 20 - 30	==	NP NP
120* Laveen		Loam Loam, very fine sandy loam.			0	100 95-100					NP-10 NP-10

See footnote at end of table.

TABLE 11. -- ENGINEERING INDEX PROPERTIES -- Continued

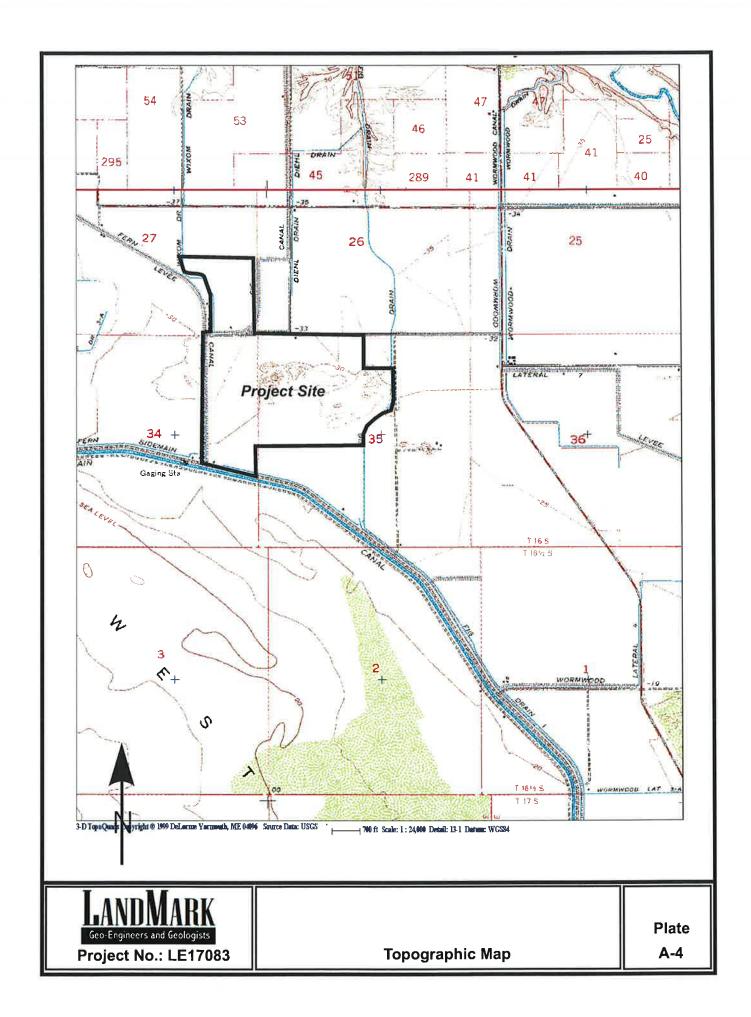
Soil name and	Depth	USDA texture	Classif		1		Frag- ments	Pe	rcentag sieve n			Liquid limit	Plas-
map symbol	200011	33211 33113113	Uni	fied	AASH'		> 3 inches	ц	10	40	200		ticity index
	In						Pot					Pet	
121 Meloland		Fine sand Stratified loamy fine sand to			A-2, A-4	A-3	0 0	95 – 100 100	90-100 100	75-100 90-100	5-30 50-65	25 - 35	NP NP-10
	26-71	silt loam.	CL,	СН	A-7		0	100	100	95-100	85-95	40-65	20-40
122	0-12	Very fine sandy	ML		A-4		0	95-100	95-100	95-100	55-85	25 - 35	NP-10
Meloland	Ì	loam. Stratified loamy fine sand to	ML		A-4		0	100	100	90-100	50-70	25-35	NP-10
	26-71	silt loam. Clay, silty clay, silty clay loam.	СH,	CL	A-7		0	100	100	95-100	85-95	40-65	20-40
123*: Meloland	12-26	Stratified loamy I fine sand to	ML ML		A-4 A-4		0	95-100 100		95-100 90-100		25-35 25-35	NP-10 NP-10
	26-38	clay, silty	сн,	CL	A-7		0	100	100	95-100	85-95	40-65	20-40
	38-60	clay loam. Stratified silt loam to loamy fine sand.	SM,	ML	A = 4		0	100	100	75-100	35-55	25-35	NP-10
Holtville	12-24 24-36	 Loam	¦CH,	CL	A-4 A-7 A-4		0 0 0	100 100 100	100 100 100	85-100 95-100 95-100		25-35 40-65 25-35	NP-10 20-35 NP-10
	36-60	loam. Loamy very fine sand, loamy fine sand.	SM.	ML	A-2,	A - 4	0	100	100	75-100	20-55		NP
124, 125 Niland	0-23	Gravelly sand Silty clay, clay, clay loam.	SM,	SP-SM CH	A-2, A-7	A-3	0		70 - 95 100	50-65 85-100		40-65	NP 20-40
126 Niland	0-23 23-60	Fine sand Silty clay	SM,	SP-SM CH	A-2,	A - 3	0 0		90-100			40 - 65	NP 20-40
127Niland	0-23 23-60	Loamy fine sand Silty clay	SM CL,	СН	A-2 A-7		0		90-100			40-65	NP 20-40
128*: Niland		Gravelly sand Silty clay, clay, clay loam.	ISM,	SP-SM CH	A-2,	A-3	0 0	90-100	70-95 100	 50-65 85-100	5-25 80-100	40-65	NP 20-40
Imperial		Silty clay Silty clay loam silty clay, clay.			A-7 A-7		0	100	100	100	85-95 85-95	50-70 50-70	25-45 25-45
129*: Pits													
130, 131 Rositas	0-27	Sand	SP-	-SM	A-3, A-1 A-2	9	0	100	80-100	40-70	5-15		N P
	27-60	Sand, fine sand loamy sand.	, SM	, SP-SI		ř	0	100	80-100	40-85	5-30		NP

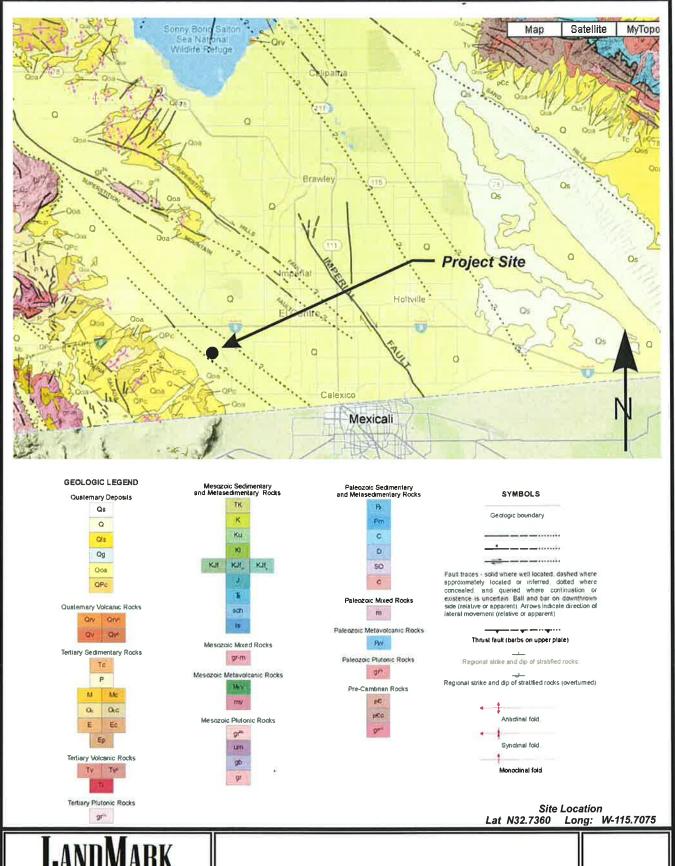
See footnote at end of table.

TABLE 11.--ENGINEERING INDEX PROPERTIES--Continued

Soil name and	Depth	USDA texture	1	ication	Frag-	l P	ercenta sieve	ge pass number-		Liquid	Plas-
map symbol			Unified		> 3 inches	4	10	40	200	limit	ticity index
120 100 400 400	In				Pet					Pet	
132, 133, 134, 135- Rositas	1	1	1	A-3, A-2	0	100	180 -1 00	50 - 80	10-25		NP
	9-60	Sand, fine sand, loamy sand.	ISM, SP-SM	A-3, A-2, A-1	0	100	80-100	40-85	5-30		ΝP
136 Rositas		Loamy fine sand Sand, fine sand, loamy sand.		A-1, A-2 A-3, A-2, A-1	0		80-100 80-100				NP NP
137Rositas	0-12 12-60	Silt loam Sand, fine sand, loamy sand.	ML ISM, SP-SM	A-4 A-3, A-2, A-1	0		100 80-100	 90-100 40-85 		20-30	NP-5 NP
138*: Rositas	0-4 4-60	Loamy fine sand Sand, fine sand, loamy sand.	SM SM, SP-SM	A-1, A-2 A-3, A-2, A-1	0	100 100	80-100 80-100	 40-85 40-85		===	N P N P
Superstition	0-6 6-60	Loamy fine sand Loamy fine sand, fine sand, sand.	SM SM	A-2 A-2	0		95-100 95-100				NP NP
139Superstition	0-6 6-60	Loamy fine sand Loamy fine sand, fine sand, sand.	SM SM	A-2 A-2	0 0		95-100 95-100				NP NP
140*: Torriorthents									4		
Rock outerop											
141*; Torriorthents											
Orthids											
142 Vint		Loamy very fine sand.	SM, ML	A-4	0	100	100	85-95	40-65	15-25	NP-5
			SM	A-2	0	95-100	95-100	70-80	20-30		NP
143 Vint	0-12	Fine sandy loam	ML, CL-ML, SM,	A-4	0	100	100	75-85	45 - 55	15-25	NP-5
	12-60	Loamy sand, loamy fine sand.	SM-SC	A-2	0	95-100	95-100	70-80	20-30		NP
144*: Vint	0-10	Very fine sandy	SM, ML	A-4	0	100	100	85 - 95	40-65	15-25	NP-5
	10-40	loam. Loamy fine sand Silty clay	SM	A-2 A-7		95-100	95 ~ 100	70-80	20-30	40-65	NP 20-35
Indio	0-12	Very fine sandy	ML	A-4	0	95-100	9 5- 100	85-100	75 - 90	20-30	NP-5
 	1	loam. Stratified loamy very fine sand	ML	A-4	0	95-100	95-100	85-100	75-90	20-30	NP-5
		to silt loam. Silty clay	CL, CH	A-7	0	100	100	95-100	85 - 95	40-65	20-35

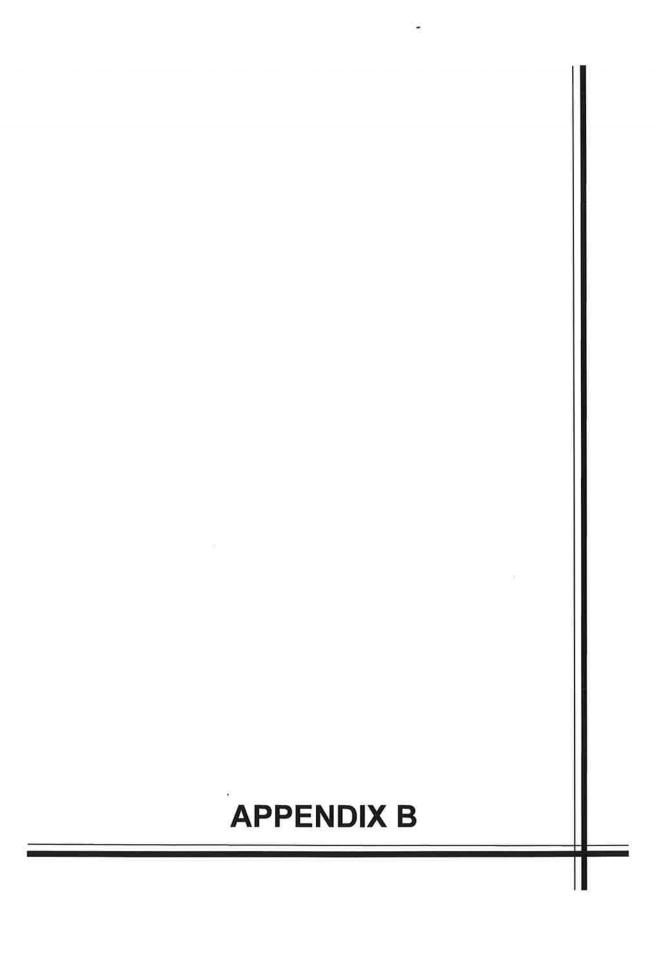
^{*} See description of the map unit for composition and behavior characteristics of the map unit.





LANDMARK
Geo-Engineers and Geologists
Project No.: LE17083

Regional Geologic Map



REFERENCES

- American Concrete Institute (ACI), 2013, ACI Manual of Concrete Practice 302.1R-04
- American Society of Civil Engineers (ASCE), 2010, Minimum Design Loads for Buildings and Other Structures: ASCE Standard 7-10.
- California Building Standards Commission, 2017, 2016 California Building Code. California Code of Regulations, Title 24, Part 2, Vol. 2 of 2.
- Caltrans, 2012, Highway Design Manual.
- California Division of Mines and Geology (CDMG), 1996, California Fault Parameters: available at http://www.consrv.ca.gov/dmg/shezp/fltindex.html.
- California Geological Survey (CGS), 2017, Fault Activity Map of California http://www.quake.ca.gov/gmaps/FAM/faultactivitymap.html#.
- California Geological Survey (CGS), 2017, Alquist-Priolo Earthquake Fault Zone Maps. http://maps.conservation.ca.gov/cgs/informationwarehouse/index.html?map=regulatorymaps
- Cetin, K. O., Seed, R. B., Der Kiureghian, A., Tokimatsu, K., Harder, L. F., Jr., Kayen, R. E., and Moss, R. E. S., 2004, Standard penetration test-based probabilistic and deterministic assessment of seismic soil liquefaction potential: ASCE JGGE, Vol., 130, No. 12, p. 1314-1340.
- Geologismiki, 2014, CLiq Computer Program, www.geologismiki.gr
- Ishihara, K. (1985), Stability of natural deposits during earthquakes, Proc. 11th Int. Conf. On Soil Mech. And Found. Engrg., Vol. 1, A. A. Balkema, Rotterdam, The Netherlands, 321-376.
- Jones, A. L., 2003, An Analytical Model and Application for Ground Surface Effects from Liquefaction, PhD. Dissertation, University of Washington, 362 p.
- Morton, P. K., 1977, Geology and mineral resources of Imperial County, California: California Division of Mines and Geology, County Report No. 7, 104 p.
- Post-Tensioning Institute (PTI), 2007a, Standard Requirements for Analysis of Shallow Concrete Foundations on Expansive Soils (3rd Edition).
- Post-Tensioning Institute (PTI), 2007b, Standard Requirements for Design of Shallow Post-Tensioned Concrete Foundations on Expansive Soils (2nd Edition).

- Robertson, P. K., 2014, Seismic liquefaction CPT-based methods: EERI 1st Workshop on Geotechnical Earthquake Engineering Liquefaction Evaluation, Mapping, Simulation and Mitigation. UC San Diego Campus, 10/12/2014.
- Robertson, P. K. and Wride, C. E., 1997, Cyclic Liquefaction and its Evaluation based on the SPT and CPT, Proceeding of the NCEER Workshop on Evaluation of Liquefaction Resistance of Soils, NCEER Technical Report 97-0022, p. 41-88.
- U.S. Geological Survey (USGS), 1990, The San Andreas Fault System, California, Professional Paper 1515.
- U.S. Geological Survey (USGS), 2017, US Seismic Design Maps Web Application, available at http://geohazards.usgs.gov/designmaps/us/application.php
- Wire Reinforcement Institute (WRI/CRSI), 2003, Design of Slab-on-Ground Foundations, Tech Facts TF 700-R-03, 23 p.
- Youd, T. L., 2005, Liquefaction-induced flow, lateral spread, and ground oscillation, GSA Abstracts with Programs, Vol. 37, No. 7, p. 252.
- Youd, T. L. and Garris, C. T., 1995, Liquefaction induced ground surface disruption: ASCE Geotechnical Journal, Vol. 121, No. 11.
- Zimmerman, R. P., 1981, Soil survey of Imperial County, California, Imperial Valley Area: U.S. Dept. of Agriculture Soil Conservation Service, 112 p.

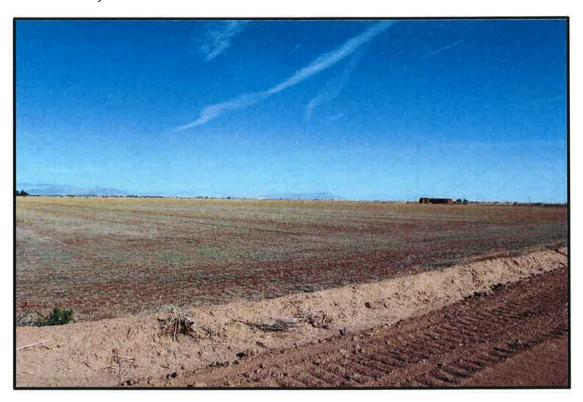
Preliminary Geotechnical and GeoHazards Report

Laurel Solar Site NEC and SEC Diehl Road and Derrick Road

El Centro, California

Prepared for:

90FI 8ME, LLC 211 Suter Street, 6th Floor San Francisco, CA 94108





Prepared by:

Landmark Consultants, Inc. 780 N. 4th Street El Centro, CA 92243 (760) 337-1100

May 2017



May 25, 2017

780 N. 4th Street El Centro, CA 92243 (760) 370-3000 (760) 337-8900 fax

77-948 Wildcat Drive Palm Desert, CA 92211 (760) 360-0665 (760) 360-0521 fax

Mr. Daniel Kolta 90FI 8ME, LLC 211 Sutter Street, 6th Floor San Francisco, CA 94108

Preliminary Geological and Geotechnical Hazard Evaluation Laurel Solar Site NEC and SEC of Derrick Road and Diehl Road El Centro, California LCI Project No. LE17082

Dear Mr. Kolta:

This preliminary geotechnical report and geologic hazards study is provided for preliminary site evaluation and permitting of the photo-voltaic solar farm at the approximately 171-acre project area located at the northeast and southeast corners of Derrick Road and Diehl Road approximately 7.5 miles west of El Centro, California.

Scope of Work

The scope of work consisted of a geologic and geotechnical hazards evaluation of the project site which addresses the following items:

- 1. General site geology.
- 2. Site location in relation to mapped earthquake faults and seismic zones.
- 3. Intensity of ground shaking at the site.
- 4. Potential for liquefaction, ground failure, and landslides at the site. No drilling will be conducted to determine potential for liquefaction settlement or soil analysis.
- 5. Soil corrosivity.
- 6. Plant growth suitability of the site soils.
- 7. Preliminary pavement structural sections.
- 8. Potential for flooding at the site from manmade facilities (dams, canals, etc.) and from natural storms.
- 9. Other potential geologic and geotechnical hazards.

Site Description

The project site is located at the northeast and southeast corners of Derrick Road and Diehl Road. The project site consists of 171-acres comprised of four agricultural fields currently in crop production. Dirt field roads bisect the southern portion of the site in an east-west direction and in a north-south direction. The Fig Drain, an earthen irrigation run-off water drainage ditch, forms the eastern boundary of the site. The Fig Drain is approximately 10 feet deep on the south and approximately 20 feet deep on the north. The Diehl Drain forms the western and northern boundaries of the site. Diehl Road, a paved rural road, crosses east-west through the center of the site. Derrick Road, a paved rural road, forms the western property boundary.

Existing solar facilities are located to the east, south and a portion of the western portion of the site. Agricultural fields are located adjacent to the north and northwestern portions of the site. The adjacent properties are approximately the same elevation as the project sites.

Site Geological Conditions

Site Geology: The project site is located in the Imperial Valley portion of the Salton Trough physiographic province. The Salton Trough is a topographic and geologic structural depression resulting from large scale regional faulting. The trough is bounded on the northeast by the San Andreas Fault and Chocolate Mountains and the southwest by the Peninsular Range and faults of the San Jacinto Fault Zone. The Salton Trough represents the northward extension of the Gulf of California, containing both marine and non-marine sediments since the Miocene Epoch. Tectonic activity that formed the trough continues at a high rate as evidenced by deformed young sedimentary deposits and high levels of seismicity. Figure 1 shows the location of the site in relation to regional faults and physiographic features.

The Imperial Valley is directly underlain by lacustrine deposits, which consist of interbedded lenticular and tabular silt, sand, and clay. Based on Unified Soil Classification System, the permeability of these soils is expected to be low to very low. The Late Pleistocene to Holocene lake deposits are probably less than 100 feet thick and derived from periodic flooding of the Colorado River which intermittently formed a fresh water lake (Lake Cahuilla).

Older deposits consist of Miocene to Pleistocene non-marine and marine sediments deposited during intrusions of the Gulf of California. Basement rock consisting of Mesozoic granite and Paleozoic metamorphic rocks are estimated to exist at depths between 15,000 - 20,000 feet.

Groundwater: The groundwater in the site area is brackish and typically encountered at a depth of between 5 to 10 feet below ground surface in the vicinity of the project site. There is uncertainty in the accuracy of short-term water level measurements, particularly in fine-grained soil. Groundwater levels may fluctuate with precipitation, irrigation of adjacent properties, drainage, and site grading. The groundwater level noted should not be interpreted to represent an accurate or permanent condition.

Onsite Wastewater Disposal: The near surface soils at the project site generally consist of silty clays and clays having a very low to low infiltration rate. The near surface soils are considered poor in supporting onsite septic systems and leach fields for wastewater disposal. Site specific studies will be required to determine that County Environmental Health standards are met in regard to soil percolation rates and separation of leach fields from groundwater.

Geological Hazards

Landsliding: No ancient landslides are shown on geologic maps of the region and no indications of landslides were observed during our site investigation. The hazard of landsliding is unlikely due to the relatively planar topography of the project site. There is a moderate potential for small slides to occur along the margins of the project site adjacent to the 10 to 20 feet deep Fig and Diehl Drain ditches.

Volcanic hazards: The site is not located proximal to any known volcanically active area and the risk of volcanic hazards is considered very low.

Tsunamis, seiches, and flooding: The site does not lie near any large bodies of water, so the threat of tsunami, seiches, or other seismically-induced flooding is considered unlikely. The project site is located in FEMA Flood Zone X, an area determined to be outside the 0.2% annual chance floodplain (FIRM Panel 06025C2050C).

Expansive soil: In general, much of the near surface soils within the project site consist of silty clays and clays having a high to very high expansion potential. The clay is expansive when wetted and can shrink with moisture loss (drying). Development of building foundations, concrete flatwork, and asphaltic concrete pavements should include provisions for mitigating potential swelling forces and reduction in soil strength, which can occur from saturation of the soil.

Corrosive Soils: The lacustrine site soils (lake bed deposits) are known to be corrosive. Typical remediation for the corrosive soil conditions consists of using concrete mixed with higher cement contents (6 sacks Type V Portland Cement) and low water-cement ratios (0.45 w/c ratio). Additionally, steel post corrosion protection is required, consisting of zinc coatings (galvanizing) or increased structural sections to compensate for metal loss due to corrosion.

Liquefaction/Seismic Settlements: Liquefaction is a potential design consideration because of possible saturated sandy substrata underlying the site. Liquefaction occurs when granular soil below the water table is subjected to vibratory motions, such as produced by earthquakes. With strong ground shaking, an increase in pore water pressure develops as the soil tends to reduce in volume. If the increase in pore water pressure is sufficient to reduce the vertical effective stress (suspending the soil particles in water), the soil strength decreases and the soil behaves as a liquid (similar to quicksand). Liquefaction can produce excessive settlement, ground rupture, lateral spreading, or failure of shallow bearing foundations.

Four conditions are generally required for liquefaction to occur:

- (1) the soil must be saturated (relatively shallow groundwater);
- (2) the soil must be loosely packed (low to medium relative density);
- (3) the soil must be relatively cohesionless (not clayey); and
- (4) groundshaking of sufficient intensity must occur to function as a trigger mechanism.

All of these conditions may exist to some degree at this site. Liquefaction settlement and ground fissures have been noted in the incised flood channel areas after strong seismic events. A site specific geotechnical investigation which includes liquefaction evaluation will be required at this site.

Seismic Hazards

The project site is located in the seismically active Imperial Valley of southern California with numerous mapped faults of the San Andreas Fault System traversing the region. The San Andreas Fault System is comprised of the San Andreas, San Jacinto, and Elsinore Fault Zones in southern California. The Imperial fault represents a transition from the more continuous San Andreas fault to a more nearly echelon pattern characteristic of the faults under the Gulf of California (USGS, 1990). We have performed a computer-aided search of known faults or seismic zones that lie within a 62 mile (100 kilometer) radius of the project site (Table 1).

A fault map illustrating known active faults relative to the site is presented on Figure 1, Regional Fault Map. A legend for the regional fault map is presented on Figure 2. The criterion for fault classification adopted by the California Geological Survey defines Earthquake Fault Zones along active or potentially active faults. An active fault is one that has ruptured during Holocene time (roughly within the last 11,000 years). A fault that has ruptured during the last 1.8 million years (Quaternary time), but has not been proven by direct evidence to have not moved within Holocene time is considered to be potentially active. A fault that has not moved during both Pleistocene and Holocene time (that is no movement within the last 1.8 million years) is considered to be inactive.

Review of the current Alquist-Priolo Earthquake Fault Zone maps (CGS, 2000a) indicates that the nearest mapped Earthquake Fault Zone is an unnamed fault located approximately 2.5 miles west of the substation project site. Geologic mapping by the USGS (Rymer and others, 2011) of the Imperial Valley after the April 4, 2010 magnitude 7.2M_w El Mayor-Cucapah Earthquake indicates movement along several known and unknown faults west of the project site. Surface rupture on these faults is possible from future seismic events in the area.

The nearest mapped Earthquake Fault Zone is the Laguna Salada fault located approximately 9.9 miles southwest of the site and the Superstition Hills fault located approximately 7.8 miles northeast of the substation site.

Groundshaking. The primary seismic hazard at the project site is the potential for strong groundshaking during earthquakes along the Laguna Salada, Superstition Hills, and Imperial faults (Figure 2).

<u>Site Acceleration</u>: The project site is considered likely to be subjected to moderate to strong ground motion from earthquakes in the region. Ground motions are dependent primarily on the earthquake magnitude and distance to the seismogenic (rupture) zone. Accelerations also are dependent upon attenuation by rock and soil deposits, direction of rupture and type of fault; therefore, ground motions may vary considerably in the same general area.

CBC General Ground Motion Parameters: The 2016 CBC general ground motion parameters are based on the Risk-Targeted Maximum Considered Earthquake (MCE_R). The U.S. Geological Survey "U.S. Seismic Design Maps Web Application" (USGS, 2017) was used to obtain the site coefficients and adjusted maximum considered earthquake spectral response acceleration parameters. The site soils have been classified as Site Class D (stiff soil profile).

Design spectral response acceleration parameters are defined as the earthquake ground motions that are two-thirds (2/3) of the corresponding MCE_R ground motions. Design earthquake ground motion parameters are provided in Table 2. A Risk Category I was determined using Table 1604.5 and the Seismic Design Category is D since S_1 is less than 0.75.

The Maximum Considered Earthquake Geometric Mean (MCE_G) peak ground acceleration (PGA_M) value was determined from the "U.S. Seismic Design Maps Web Application" (USGS, 2017) for liquefaction and seismic settlement analysis in accordance with 2016 CBC Section 1803.5.12 and CGS Note 48 (PGA_M = $F_{PGA}*PGA$). A PGA_M value of 0.50g has been determined for this project site.

Surface Rupture: The project site does not lie within a State of California, Alquist-Priolo Earthquake Fault Zone. Surface fault rupture at the project site is considered to be low.

Other Hazards

Hazardous Materials: The site is not located in proximity to any known hazardous materials (methane gas, tar seeps, hydrogen sulfide gas), and the risk of hazardous materials is considered very low.

Radon 222 Gas: Radon gas is not believed to be a potential hazard at the site. A report titled "California Statewide Radon Survey-Screening Results", dated November 1990 and published by the California State Department of Health Services, notes that Southern California showed a low risk of elevated radon levels, based on 2-day tests conducted from January through April 1990. Some of the reported testing was performed in Imperial County; however, no data was observed as being at or near the project site.

Naturally occurring asbestos: The site is not located in proximity to any known naturally occurring asbestos, and the risk of naturally occurring asbestos is considered very low.

Hydrocollapse: The site is dominantly underlain by clays that are not expected to collapse with the addition of water to the site. The risk of hydrocollapse is considered very low.

Regional Subsidence: Regional subsidence due to geothermal resource activities has not been documented in the area west of the New River; therefore, the risk of regional subsidence is considered low.

Conclusion

This preliminary report was prepared according to the generally accepted *geotechnical* engineering standards of practice that existed in Imperial County at the time the report was prepared. No express or implied warranties are made in connection with our services.

Our research did not reveal conditions that would preclude implementation of the proposed project provided site specific geotechnical investigations are conducted prior to site development to provide geotechnical criteria for the design and construction of this project.

No. 31921 EXPIRES 12-31-18

We appreciate the opportunity to provide our findings and professional opinions regarding geologic and geotechnical hazards at the site. If you have any questions or comments regarding our findings, please call our office at (760) 370-3000.

Sincerely Yours;

Landmark Consultants, Inc.

|

020 220

CERTIFIED ENGINEERING GEOLOGIST

Steven K Williams, PG, CEG Senior Engineering Geologist Jeffrey O. Lyon, PE President

Table 1
Summary of Characteristics of Closest Known Active Faults

Fault Name	Approximate Distance (miles)	Approximate Distance (km)	Maximum Moment Magnitude (Mw)	Fault Length (km)	Slip Rate (mm/yr)
Unnamed 1*	2.5	4.1			
Yuha*	4.5	7.2			
Unnamed 2*	4.9	7.8			
Shell Beds	7.4	11.8			
Superstition Hills	7.8	12.5	6.6	23 ± 2	4 ± 2
Yuha Well *	7.8	12.5			
Superstition Mountain	9.4	15.1	6.6	24 ± 2	5 ± 3
Laguna Salada	9.9	15.9	7	67 ± 7	3.5 ± 1.5
Vista de Anza*	11.0	17.5			
Imperial	13.0	20.8	7	62 ± 6	20 ± 5
Borrego (Mexico)*	13.6	21.7			
Painted Gorge Wash*	14.1	22.6			
Brawley *	14.5	23.2			
Ocotillo*	15.5	24.8			
Rico *	18.0	28.7			
Elsinore - Coyote Mountain	19.2	30.7	6.8	39 ± 4	4 ± 2
Elmore Ranch	21.2	33.9	6.6	29 ± 3	1 ± 0.5
Pescadores (Mexico)*	21.2	34.0			
Cerro Prieto *	22.3	35.7			
Cucapah (Mexico)*	23.5	37.6			
San Jacinto - Borrego	24.9	39.8	6.6	29 ± 3	4 ± 2
San Andreas - Coachella	41.6	66.5	7.2	96 ± 10	25 ± 5

^{*} Note: Faults not included in CGS database.

Table 2 2016 California Building Code (CBC) and ASCE 7-10 Seismic Parameters

CBC Reference

Soil Site Class:

D

Table 20.3-1

Latitude:

32.7490 N

Longitude: -115.7043 W

Risk Category:

I D

Seismic Design Category:

Maximum Considered Earthquake (MCE) Ground Motion

Mapped MCE _R Short Period Spectral Response	S_s	1.500 g	Figure 1613.3.1(1)
Mapped MCE _R 1 second Spectral Response	S_1	0.600 g	Figure 1613.3.1(2)
Short Period (0.2 s) Site Coefficient	$\mathbf{F_a}$	1.00	Table 1613.3.3(1)
Long Period (1.0 s) Site Coefficient	$\mathbf{F_v}$	1.50	Table 1613.3.3(2	2)
MCE _R Spectral Response Acceleration Parameter (0.2 s)	S_{MS}	1.500 g	$= F_a * S_s$	Equation 16-37

 S_{M1}

Design Earthquake Ground Motion

MCE_R Spectral Response Acceleration Parameter (1.0 s)

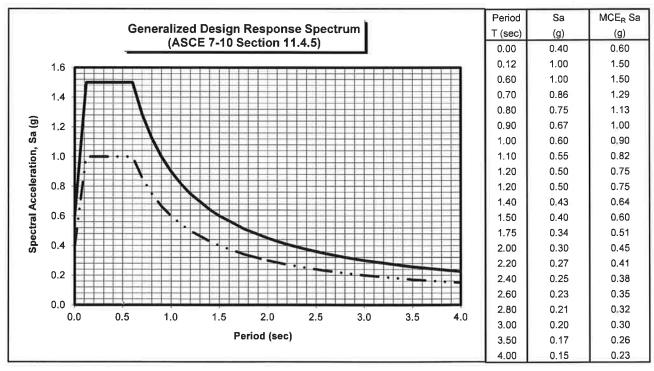
Design Spectral Response Acceleration Parameter (0.2 s)	S_{DS}	$1.000 \text{ g} = 2/3 * S_{MS}$	Equation 16-39
Design Spectral Response Acceleration Parameter (1.0 s)	S_{D1}	$0.600 \text{ g} = 2/3*S_{M1}$	Equation 16-40
	T_{L}	8.00 sec	ASCE Figure 22-12
	T_{0}	$0.12 \text{ sec} = 0.2 \cdot S_{D1} / S_{DS}$	

 $0.60 \text{ sec} = S_{D1}/S_{DS}$ T_{S}

0.900 g

Peak Ground Acceleration PGA_{M} 0.50 g ASCE Equation 11.8-1

Equation 16-38



Design Response Spectra MCE_R Response Spectra

MARK

Regional Fault Map

Figure 1



Source: California Geological Survey 2010 Fault Activity Map of California http://www.quake.ca.gov/gmaps/FAM/faultactivitymap.html#



Map of Local Faults

Figure 2

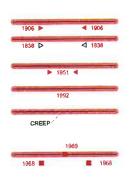
EXPLANATION

Fault traces on land are indicated by solid lines where well located, by dashed lines where approximately located or inferred, and by dotted lines where concealed by younger rocks or by lakes or bays. Fault traces are queried where continuation or existence is uncertain. Concealed faults in the Great Valley are based on maps of selected subsurface horizons, so locations shown are approximate and may indicate structural trend only. All offshore faults based on seismic reflection profile records are shown as solid lines where well defined, dashed where inferred, queried where uncertain.

FAULT CLASSIFICATION COLOR CODE (Indicating Recency of Movement)

Fault along which historic (last 200 years) displacement has occurred and is associated with one or more of the following:

- (a) a recorded earthquake with surface rupture. (Also included are some well-defined surface breaks caused by ground shaking during earthquakes, e.g. extensive ground breakage, not on the White Wolf fault, caused by the Arvin-Tehachapi earthquake of 1952). The date of the associated earthquake is indicated. Where repeated surface ruptures on the same fault have occurred, only the date of the latest movement may be indicated, especially if earlier reports are not well documented as to location of ground breaks.
- (b) fault creep slippage slow ground displacement usually without accompanying earthquakes.
- (c) displaced survey lines.



A triangle to the right or left of the date indicates termination point of observed surface displacement. Solid red triangle indicates known location of rupture termination point. Open black triangle indicates uncertain or estimated location of rupture termination point.

Date bracketed by triangles indicates local fault break.

No triangle by date indicates an intermediate point along fault break.

Fault that exhibits fault creep slippage. Hachures indicate linear extent of fault creep. Annotation (creep with leader) indicates representative locations where fault creep has been observed and recorded.

Square on fault indicates where fault creep slippage has occured that has been triggered by an earthquake on some other fault. Date of causative earthquake indicated. Squares to right and left of date indicate terminal points between which triggered creep slippage has occurred (creep either continuous or intermittent between these end points).

Holocene fault displacement (during past 11,700 years) without historic record. Geomorphic evidence for Holocene faulting includes sag ponds, scarps showing little erosion, or the following features in Holocene age deposits: offset stream courses, linear scarps, shutter ridges, and triangular faceted spurs. Recency of faulting offshore is based on the interpreted age of the youngest strata displaced by faulting.

Late Quaternary fault displacement (during past 700,000 years). Geomorphic evidence similar to that described for Holocene faults except features are less distinct. Faulting may be younger, but lack of younger overlying deposits precludes more accurate age classification.

Quaternary fault (age undifferentiated). Most faults of this category show evidence of displacement sometime during the past 1.6 million years; possible exceptions are faults which displace rocks of undifferentiated Plio-Pleistocene age. Unnumbered Quaternary faults were based on Fault Map of California, 1975. See Bulletin 201, Appendix D for source data.

Pre-Quaternary fault (older that 1.6 million years) or fault without recognized Quaternary displacement. Some faults are shown in this category because the source of mapping used was of reconnaissnce nature, or was not done with the object of dating fault displacements. Faults in this category are not necessarily inactive.

LANDMARK
Geo-Engineers and Geologists
Project No.: LE17082

Fault Map Legend

Figure 3a

ADDITIONAL FAULT SYMBOLS

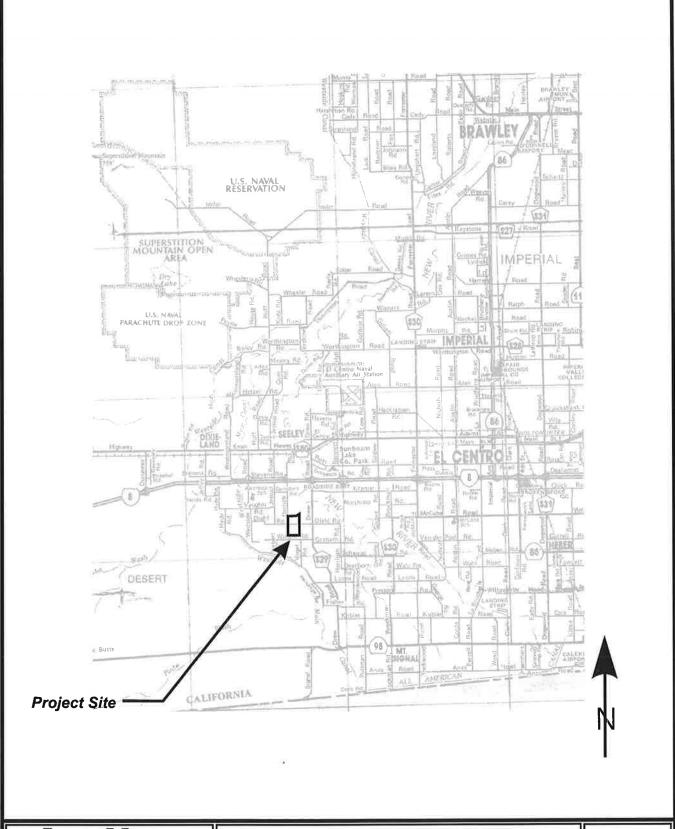
	Bar and ball on downthrown side (relative or apparent).
	Arrows along fault indicate relative or apparent direction of lateral movement.
<u></u>	Arrow on fault indicates direction of dip.
3.	Low angle fault (barbs on upper plate). Fault surface generally dips less than 45° but locally may have been subsequently steepened. On offshore faults, barbs simply indicate a reverse fault regardless of steepness of dip.
	OTHER SYMBOLS
	Numbers refer to annotations listed in the appendices of the accompanying report. Annotations include fault name, age of fault displacement, and pertinent references including Earthquake Fault Zone maps where a fault has been zoned by the Alquist-Priolo Earthquake Fault Zoning Act. This Act requires the State Geologist to delineate zones to encompass faults with Holocene displacement.
	Structural discontinuity (offshore) separating differing Neogene structural domains. May indicate discontinuities between basement rocks.
	Brawley Seismic Zone, a linear zone of seismicity locally up to 10 km wide associated with the releasing step between the Imperial and San Andreas faults.

Geologic		С	Years Before	Fault	Recency	DESCRIPTION					
	ime cale		Present (Approx.)	Symbol	of Movement	ON LAND	OFFSHORE				
	y	Historic		-		Displacement during historic time (includes areas of known fault creep					
	Late Quatemary	Holocone	200		- 2	Displacement during Holocome time	Fault uffliers seafloor additionnis or strata of holocone age				
Quaternary	Late C	6	— 11,700 —		.5	Faults showing existing of conferenced during teas. Quaternary time.	Food cuts strata of Lace Prostocome age				
Quate	Early Quatemary	Pleistocene	700,000 —		-1-	Undwided Quaternary lawls- most builts in this category show witherce of displacement during the that 1.800,000 years, possible asceptions are faults which displace rocks of undifferentiated Pass Placeticume egu	Fault cuts streta of Quaternary age				
Pre-Quaternary						Faults without recognized Quaternary displacement or showing evidence of no displacement during Quaternary time. Not necessarily inactive.	Faull cuts strata of Pliocene or older age.				

Quaternary now recognized as extending to 2.6 Ma (Walker and Geissman, 2009). Quaternary faults in this map were established using the
previous 1.6 Ma criterion.

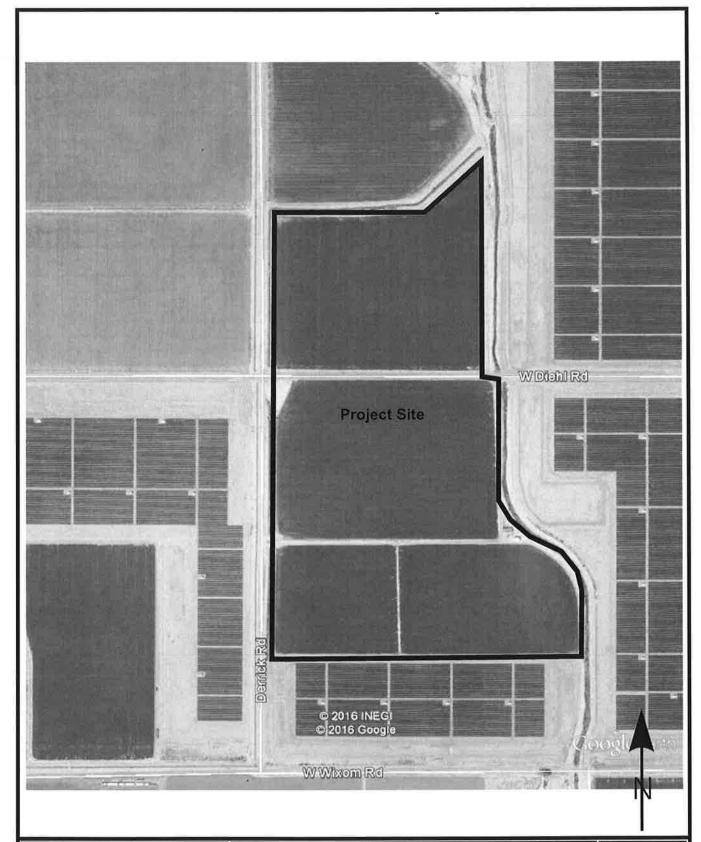


APPENDIX A



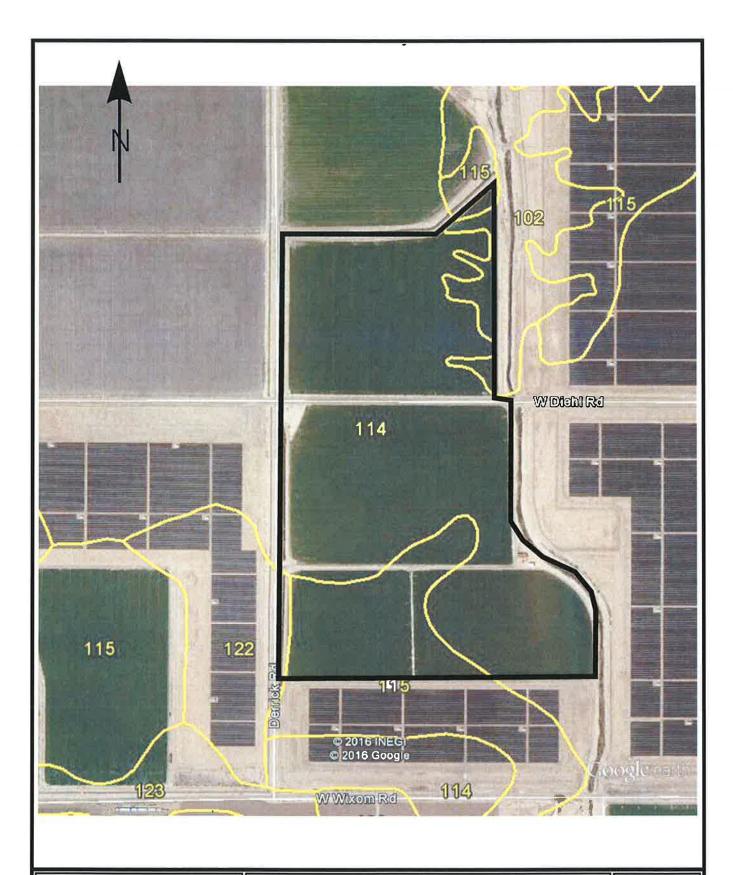
LANDMARK
Geo-Engineers and Geologists
Project No.: LE17082

Vicinity Map



LANDMARK
Geo-Engineers and Geologists
Project No.: LE17082

Site Map



LANDMARK

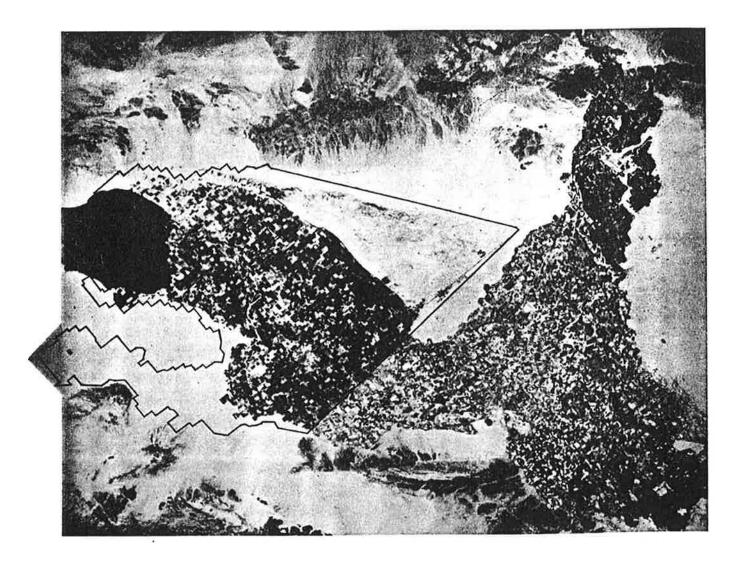
Geo-Engineers and Geologists

Project No.: LE17082

Soil Survey Map

Soil Survey of

IMPERIAL COUNTY CALIFORNIA IMPERIAL VALLEY AREA



United States Department of Agriculture Soil Conservation Service
in cooperation with
University of California Agricultural Experiment Station
and
Imperial Irrigation District

TABLE 11.--ENGINEERING INDEX PROPERTIES

[The symbol > means more than. Absence of an entry indicates that data were not estimated]

Soil name and	Depth	USDA texture	Classif		Frag- ments	i Pi	ercentag sieve m	ge passi number		Liquid	Plas-
map symbol			Unified		> 3 inches	4	10	40	200	<u> </u>	ticity index
	I In				Pot					Pat	
100 Antho		Loamy fine sand Sandy loam, fine sandy loam.	SM	A-2 A-2, A-4	0	100 9 0-1 00		75-85 50-60			N P N P
101*:			au	4 2		100	100	75 05	10.20		N.D.
Antho		Sandy loam, fine sand sandy loam, fine	SM	A-2 A-2, A-4	0 0	100 90=100 	75 - 95	50-60	15-40		N P N P
Superstition	6-60	Fine sand Loamy fine sand, fine sand, sand.		A-2 A-2	0 0		95 - 100 95 - 100			===	N P N P
102*. Badland											
103 Carsitas	110-60	Gravelly sand Gravelly sand, gravelly coarse sand, sand.	ISP, SP-SM	A-1, A-2 A-1		60 - 90 60 - 90					N P N P
104 * Fluvaquents											
105 Glenbar	13-60	Clay loam Clay loam, silty clay loam.		A-6 A-6	0 0	100 100		90-100 90-100			15-30 15-30
106 Glenbar	13-60	Clay loam Clay loam, silty clay loam.		A-6, A-7 A-6, A-7		100 100		90-100 90-100		35-45 35-45	15-25 15-25
107* Glenbar	0-13	Loam	ML, CL-ML, CL	A – 4	0	100	100	100	70-80	20-30	NP-10
		Clay loam, silty clay loam.		A-6, A-7	0	100	100	95-100	75-95	35-45	15-30
108 Holtville	14-22 22-60	LoamClay, silty clay Silt loam, very fine sandy loam,	CL, CH	A – 4 A – 7 A – 4	0 0 0	100 100 100	1 100	85-100 95-100 95-100	85-95		NP-10 20-35 NP-10
	17-24 24-35	Clay, silty clay Silt loam, very fine sandy	CL, CH	A-7 A-7 A-4	0 0 0	100 100 100	100	95-100 95-100 95-100	85-95	40-65	20-35 20-35 NP-10
		loam. Loamy very fine sand, loamy fine sand.	SM, ML	A-2, A-4	0	100	100	75-100	20 - 55		ΝP
110Holtville	17→24 24 - 35	Silty clay Clay, silty clay Silt loam, very fine sandy loam.	CH, CL	A-7 A-7 A-4	0 0 0	100 100 100		95-100 95-100 95-100	185-95	40-65 40-65 25-35	20-35 20-35 NP-10
	35-60	Loamy very fine sand, loamy fine sand.	SM, ML	A-2, A-4	0	100	100	75-100	20-55		ΝP

See footnote at end of table.

TABLE 11.--ENGINEERING INDEX PROPERTIES--Continued

Soil name and	Depth	USDA texture	Classi	fication	Frag-			e passi umber		Liquid	Plas-
map symbol	. Dopon	551 56x 561 6	Unified	AASHTO	> 3	T	10	40	200	limit	ticity index
111*:	<u>In</u>				Pet					Pet	
Holtville	10-22	Silty clay loam Clay, silty clay Silt loam, very fine sandy loam.	CL, CH	A-7 A-7 A-4	0 0	100 100 100	100	95-100 95-100 95-100	85-95	40-65 40-65 25-35	20-35 20-35 NP-10
Imperial	112-60	Silty clay loam Silty clay loam, silty clay, clay.		A-7 A-7	0	100 100	100 100		85-95 85-95	40-50 50-70	10-20 25-45
112 Imperial	12-60	Silty clay Silty clay loam, silty clay, clay.		A-7 A-7	0	100 100	100 100		85-95 85-95	50-70 50-70	25-45 25-45
113 Imperial	112-60		CH CH	A-7 A-7	0	100 100	100 100		85-95 85-95		25-45 25-45
114Imperial	112-60	Silty clay Silty clay loam, silty clay, clay.		A-7 A-7	0	100 100	100 100		85-95 85-95	50-70 50-70	25-45 25-45
115#: Imperial		Silty clay loam Silty clay loam, silty clay, clay.		A-7 A-7	0	100 100	100 100		85-95 85-95	40-50 50-70	10-20 25-45
Glenbar		Silty clay loam Clay loam, silty clay loam.		A-6, A-7		100 100	100	90-100 90-100		35-45 35-45	15-25 15-25
116*: Imperial		Silty clay loam Silty clay loam, silty clay, clay.		A-7 A-7	0	100 100	100 100	100 100	85-95 85-95	40-50 50 - 70	10-20 25-45
Glenbar	0-13 13-60	Silty clay loam Clay loam, silty clay loam.	CL	A-6, A-7 A-6	0	100	100 100	90-100 90-100		35-45 35-45	15-25 15-30
117, 118Indio	112-72	Loam	r ML	A – 4 A – 4	0	95-100 95-100					NP-5 NP-5
119*: Indio		LoamStratified loamy very fine sand to silt loam.	/ I ML	A – 4 A – 4	0	95-100 95-100					NP-5 NP-5
Vint		Loamy fine sand Loamy sand, loamy fine sand.	SM SM	A-2 A-2	0	95-100 95-100	95-100 	70-80	20-30		NP NP
120* Laveen	0-12 12-60	Loam Loam, very fine sandy loam.	- ML, CL- ML, CL-	ML A-4 ML A-4	0	100 95-100	195-100 185-95 1	75-85 70-80	55-65 55-65	20-30 15-25	NP-10

See footnote at end of table.

TABLE 11. -- ENGINEERING INDEX PROPERTIES -- Continued

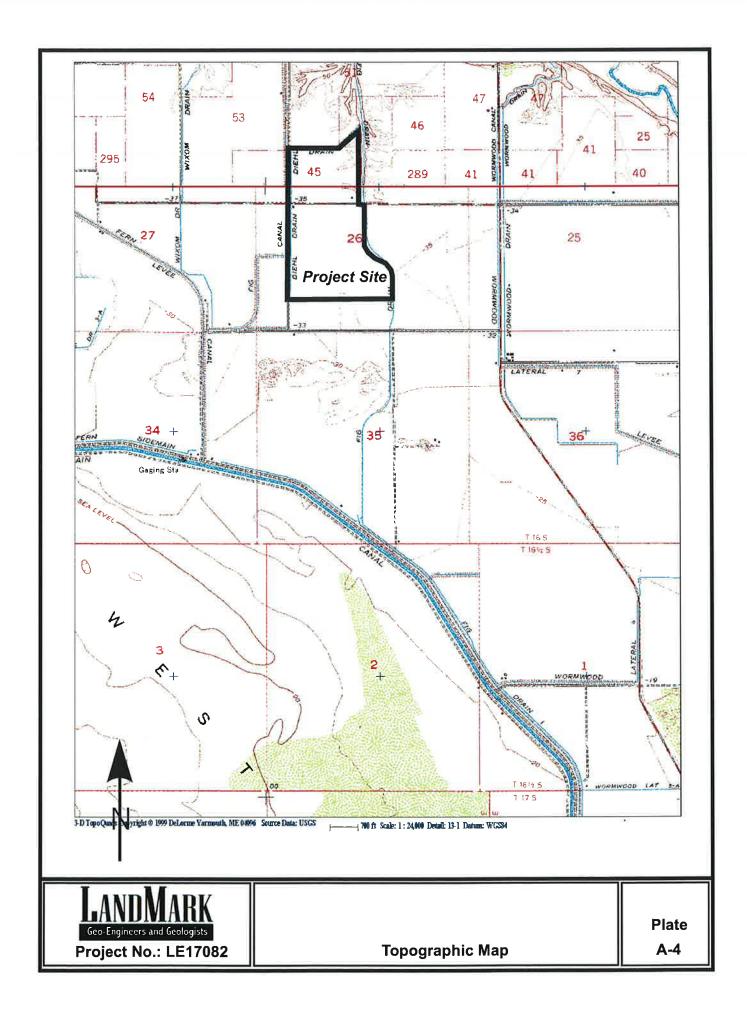
	Depth	USDA texture					ments		sieve n	umber		Liquid	Plas-
map symbol	Depun	John John J	Un:	ified	AASI	OTE	> 3 inches	4	10	40	200	limit	ticity index
	In						Pet					Pot	
121 Meloland	12-26	Fine sand Stratified loamy fine sand to			A-2, A-4	A-3	0	95 - 100 100	90-100 100	75-100 90-100	5 - 30 50-65	25 - 35	NP-10
	26-71	silt loam. Clay, silty clay, silty clay loam.	CL,	СН	A-7		0	100	100	95-100	185 - 95	40-65	20-40
1	0-12	Very fine sandy	ML		A-4		0	95-100	95-100	95-100	55-85	25-35	NP-10
Meloland		loam. Stratified loamy fine sand to	ML		A-4		0	100	100	90-100	50-70	25-35	NP-10
	26-71	silt loam. Clay, silty clay, silty clay loam.	СН,	CL	A-7		0	100	100	95-100	85-95	40-65	20-40
123*: Meloland	0-12	Loam	¦ ¦ML		A-4			95-100				25-35	NP-10
	12-26	Stratified loamy fine sand to silt loam.	ML 		A-4		0		100			25-35	NP-10
	26-38		СН,	CL	A-7		0	100		95 - 100 		40-65	20-40
	38-60	Stratified silt loam to loamy fine sand.	SM,	ML	A – 4		0	100	100 	75-100 	135 - 55	25 ~ 35	
	¦12 - 24 ¦24-36	<pre>!Clay, silty clay Silt loam, very fine sandy</pre>	ICH,	CL	A-4 A-7 A-4		0 0 0	100	100	85-100 95-100 95-100		25-35 40-65 25-35	NP-10 20-35 NP-10
	¦ }36 ~ 60 ¦	loam. Loamy very fine sand, loamy fine sand.	SM,	ML	A-2,	A = 4	0	100	100	75-100	20-55		ЯP
124, 125 Niland	0=23 23=60	Gravelly sand Silty clay, clay, clay loam.	SM, CL,	SP-SM CH	A-2, A-7	A-3	0		170 - 95			40 - 65	NP 20-40
126 Niland	0-23 23 - 60	Fine sand Silty clay	SM,	SP-SM CH	A-2,	A - 3	0		100			40-65	NP 20-40
127 Niland	0-23 23-60	Loamy fine sand Silty clay	SM CL,	СН	A-2 A-7		0		90-100 100			40-65	NP 20-40
128*: Niland		Gravelly sand Silty clay, clay, clay loam.	SM,	SP-SM CH	A-2, A-7	A-3	3 O O				5-25 80-100	40-65	NP 20-40
<pre>fmperial</pre>		Silty clay Silty clay loam, silty clay, clay.			A-7		0	100	100	100	85 - 95 85 - 95	50-70 50-70	25-45 25-45
129*: Pits	1												
130, 131 Rositas	0-27	Sand	SP-	-SM	A-3 A-	١,	0	100	80 - 100	140-70	5-15		NP
	27-60	Sand, fine sand, loamy sand.	SM	SP-SM		, 2,	0	100	80-100	40-85	5-30		NP

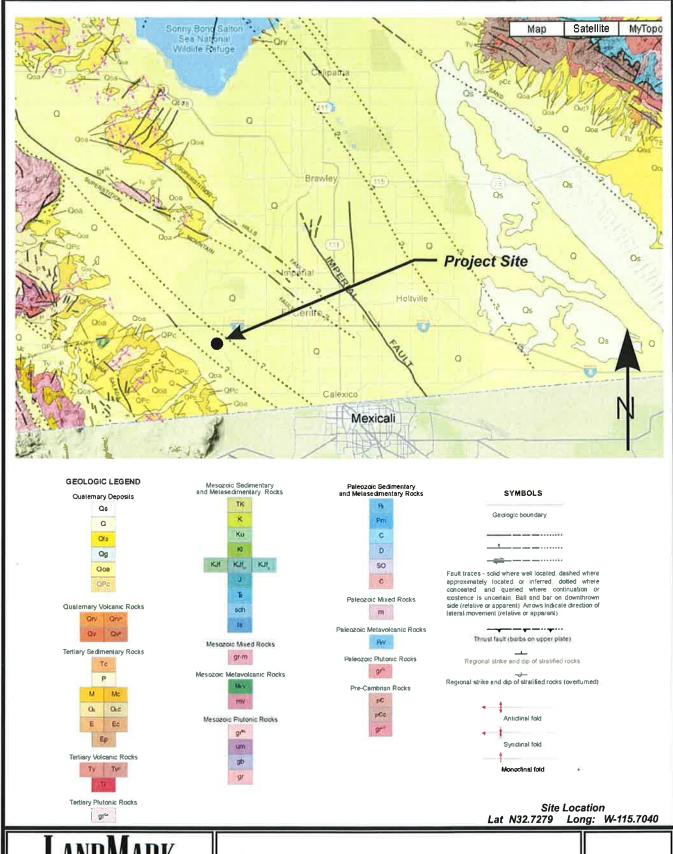
See footnote at end of table.

TABLE 11.--ENGINEERING INDEX PROPERTIES--Continued

Soil name and	Depth	USDA texture	Classif	ication	Frag- ments	P	ercenta, sieve	ge pass number-		Liguid	Plas-
map symbol	1	Jan	Unified		> 3 inches	4	10	40	200	limit	ticity index
	<u>In</u>				Pet					Pet	
132, 133, 134, 135- Rositas	0-9	Fine sand	ISM	A-3,	0	100	80-100	150-80	10-25		NP
	9-60	Sand, fine sand, loamy sand.	SM, SP-SM	A-3, A-2, A-1	0	100	80-100	40-85	5-30		ΝP
136 Rositas		Loamy fine sand Sand, fine sand, loamy sand.		A-1, A-2 A-3, A-2, A-1	0 0		80-100 80-100			==	NP NP
137 Rositas	0-12 12-60	 Silt loam Sand, fine sand, loamy sand.	ML SM, SP-SM	A-4 A-3, A-2, A-1	0 0	100 100	100 80-100	90-100 40-85		20-30	NP-5 NP
138*:	<u> </u>	 	1				!				
Rositas		Loamy fine sand Sand, fine sand, loamy sand.	SM SM, SP-SM 	A-1, A-2 A-3, A-2, A-1	0		80-100 80-100			===	N P N P
Superstition	6-60	Loamy fine sand Loamy fine sand, fine sand, sand.	SM SM	A-2 A-2	0		95-100 95-100				NP NP
139Superstition	6-60	Loamy fine sand Loamy fine sand, fine sand, sand.		A-2 A-2	0		95-100 95-100				NP NP
140#: Torriorthents			1 1 1								
Rock outcrop			i]						i		
141*: Torriorthents											
Orthids											
142	0-10	Loamy very fine	SM, ML	A-4	0	100	100	85-95	 40 – 65	15-25	NP-5
Vint	10-60	sand. Loamy fine sand	SM	A-2	0	95-100	95 -1 00	70-80	 20 – 30		NP
143 Vint	0-12	Fine sandy loam	ML, CL-ML, SM,	A-4	0	100	100	75 - 85	 45 – 55	15-25	NP-5
		Loamy sand, loamy fine sand.	SM-SC	A-2	0	95-100	95-100	70-80	20-30		NΡ
144#:										45	
1	1	Very fine sandy loam.		A-4	0	100		85-95	1	15~25	NP-5
		Loamy fine sand Silty clay		A-2 A-7			95~100 100			40-65	NP 20-35
Indio	0-12	Very fine sandy	ML	A-4	0	95-100	95-100	85 - 100	: 75 - 90	20-30	NP-5
	İ	loam. Stratified loamy very fine sand	ML	A-4	0	95-100	95-100	85-100	 75 - 90	20-30	NP-5
		to silt loam. Silty clay	CL, CH	A-7	0	100	100	95-100	 85 - 95	40-65	20-35

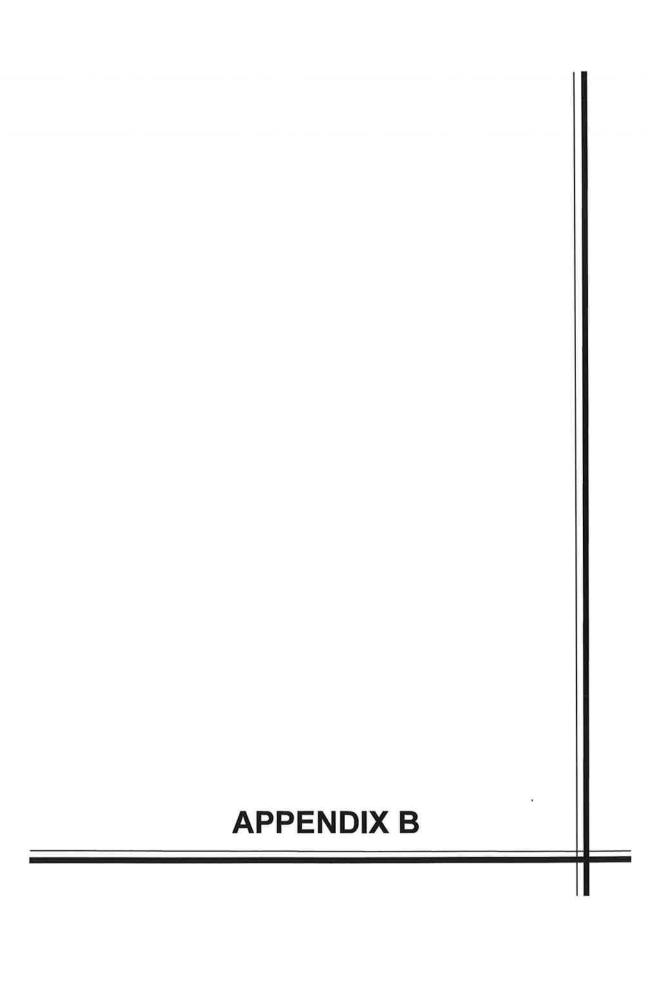
 $^{{}^{*}}$ See description of the map unit for composition and behavior characteristics of the map unit.





Geo-Engineers and Geologists
Project No.: LE17082

Regional Geologic Map



REFERENCES

- American Concrete Institute (ACI), 2013, ACI Manual of Concrete Practice 302.1R-04
- American Society of Civil Engineers (ASCE), 2010, Minimum Design Loads for Buildings and Other Structures: ASCE Standard 7-10.
- California Building Standards Commission, 2017, 2016 California Building Code. California Code of Regulations, Title 24, Part 2, Vol. 2 of 2.
- Caltrans, 2012, Highway Design Manual.
- California Division of Mines and Geology (CDMG), 1996, California Fault Parameters: available at http://www.consrv.ca.gov/dmg/shezp/fltindex.html.
- California Geological Survey (CGS), 2017, Fault Activity Map of California http://www.quake.ca.gov/gmaps/FAM/faultactivitymap.html#.
- California Geological Survey (CGS), 2017, Alquist-Priolo Earthquake Fault Zone Maps. http://maps.conservation.ca.gov/cgs/informationwarehouse/index.html?map=regulatorymaps
- Cetin, K. O., Seed, R. B., Der Kiureghian, A., Tokimatsu, K., Harder, L. F., Jr., Kayen, R. E., and Moss, R. E. S., 2004, Standard penetration test-based probabilistic and deterministic assessment of seismic soil liquefaction potential: ASCE JGGE, Vol., 130, No. 12, p. 1314-1340.
- Geologismiki, 2014, CLiq Computer Program, www.geologismiki.gr
- Ishihara, K. (1985), Stability of natural deposits during earthquakes, Proc. 11th Int. Conf. On Soil Mech. And Found. Engrg., Vol. 1, A. A. Balkema, Rotterdam, The Netherlands, 321-376.
- Jones, A. L., 2003, An Analytical Model and Application for Ground Surface Effects from Liquefaction, PhD. Dissertation, University of Washington, 362 p.
- Morton, P. K., 1977, Geology and mineral resources of Imperial County, California: California Division of Mines and Geology, County Report No. 7, 104 p.
- Post-Tensioning Institute (PTI), 2007a, Standard Requirements for Analysis of Shallow Concrete Foundations on Expansive Soils (3rd Edition).
- Post-Tensioning Institute (PTI), 2007b, Standard Requirements for Design of Shallow Post-Tensioned Concrete Foundations on Expansive Soils (2nd Edition).

- Robertson, P. K., 2014, Seismic liquefaction CPT-based methods: EERI 1st Workshop on Geotechnical Earthquake Engineering Liquefaction Evaluation, Mapping, Simulation and Mitigation. UC San Diego Campus, 10/12/2014.
- Robertson, P. K. and Wride, C. E., 1997, Cyclic Liquefaction and its Evaluation based on the SPT and CPT, Proceeding of the NCEER Workshop on Evaluation of Liquefaction Resistance of Soils, NCEER Technical Report 97-0022, p. 41-88.
- U.S. Geological Survey (USGS), 1990, The San Andreas Fault System, California, Professional Paper 1515.
- U.S. Geological Survey (USGS), 2017, US Seismic Design Maps Web Application, available at http://geohazards.usgs.gov/designmaps/us/application.php
- Wire Reinforcement Institute (WRI/CRSI), 2003, Design of Slab-on-Ground Foundations, Tech Facts TF 700-R-03, 23 p.
- Youd, T. L., 2005, Liquefaction-induced flow, lateral spread, and ground oscillation, GSA Abstracts with Programs, Vol. 37, No. 7, p. 252.
- Youd, T. L. and Garris, C. T., 1995, Liquefaction induced ground surface disruption: ASCE Geotechnical Journal, Vol. 121, No. 11.
- Zimmerman, R. P., 1981, Soil survey of Imperial County, California, Imperial Valley Area: U.S. Dept. of Agriculture Soil Conservation Service, 112 p.

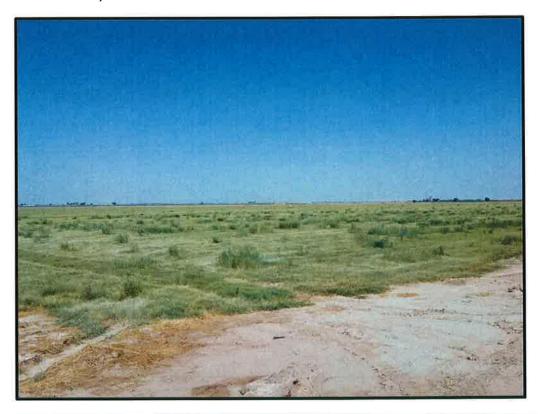
Preliminary Geotechnical and GeoHazards Report

Laurel 2 Solar Site SEC Liebert Road and Wixom Road

El Centro, California

Prepared for:

90FI 8ME, LLC 211 Sutter Street, 6th Floor San Francisco, CA 94108





Prepared by:

Landmark Consultants, Inc. 780 N. 4th Street El Centro, CA 92243 (760) 337-1100

August 2017



August 31, 2017

780 N. 4th Street El Centro, CA 92243 (760) 370-3000 (760) 337-8900 fax

77-948 Wildcat Drive Palm Desert, CA 92211 (760) 360-0665 (760) 360-0521 fax

Mr. Daniel Kolta 90FI 8ME, LLC 211 Sutter Street, 6th Floor San Francisco, CA 94108

Preliminary Geological and Geotechnical Hazard Evaluation Laurel 2 Solar Site SEC Liebert Road and Wixom Road El Centro, California LCI Project No. LE17141

Dear Mr. Kolta:

This preliminary geotechnical report and geologic hazards study is provided for preliminary site evaluation and permitting of the photo-voltaic solar farm at the approximately 280-acre project area (APNs 051-300-032, 051-300-036, 051-310-027 and 051-310-028) located at the southeast and northwest corners of Vaughn Road and Jessup Road approximately 8 miles west of El Centro, California.

Scope of Work

The scope of work consisted of a geologic and geotechnical hazards evaluation of the project site which addresses the following items:

- 1. General site geology.
- 2. Site location in relation to mapped earthquake faults and seismic zones.
- 3. Intensity of ground shaking at the site.
- 4. Potential for liquefaction, ground failure, and landslides at the site. No drilling was conducted to determine potential for liquefaction settlement or soil analysis.
- 5. Soil corrosivity.
- 6. Plant growth suitability of the site soils.
- 7. Preliminary pavement structural sections.
- 8. Potential for flooding at the site from manmade facilities (dams, canals, etc.) and from natural storms.
- 9. Other potential geologic and geotechnical hazards.

Site Description

The project site is located at the southeast and northwest corners of Vaughn Road and Jessup Road approximately 8 miles west of El Centro, California. The project site consists of approximately 280-acres comprised of two separate parcels each consisting of two (2) agricultural fields currently in crop production.

<u>Parcel 1:</u> Parcel 1 is comprised of two (2) agricultural fields totaling approximately 160 acres. The parcel is roughly square in plan view. The parcel is bounded on the south by Diehl Road and the west by Jessup Road. Derrick Road forms the eastern boundary of the site. A dirt field road and irrigation canal bisects the site in an east-west direction.

<u>Parcel 2:</u> Parcel 2 is comprised of two (2) agricultural fields totaling approximately 120 acres. The parcel is roughly rectangular in plan view. The parcel is bounded on the south by Vaughn Road and the east by Jessup Road. Derrick Road forms the eastern boundary of the site. A dirt field road bisects the site in a north-south direction.

Agricultural fields are located around the perimeter of the project site. Dirt field roads are located along the margins and also cross the parcels. The adjacent properties are approximately the same elevation as the project sites.

Site Geological Conditions

Site Geology: The project site is located in the Imperial Valley portion of the Salton Trough physiographic province. The Salton Trough is a topographic and geologic structural depression resulting from large scale regional faulting. The trough is bounded on the northeast by the San Andreas Fault and Chocolate Mountains and the southwest by the Peninsular Range and faults of the San Jacinto Fault Zone. The Salton Trough represents the northward extension of the Gulf of California, containing both marine and non-marine sediments since the Miocene Epoch. Tectonic activity that formed the trough continues at a high rate as evidenced by deformed young sedimentary deposits and high levels of seismicity. Figure 1 shows the location of the site in relation to regional faults and physiographic features.

The Imperial Valley is directly underlain by lacustrine deposits, which consist of interbedded lenticular and tabular silt, sand, and clay. Based on Unified Soil Classification System, the permeability of these soils is expected to be low to moderate.

The Late Pleistocene to Holocene lake deposits are probably less than 100 feet thick and derived from periodic flooding of the Colorado River which intermittently formed a fresh water lake (Lake Cahuilla).

Older deposits consist of Miocene to Pleistocene non-marine and marine sediments deposited during intrusions of the Gulf of California. Basement rock consisting of Mesozoic granite and Paleozoic metamorphic rocks are estimated to exist at depths between 15,000 - 20,000 feet.

Groundwater: The groundwater in the site area is brackish and typically encountered at a depth of between 5 to 10 feet below ground surface in the vicinity of the project site. There is uncertainty in the accuracy of short-term water level measurements, particularly in fine-grained soil. Groundwater levels may fluctuate with precipitation, irrigation of adjacent properties, drainage, and site grading. The groundwater level noted should not be interpreted to represent an accurate or permanent condition.

Onsite Wastewater Disposal: The near surface soils at the project site generally consist of silts and silty sands having a moderate infiltration rate. The near surface soils are considered good in supporting onsite septic systems and leach fields for wastewater disposal. Site specific studies will be required to determine that County Environmental Health standards are met in regard to soil percolation rates and separation of leach fields from groundwater.

Geological Hazards

Landsliding: No ancient landslides are shown on geologic maps of the region and no indications of landslides were observed during our site investigation. The hazard of landsliding is unlikely due to the relatively planar topography of the project site.

Volcanic hazards: The site is not located proximal to any known volcanically active area and the risk of volcanic hazards is considered very low.

Tsunamis, seiches, and flooding: The site does not lie near any large bodies of water, so the threat of tsunami, seiches, or other seismically-induced flooding is considered unlikely. The project site is located in FEMA Flood Zone X, an area determined to be outside the 0.2% annual chance floodplain (FIRM Panel 06025C1700C).

Expansive soil: In general, much of the near surface soils within the project site consist of silty clays and clay having a moderate to high expansion potential. A site specific geotechnical investigation will be required at this site to determine the extent and effect of expansive soils.

Corrosive Soils: The lacustrine site soils (lake bed deposits) are known to be corrosive. Typical remediation for the corrosive soil conditions consists of using concrete mixed with higher cement contents (6 sacks Type V Portland Cement) and low water-cement ratios (0.45 w/c ratio). Additionally, steel post corrosion protection is required, consisting of zinc coatings (galvanizing) or increased structural sections to compensate for metal loss due to corrosion.

Liquefaction/Seismic Settlements: Liquefaction is a potential design consideration because of possible saturated sandy substrata underlying the site. Liquefaction occurs when granular soil below the water table is subjected to vibratory motions, such as produced by earthquakes. With strong ground shaking, an increase in pore water pressure develops as the soil tends to reduce in volume. If the increase in pore water pressure is sufficient to reduce the vertical effective stress (suspending the soil particles in water), the soil strength decreases and the soil behaves as a liquid (similar to quicksand). Liquefaction can produce excessive settlement, ground rupture, lateral spreading, or failure of shallow bearing foundations.

Four conditions are generally required for liquefaction to occur:

- (1) the soil must be saturated (relatively shallow groundwater);
- (2) the soil must be loosely packed (low to medium relative density);
- (3) the soil must be relatively cohesionless (not clayey); and
- (4) groundshaking of sufficient intensity must occur to function as a trigger mechanism.

All of these conditions may exist to some degree at this site. Liquefaction settlement and ground fissures were noted along the Westside Main Canal in the area of the project site after the April 4, 2010 magnitude 7.2M_w El Mayor-Cucapah Earthquake. McCrink and others (2011) reported several liquefaction related failures to the embankment of the Westside Main Canal southwest of the project site.

Seismic Hazards

The project site is located in the seismically active Imperial Valley of southern California with numerous mapped faults of the San Andreas Fault System traversing the region. The San Andreas Fault System is comprised of the San Andreas, San Jacinto, and Elsinore Fault Zones in southern California. The Imperial fault represents a transition from the more continuous San Andreas fault to a more nearly echelon pattern characteristic of the faults under the Gulf of California (USGS, 1990). We have performed a computer-aided search of known faults or seismic zones that lie within a 62 mile (100 kilometer) radius of the project site (Table 1).

A fault map illustrating known active faults relative to the site is presented on Figure 1, Regional Fault Map. A legend for the regional fault map is presented on Figure 2. The criterion for fault classification adopted by the California Geological Survey defines Earthquake Fault Zones along active or potentially active faults. An active fault is one that has ruptured during Holocene time (roughly within the last 11,000 years). A fault that has ruptured during the last 1.8 million years (Quaternary time), but has not been proven by direct evidence to have not moved within Holocene time is considered to be potentially active. A fault that has not moved during both Pleistocene and Holocene time (that is no movement within the last 1.8 million years) is considered to be inactive.

Review of the current Alquist-Priolo Earthquake Fault Zone maps (CGS, 2000a) indicates that the nearest mapped Earthquake Fault Zone is an unnamed fault located approximately 2.1 miles west of the substation project site. Geologic mapping by the USGS (Rymer and others, 2011) of the Imperial Valley after the April 4, 2010 magnitude 7.2M_w El Mayor-Cucapah Earthquake indicates movement along several known and unknown faults west of the project site. Surface rupture on these faults is possible from future seismic events in the area.

The nearest mapped major Earthquake Fault Zone is the Laguna Salada fault located approximately 9.5 miles southwest of the site and the Superstition Hills fault located approximately 8.0 miles northeast of the project site.

Groundshaking. The primary seismic hazard at the project site is the potential for strong groundshaking during earthquakes along the Superstition Hills, Imperial, Cerro Prieto, and Laguna Salada faults (Figure 2).

<u>Site Acceleration</u>: The project site is considered likely to be subjected to moderate to strong ground motion from earthquakes in the region. Ground motions are dependent primarily on the earthquake magnitude and distance to the seismogenic (rupture) zone. Accelerations also are dependent upon attenuation by rock and soil deposits, direction of rupture and type of fault; therefore, ground motions may vary considerably in the same general area.

CBC General Ground Motion Parameters: The 2016 CBC general ground motion parameters are based on the Risk-Targeted Maximum Considered Earthquake (MCE_R). The U.S. Geological Survey "U.S. Seismic Design Maps Web Application" (USGS, 2017) was used to obtain the site coefficients and adjusted maximum considered earthquake spectral response acceleration parameters. The site soils have been classified as Site Class D (stiff soil profile).

Design spectral response acceleration parameters are defined as the earthquake ground motions that are two-thirds (2/3) of the corresponding MCE_R ground motions. Design earthquake ground motion parameters are provided in Table 2. A Risk Category II was determined using Table 1604.5 and the Seismic Design Category is D since S_1 is less than 0.75.

The Maximum Considered Earthquake Geometric Mean (MCE_G) peak ground acceleration (PGA_M) value was determined from the "U.S. Seismic Design Maps Web Application" (USGS, 2017) for liquefaction and seismic settlement analysis in accordance with 2016 CBC Section 1803.5.12 and CGS Note 48 (PGA_M = $F_{PGA}*PGA$). A PGA_M value of 0.50g has been determined for this project site.

Surface Rupture: The project site does not lie within a State of California, Alquist-Priolo Earthquake Fault Zone. Surface fault rupture at the project site is considered to be low. The nearest mapped earthquake fault zone is located approximately 2.3 miles southwest of the project site. This is an unnamed fault that was mapped after the 2010 7.2Mw El Mayor-Cucapah Earthquake.

Other Hazards

Hazardous Materials: The site is not located in proximity to any known hazardous materials (methane gas, tar seeps, hydrogen sulfide gas), and the risk of hazardous materials is considered very low.

Radon 222 Gas: Radon gas is not believed to be a potential hazard at the site. A report titled "California Statewide Radon Survey-Screening Results", dated November 1990 and published by the California State Department of Health Services, notes that Southern California showed a low risk of elevated radon levels, based on 2-day tests conducted from January through April 1990. Some of the reported testing was performed in Imperial County; however, no data was observed as being at or near the project site.

Naturally occurring asbestos: The site is not located in proximity to any known naturally occurring asbestos, and the risk of naturally occurring asbestos is considered very low.

Hydrocollapse: The site is dominantly underlain by clays that are not expected to collapse with the addition of water to the site. The risk of hydrocollapse is considered very low.

Regional Subsidence: Regional subsidence due to geothermal resource activities has not been documented in the area west of the New River; therefore, the risk of regional subsidence is considered low.

Conclusion

This preliminary report was prepared according to the generally accepted *geotechnical* engineering standards of practice that existed in Imperial County at the time the report was prepared. No express or implied warranties are made in connection with our services.

Our research did not reveal conditions that would preclude implementation of the proposed project provided site specific geotechnical investigations are conducted prior to site development to provide geotechnical criteria for the design and construction of this project.

We appreciate the opportunity to provide our findings and professional opinions regarding geologic and geotechnical hazards at the site. If you have any questions or comments regarding our findings, please call our office at (760) 370-3000.

CERTIFIED ENGINEERING GEOLOGIST

CEG 2261

No. 31921 EXPIRES 12-31-18

Sincerely Yours;

Landmark Consultants, Inc.

Steven K. Williams, PG, CEG

Senior Engineering Geologist

Jeffrey O. Lyon, PE President

Table 1
Summary of Characteristics of Closest Known Active Faults

Fault Name	Approximate Distance (miles)	Approximate Distance (km)	Maximum Moment Magnitude (Mw)	Fault Length (km)	Slip Rate (mm/yr)
Unnamed 1*	2.3	3.7			
Yuha*	4.3	6.9			
Unnamed 2*	5.5	8.8			
Shell Beds	6.7	10.7			
Yuha Well *	7.1	11.4			
Superstition Hills	8.0	12.8	6.6	23 ± 2	4 ± 2
Superstition Mountain	9.1	14.5	6.6	24 ± 2	5 ± 3
Laguna Salada	9.5	15.2	7	67 ± 7	3.5 ± 1.5
Vista de Anza*	10.4	16.6			
Painted Gorge Wash*	13.3	21.3			
Imperial	13.4	21.5	7	62 ± 6	20 ± 5
Borrego (Mexico)*	14.2	22.7			
Ocotillo*	14.8	23.7			
Brawley *	15.0	23.9			
Elsinore - Coyote Mountain	18.5	29.5	6.8	39 ± 4	4 ± 2
Rico *	18.5	29.6			
Elmore Ranch	20.5	32.9	6.6	29 ± 3	1 ± 0.5
Pescadores (Mexico)*	21.9	35.1			
Cerro Prieto *	23.1	37.0			
San Jacinto - Borrego	24.1	38.6	6.6	29 ± 3	4 ± 2
Cucapah (Mexico)*	24.2	38.7			
San Andreas - Coachella	41.1	65.8	7.2	96 ± 10	25 ± 5

^{*} Note: Faults not included in CGS database.

Regional Fault Map

Figure 1



Source: California Geological Survey 2010 Fault Activity Map of California http://www.quake.ca.gov/gmaps/FAM/faultactivitymap.html#



Map of Local Faults

Figure 2

EXPLANATION

Fault traces on land are indicated by solid lines where well located, by dashed lines where approximately located or inferred, and by dotted lines where concealed by younger rocks or by lakes or bays. Fault traces are queried where continuation or existence is uncertain. Concealed faults in the Great Valley are based on maps of selected subsurface horizons, so locations shown are approximate and may indicate structural trend only. All offshore faults based on seismic reflection profile records are shown as solid lines where well defined, dashed where Inferred, queried where uncertain.

FAULT CLASSIFICATION COLOR CODE (Indicating Recency of Movement)

Fault along which historic (last 200 years) displacement has occurred and is associated with one or more of the following:

- (a) a recorded earthquake with surface rupture. (Also included are some well-defined surface breaks caused by ground shaking during earthquakes, e.g. extensive ground breakage, not on the White Wolf fault, caused by the Arvin-Tehachapi earthquake of 1952). The date of the associated earthquake is indicated. Where repeated surface ruptures on the same fault have occurred, only the date of the latest movement may be indicated, especially if earlier reports are not well documented as to location of ground breaks.
- (b) fault creep slippage slow ground displacement usually without accompanying earthquakes.
- (c) displaced survey lines.

A triangle to the right or left of the date indicates termination point of observed surface displacement. Solid red triangle indicates known location of rupture termination point. Open black triangle indicates uncertain or estimated location of rupture termination point.

Date bracketed by triangles indicates local fault break.

No triangle by date indicates an intermediate point along fault break.

Fault that exhibits fault creep slippage. Hachures indicate linear extent of fault creep. Annotation (creep with leader) indicates representative locations where fault creep has been observed and recorded.

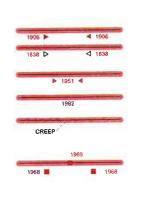
Square on fault indicates where fault creep slippage has occured that has been triggered by an earthquake on some other fault. Date of causative earthquake indicated. Squares to right and left of date indicate terminal points between which triggered creep slippage has occurred (creep either continuous or intermittent between these end points).

Holocene fault displacement (during past 11,700 years) without historic record. Geomorphic evidence for Holocene faulting includes sag ponds, scarps showing little erosion, or the following features in Holocene age deposits: offset stream courses, linear scarps, shutter ridges, and triangular faceted spurs. Recency of faulting offshore is based on the interpreted age of the youngest strata displaced by faulting.

Late Quaternary fault displacement (during past 700,000 years). Geomorphic evidence similar to that described for Holocene faults except features are less distinct. Faulting may be younger, but lack of younger overlying deposits precludes more accurate age classification.

Quaternary fault (age undifferentiated). Most faults of this category show evidence of displacement sometime during the past 1.6 million years; possible exceptions are faults which displace rocks of undifferentiated Plio-Pleistocene age. Unnumbered Quaternary faults were based on Fault Map of California, 1975. See Bulletin 201, Appendix D for source data.

Pre-Quaternary fault (older that 1.6 million years) or fault without recognized Quaternary displacement. Some faults are shown in this category because the source of mapping used was of reconnaissnce nature, or was not done with the object of dating fault displacements. Faults in this category are not necessarily inactive.



LANDWARK

Geo-Engineers and Geologists

Project No.: LE17141

Fault Map Legend

Figure 3a

ADDITIONAL FAULT SYMBOLS

	Bar and ball on downthrown side (relative or apparent).
	Arrows along fault indicate relative or apparent direction of lateral movement.
	Arrow on fault indicates direction of dip.
_{\sqrt} 2.	Low angle fault (barbs on upper plate). Fault surface generally dips less than 45° but locally may have been subsequently steepened. On offshore faults, barbs simply indicate a reverse fault regardless of steepness of dip.
	OTHER SYMBOLS
	Numbers refer to annotations listed in the appendices of the accompanying report. Annotations include fault name, age of fault displacement, and pertinent references including Earthquake Fault Zone maps where a fault has been zoned by the Alquist-Priolo Earthquake Fault Zoning Act. This Act requires the State Geologist to delineate zones to encompass faults with Holocene displacement.
	Structural discontinuity (offshore) separating differing Neogene structural domains. May indicate discontinuities between basement rocks.
	Brawley Seismic Zone, a linear zone of seismicity locally up to 10 km wide associated with the releasing

Geologic B		Years Before	Before Fault		DESCRIPTION					
	ime		Present (Approx.)	Symbol	of Movement	ON LAND	OFFSHORE			
	Included				Displacement during historic time (includes areas of known fault creep					
	Late Quaternary	Повесте	200-			Deptacement during Holocene time.	Fault offsets seaffour sediments or strata of Horizone age			
Quaternary	Late Q	9	11,700		-2-	Fault showing explanes of displacement during tale Quaternary time	Facilitate strate of Liste Presidence ago			
Quate	Early Quatemary	Pleistocene			7-	Undivided Quaternary faults most faults in the callingary show evaluate of displacement during the fact 1 600,000 years, possible recupition after faults which displace rocks of undifferentiated Piro-Planstocame, and	Fault cuts strata of Quaternary age			
Pre-Quaternary			— 1,600,000°—	~		Faults without recognized Quaternary displacement or showing evidence of no displacement during Quaternary time. Not necessarily inactive.	Fault cuts strata of Pliocene or older age.			
۵_			4.5 billion (Age of Earth)							

^{*} Quaternary now recognized as extending to 2.6 Ma (Walker and Geissman, 2009). Quaternary faults in this map were established using the previous 1.6 Ma criterion.



8.0

0.6

0.4

0.2

0.0

0.0

0.5

1.0

Table 2 2016 California Building Code (CBC) and ASCE 7-10 Seismic Parameters

CBC Reference Table 20.3-1

D Soil Site Class:

Latitude: 32.7558 N

D

Longitude: -115.7155 W

Risk Category:

Seismic Design Category:

Maximum Considered Earthquake (MCE) Ground Motion

Mapped MCE _R Short Period Spectral Response	S_s	1.500 g	Figure 1613.3.1(1)
Mapped MCE _R 1 second Spectral Response	S_1	0.600 g	Figure 1613.3.1(2)
Short Period (0.2 s) Site Coefficient	$\mathbf{F_a}$	1.00	Table 1613.3.3(1)
Long Period (1.0 s) Site Coefficient	$\mathbf{F}_{\mathbf{v}}$	1.50	Table 1613.3.3(2)

Equation 16-37 MCE_R Spectral Response Acceleration Parameter (0.2 s) 1.500 g $= F_a * S_s$ S_{MS} MCE_R Spectral Response Acceleration Parameter (1.0 s) 0.900 g $= F_v * S_1$ Equation 16-38 S_{M1}

Design Earthquake Ground Motion

Equation 16-39 Design Spectral Response Acceleration Parameter (0.2 s) 1.000 g $= 2/3*S_{MS}$ S_{DS} Design Spectral Response Acceleration Parameter (1.0 s) 0.600 g $= 2/3*S_{M1}$ Equation 16-40 S_{D1}

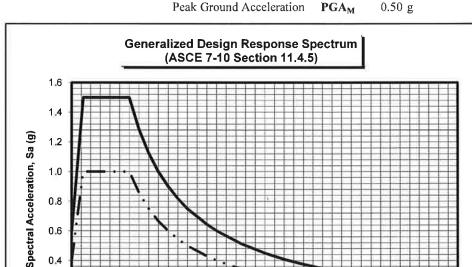
ASCE Figure 22-12 T_L 8.00 sec

4.0

Period

 T_0 $0.12 \text{ sec} = 0.2 \text{ }^{\circ}\text{S}_{D1}/\text{S}_{DS}$ $0.60 \text{ sec} = S_{D1}/S_{DS}$ T_S

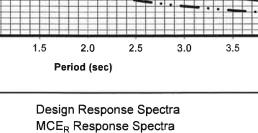
ASCE Equation 11.8-1



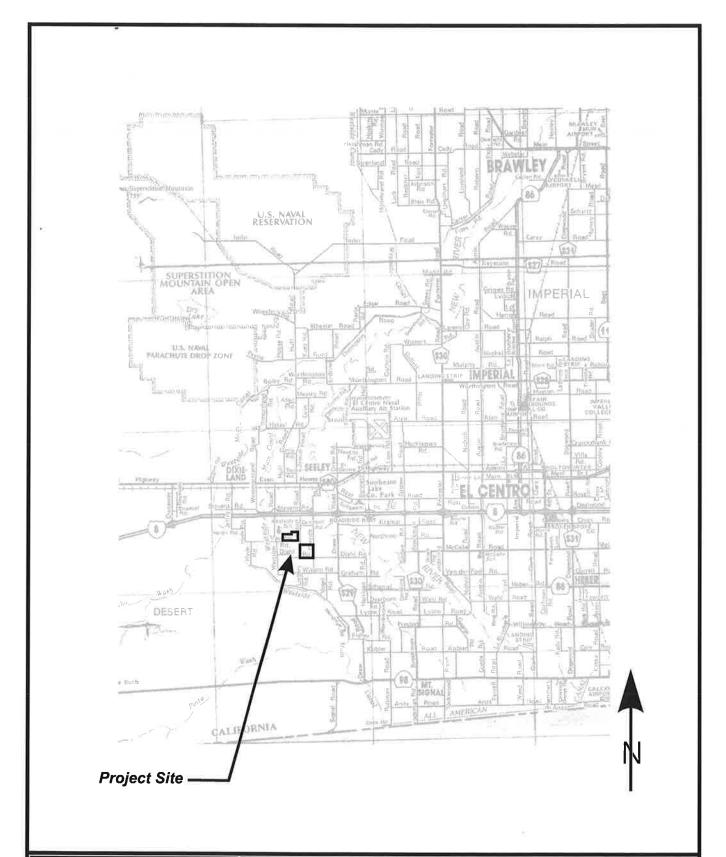
	T (sec)	(g)	(g)		
	0.00	0.40	0.60		
	0.12	1.00	1.50		
H	0.60	1.00	1.50		
9	0.70	0.86	1.29		
	0.80	0.75	1.13		
	0.90	0.67	1.00		
	1.00	0.60	0.90		
	1.10	0.55	0.82		
	1.20	0.50	0.75		
d	1.20	0.50	0.75		
	1.40	0.43	0.64		
	1.50	0.40	0.60		
H	1.75	0.34	0.51		
ľ	2.00	0.30	0.45		
	2.20	0.27	0.41		
	2.40	0.25	0.38		
	2.60	0.23	0.35		
P	2.80	0.21	0.32		
	3.00	0.20	0.30		
	3.50	0.17	0.26		
	4.00	0.15	0.23		

Sa

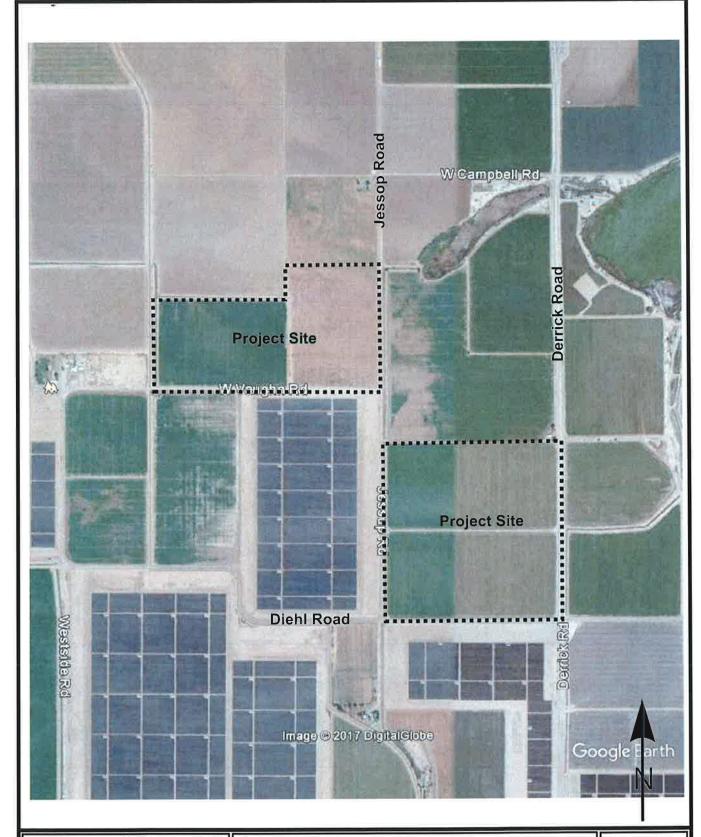
MCE_R Sa



APPENDIX A



Vicinity Map



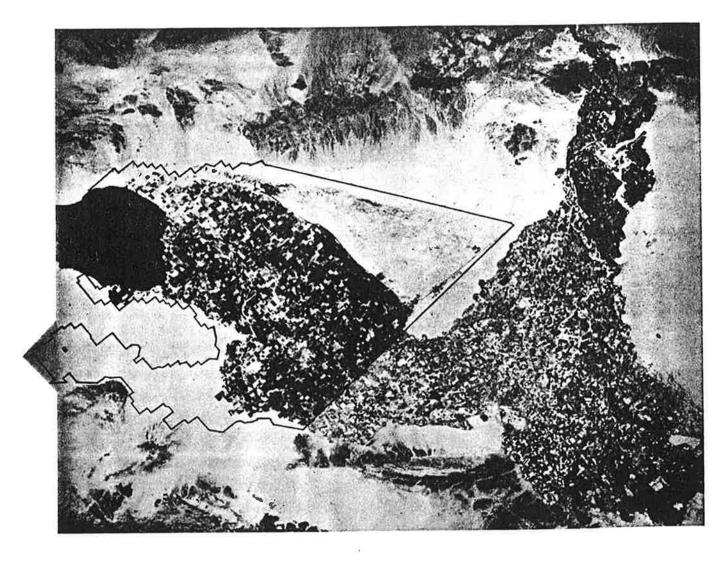
Site Map



Soil Survey Map

Soil Survey of

IMPERIAL COUNTY CALIFORNIA IMPERIAL VALLEY AREA



United States Department of Agriculture Soil Conservation Service
in cooperation with
University of California Agricultural Experiment Station
and
Imperial Irrigation District

TABLE 11.--ENGINEERING INDEX PROPERTIES

[The symbol > means more than. Absence of an entry indicates that data were not estimated]

Soil name and	Depth	USDA texture		<u>fication</u>	Frag-	P (ercentag sieve	ge pass: number		Liquid	Plas-
map symbol		 	Unified	AASHTO	> 3 inches	4	10	40	200	limit	ticity index
	In				Pct					Pot	
100 Antho		Loamy fine sand Sandy loam, fine sandy loam.		A-2, A-4	0	100 9 0-1 00		75-85 50-60		===	N P N P
101*: Antho	00	l acou fine and	CM.	A-2	0	100	100	75_85	10-30		NP
KII CIIO-23	8-60	Sandy loam, fine sand sandy loam.	SM	A-2, A-4	0	90-100	75-95	50-60	15-40		N P
Superstition		Fine sand Loamy fine sand, fine sand, sand.		A-2 A-2	0		95-100 95-100			===	N P N P
102*. Badland											
103Carsitas	110-60	Gravelly sand Gravelly sand, gravelly coarse sand, sand.	ISP, SP-S	M A-1, A-2 M A-1	0-5 0-5	60-90 60-90	50-85 50-85	30 - 55 25 - 50	0-10 0-10		N P N P
104* Fluvaquents											
105 Glenbar	113-60	Clay loam Clay loam, silty clay loam.		A-6 A-6	0	100 100		90-100 90-100		35-45 35-45	15-30 15-30
106 Glenbar	113-60	Clay loam Clay loam, silty clay loam.	CL CL	A-6, A-7 A-6, A-7	0	100 100		90-100 90-100		35-45 35-45	15-25 15-25
107* Glenbar	0-13	Loam	ML, CL-ML, CL	A – 4	0	100	100	100	70-80	20-30	NP-10
	13-60	Clay loam, silty clay loam.	50	A-6, A-7	0	100	100	95-100	75-95	35-45	15-30
108Holtville	114-22 122 - 60	Loam	ICL, CH	A – 4 A – 7 A – 4	0 0	100 100 100	100	85-100 95-100 95-100	85-95	40-65	NP-10 20-35 NP-10
109 Holtville	117-24 124-35	Silty clay Clay, silty clay Silt loam, very fine sandy	CL, CH	A-7 A-7 A-4	0 0	100 100 100	100	95-100 95-100 95-100	85-95	40-65	20-35 20-35 NP-10
	35-60	loam. Loamy very fine sand, loamy fine sand.	SM, ML	A-2, A-4	0	100	100	75-100	20-55		ΝP
110 Holtville	117-24	Silty clay Clay, silty clay Silt loam, very fine sandy	CH, CL	A-7 A-7 A-4	0 0	100 100 100		95-100 195-100 195-100	185-95	40-65 40-65 25-35	20-35 20-35 NP-10
	35-60	loam. Loamy very fine sand, loamy fine sand.	SM, ML	A-2, A-4	0	100	100	75-100	20-55		ΝP

See footnote at end of table.

TABLE 11.--ENGINEERING INDEX PROPERTIES--Continued

Soil name and	Depth	IIGDA	A texture	Class	sific	ation		rag- ents			e passi umber		Liquid	Plas-
map symbol	Depth	USDK	texture	Uniți	ed	AASHTO	0	> 3	4	10	40	200	limit	ticity index
	<u>In</u>				-1			Pet					Pot	
111*: Holtville	10 - 22 22-60	Clay,	silty clay oam, very	CL, CH	- 17	A - 7 A - 7 A - 4		0 0	100 100 100	100	95-100 95-100 95-100	85-95	40-65 40-65 25-35	20-35 20-35 NP-10
Imperial	0-12 12-60	Silty Silty silty clay.	clay loam, clay,	CH CL		A – 7 A – 7		0	100	100 100		85-95 85-95	40-50 50-70	10-20 25-45
112 Imperial	12-60	Silty Silty silty clay.	clay loam,	СН СН		A-7 A-7		0	100 100	100 100	100	85-95 85 - 95	50-70 50-70	25-45 25-45
113 Imperial	¦12-60	Silty Silty clay, clay	silty	СН СН		A-7 A-7		0	100 100	100	100	85-95 85-95	50-70 50-70	25-45 25-45
114Imperial	0-12 12-60	Silty Silty silty clay.		СН СН		A-7 A-7		0	100 100	100 100	100 100	85-95 85-95	50-70 50-70	25-45 25-45
115*: Imperial	0-12 12-60	Silty Silty silty clay.	clay loam, clay,	CH		A-7 A-7		0	100 100	100	100 100	85-95 85-95	40-50 50-70	10-20 25-45
Glenbar	0-13 13-60	Silty Clay 1 clay	oam, silty	CL		A-6, A A-6, A		0	100 100		90-100 90-100			15-25 15-25
116*: Imperial	0-13 13-60	Silty	clay loam, / clay,	CL CH		A-7 A-7		0	100	100 100	100 100	85-95 85-95		10-20 25-45
Glenbar			clay loam loam, silty loam.			A-6, A	A-7	0	100	100 100	90-100 90-100		35-45 35-45	15-25 15-30
117, 118 Indio	112-72	Strat:	ified loamy fine sand ilt loam.	r¦ML		A-4 A-4		0	95-100 95-100	95-100 95-100	85-100 85-100	75-90 75-90	20-30 20-30	NP-5 NP-5
119*: Indio		Strat: very	ified loamy fine sand ilt loam.	/ ML		A – 4 A – 4		0	95-100 195-100	95-100 95-100	85-100 85-100	75-90 75-90	20-30 20-30	
Vint		Loamy	y fine	SM SM		A-2 A-2		0	95-100 95-100	195-100	70-80	20-30	==	NP NP
120* Laveen	0=1; 12=60	Loam,	very fine y loam.	- ML, C	L-ML L-ML	A-4 A-4		0	100 195-100	95-100 85-95	75-85 70-80	55-65 55-65	20-30 15-25	NP-10 NP-10

See footnote at end of table.

TABLE 11. -- ENGINEERING INDEX PROPERTIES -- Continued

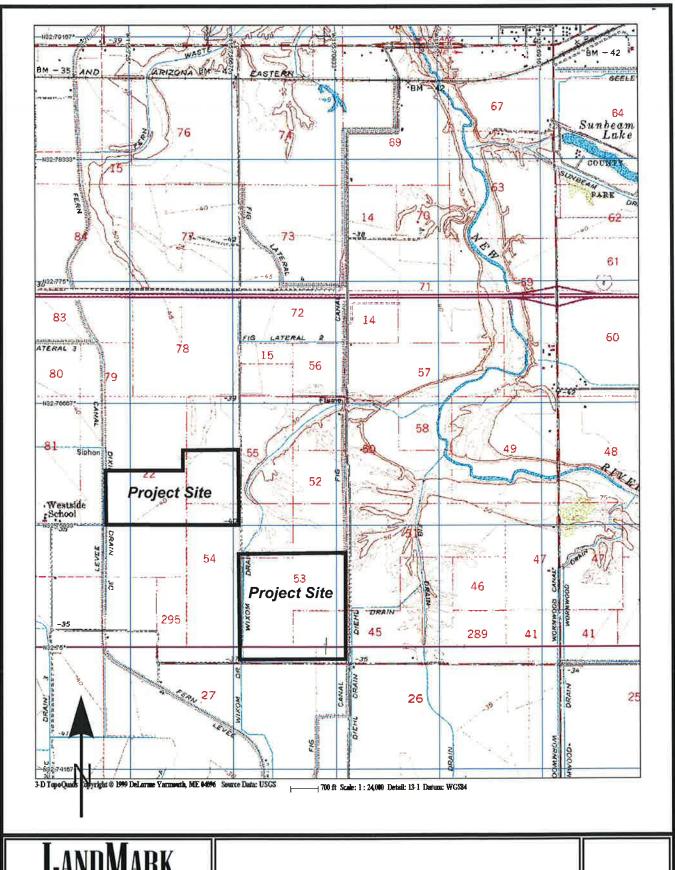
Soil name and	Depth	USDA texture		assif	catio		Frag- ments	Pe	rcentag sieve n	e passi umber		Liquid	Plas-
map symbol	pahen	ODDR CEXCUITS	Uni	fied	AASI	OTE	> 3 inches	ц	10	40	200	limit	ticity index
	In						Pot					Pot	
121 Meloland		Fine sand Stratified loamy fine sand to			A-2, A-4	A-3	0	95 - 100 100	90-100 100			25 - 35	NP NP-10
	26-71	silt loam. Clay, silty clay, silty clay loam.	CL,	сн	A-7		0	100	100	95-100	85-95	40-65	20-40
122	0-12	Very fine sandy	ML		A-4		0	95-100	95-100	95-100	55 - 85	25-35	NP-10
Meloland		loam. Stratified loamy fine sand to	ML		A-4		0	100	100	90-100	50-70	25-35	NP-10
	26-71	silt loam. Clay, silty clay, silty clay loam.	сн,	CL	A-7		0	100	100	95-100	85-95	40-65	20-40
123*:	0.12	Loam	i I M T		A-4		0	 95-100	95-100	95-100	 55-85	25-35	NP-10
Meloland		Stratified loamy fine sand to silt loam.			A-4		0	100				25 - 35	NP-10
	26-38	Clay, silty clay, silty	СН	CL	A-7		0	100	100	95-100	85-95	40-65	20-40
	38-60	clay loam. Stratified silt loam to loamy fine sand.	SM,	ML	A – 4		0	100	100	75-100	35-55	25-35	NP-10
Holtville	12-24 24-36	Loam Clay, silty clay Silt loam, very fine sandy	CH,	CL	A-4 A-7 A-4		0 0	100 100 100	100	85-100 95-100 95-100	85-95	25-35 40-65 25-35	NP-10 20-35 NP-10
	36-60	loam. Loamy very fine sand, loamy fine sand.	SM,	ML	A-2,	A – 4	0	100	100	75-100	20-55		ЯP
124, 125 Niland		Gravelly sand Silty clay, clay, clay loam.	SM, CL,	SP-SM CH	A-2, A-7	A-3	0	90-100				40-65	ИР 20-40
126Niland	0-23	Fine sand Silty clay	SM, CL,	SP-SM CH	A-2, A-7	A – 3	0	100	90-100 100			40-65	NP 20-40
127 Niland		Loamy fine sand Silty clay		СН	A-2 A-7		0	90-100	90-100 100			40-65	NP 20-40
128*: Niland		Gravelly sand Silty clay, clay, clay loam.	SM, CL,		A-2, A-7	A-3	0 0	90-100			5-25 80-100		NP 20-40
Imperial		Silty clay Silty clay loam, silty clay, clay.			A-7 A-7		0	100	100 100	100	85-95 85-95	50 - 70 50 - 70	25-45 25-45
129*: Pits										1	 	1 1 1	
130, 131 Rositas	0-27	Sand	SP-	SM	A-3 A-1 A-2	,	0	100	80-100	40-70 	5-15		NP
	27-60	Sand, fine sand, loamy sand.	SM,	SP-SM		2,	0	100	80-100	40-85 	5-30		NP

See footnote at end of table.

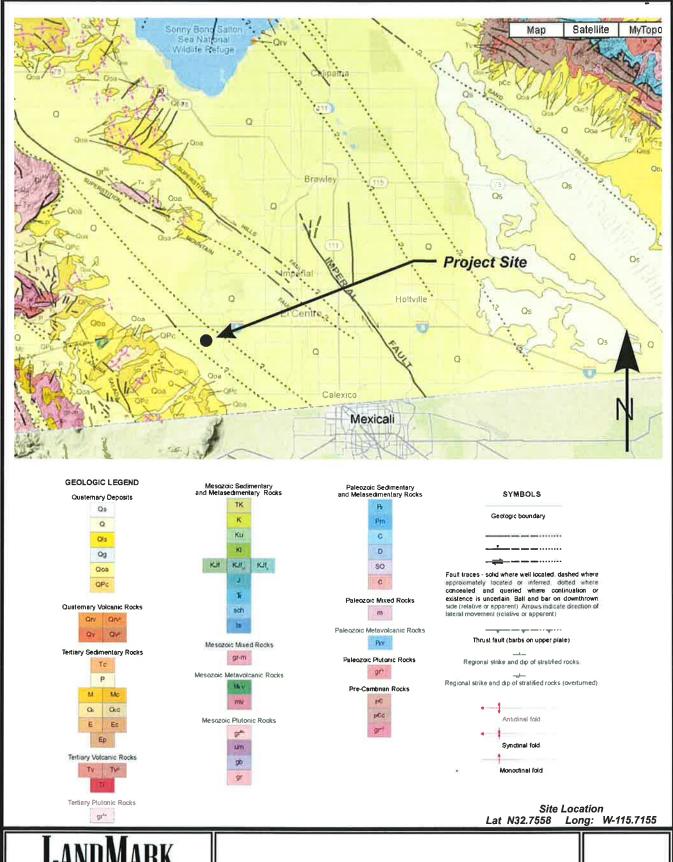
TABLE 11.--ENGINEERING INDEX PROPERTIES--Continued

Soil name and	Depth	USDA texture		ication	Frag- ments	P	ercenta sieve	ge pass number-		Liquid	Plas-
map symbol	ļ.,		Unified	AASHTO	> 3 inches	4	10	40	200	limit	ticity index
120 122 124 125	In	Edua4			Pet					Pot	
132, 133, 134, 135- Rositas	1	1	i	A-3, A-2	0	100	1	50-80 	1		NP
	9-60	Sand, fine sand, loamy sand.	ISM, SP-SM	A-3, A-2, A-1	0	100	80-100	140-85	5-30		NP
136Rositas	0-4 4-60	Loamy fine sand Sand, fine sand, loamy sand.	SM SM, SP-SM	A-1, A-2 A-3, A-2, A-1	0		80-100 80-100			==	NP NP
137Rositas	0-12 12-60	Silt loam Sand, fine sand, loamy sand.	SM, SP-SM	A-4 A-3, A-2, A-1	0 0		100 180-100			20 – 30	NP-5 NP
138*:											
Rositas	1 0-4 4-60 	Sand Sand Sand Sand Sand Sand Ioamy sand Ioamy sand Sa	ISM SM, SP-SM 	A-1, A-2 A-3, A-2, A-1	0		80-100 80-100				NP NP
Superstition		Loamy fine sand Loamy fine sand, fine sand, sand.		A-2 A-2	0		 95-100 95-100				NP NP
139 Superstition	6-60	Loamy fine sand Loamy fine sand, fine sand, sand.		A-2 A-2	0 0		 95-100 95-100 				NP NP
140*: Torriorthents											
Rock outcrop							İ	 			
141*; Torriorthents											
Orthids											
142Vint	0-10	Loamy very fine	SM, ML	A-4	0	100	100	85-95	40-65	15-25	NP-5
		Loamy fine sand	SM	A-2	0	95- 100	95-100	70-80	20-30		NP
143Vint	0-12	Fine sandy loam	ML, CL-ML, SM,	A-4	0	100	100	75→85	45 - 55	15-25	NP-5
	12-60	Loamy sand, loamy fine sand.	SM-SC	A-2	0	95-100	95-100	70-80	20-30		NР
144#: Vint	0_10	Very fine sandy	CM MT	A-4	0	100	100	05.05	no 65	15 25	NO 5
İ	- 1	loam.		i		100	1	85-95		15-25	NP=5
		Loamy fine sand Silty clay		A-2 A-7	0		95~100 100			40-65	NP 20-35
Indio	0-12	Very fine sandy	ML	A-4	0	95-100	95-100	85-100	75-90	20-30	NP-5
	1	loam. Stratified loamy very fine sand	ML	A-4	0	95-100	95-100	85-100	75-90	20-30	NP-5
		to silt loam. Silty clay	CL, CH	A-7	0	100	100	95-100	85-95	40-65	20-35

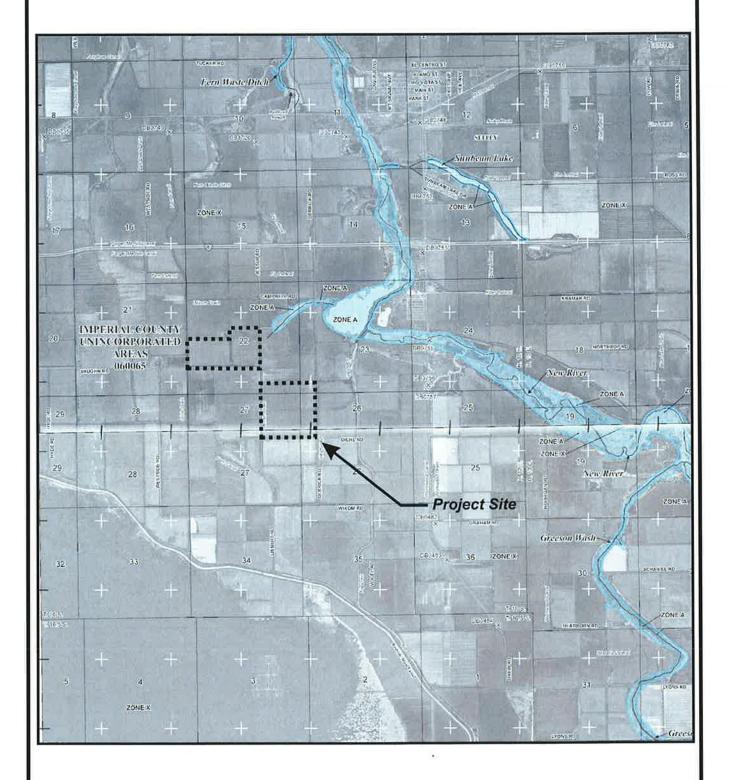
^{*} See description of the map unit for composition and behavior characteristics of the map unit.



Topographic Map



Regional Geologic Map



Reference: Federal Emergency Management Agency (FEMA) Imperial County-Panel Numbers 06025C1770C and 06025C2050C



Flood Insurance Rate Map (FIRM)

LEGEND



SPECIAL FLOOD HAZARD AREAS SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD

The 1% annual flood (100-year flood), also known as the base flood, is the flood that has a 1% chance of being equaled or exceeded in any given year. The Special Flood Hazard Area is the area subject to flooding by the 1% annual chance flood. Areas of Special Flood Hazard include Zones A, AE, AH, AO, AR, A99, V, and VE. The Base Flood Elevation is the water-surface elevation of the 1% annual chance flood.

ZONE A	No Base Flood Elevations determined.
ZONE AE	Base Flood Elevations determined.
ZONE AH	Flood depths of 1 to 3 feet (usually areas of ponding); Base Flood Elevations determined.
ZONE AO	Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities also determined.
ZONE AR	Special Flood Hazard Area formerly protected from the 1% annual chance flood by a flood control system that was subsequently decertified. Zone AR indicates that the former flood control system is being restored to provide protection from the 1% annual chance or greater flood.
ZONE A99	Area to be protected from 1% annual chance flood by a Federal flood protection system under construction; no Base Flood Elevations determined.
ZONE V	Coastal flood zone with velocity hazard (wave action); no Base Flood Elevations determined.
ZONE VÉ	Coastal flood zone with velocity hazard (wave action); Base Flood Elevations determined,
Control of the Contro	

FLOODWAY AREAS IN ZONE AE

The floodway is the channel of a stream plus any adjacent floodplain areas that must be kept free of encroachment so that the 1% annual chance flood can be carried without substantial increases in flood heights.



OTHER FLOOD AREAS

ZONE X

Areas of 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood.



OTHER AREAS

Areas determined to be outside the 0.2% annual chance floodplain.

ZONE D

Areas in which flood hazards are undetermined, but possible.



COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS

OTHERWISE PROTECTED AREAS (OPAs)

CBRS areas and OPAs are normal	ally located within or adjacent to Special Flood Hazard Areas.
	1% annual chance floodplain boundary
:	0.2% annual chance floodplain boundary
	Floodway boundary
	Zone D boundary
	CBRS and OPA boundary
	Boundary dividing Special Flood Hazard Area Zones and boundary dividing Special Flood Hazard Areas of different Base Flood Elevations, flood depths or flood velocities.
~~~ 513 ~~~	Base Flood Elevation line and value; elevation in feet®
(EL 987)	Base Flood Elevation value where uniform within zone; elevation in feet $\ensuremath{^{\circ}}$
* Peferenced to the North Ameri	can Vertical Dahum of 1986

●M1.5

(A)——(A)	Cross section line								
<u>a</u>	Transect line								
87°07'45", 32°22'30"	Geographic coordinates referenced to the North American Datum of 1983 (NAD 83), Western Hemisphere								
²⁴ 76 ^{∞∞⊸} N	1000-meter Universal Transverse Mercator grid values, zone 11N								
600000 FT	5000-foot grid ticks: California State Plane coordinate system, zone VI (FIPSZONE 0406), Lambert Conformal Conic projection								
DX5510 X	Bench mark (see explanation in Notes to Users section of this FIRM panel)								

River Mile

# **APPENDIX B**

# REFERENCES

- California Building Standards Commission, 2017, 2016 California Building Code. California Code of Regulations, Title 24, Part 2, Vol. 2 of 2.
- California Division of Mines and Geology (CDMG), 1996, California Fault Parameters: available at http://www.consrv.ca.gov/dmg/shezp/fltindex.html.
- California Geological Survey (CGS), 2016, Fault Activity Map of California <a href="http://www.quake.ca.gov/gmaps/FAM/faultactivitymap.html#">http://www.quake.ca.gov/gmaps/FAM/faultactivitymap.html#</a>.
- California Geological Survey (CGS), 2016, Alquist-Priolo Earthquake Fault Zone Maps. <a href="http://maps.conservation.ca.gov/cgs/informationwarehouse/index.html?map=regulatorymaps">http://maps.conservation.ca.gov/cgs/informationwarehouse/index.html?map=regulatorymaps</a>
- Cetin, K. O., Seed, R. B., Der Kiureghian, A., Tokimatsu, K., Harder, L. F., Jr., Kayen, R. E., and Moss, R. E. S., 2004, Standard penetration test-based probabilistic and deterministic assessment of seismic soil liquefaction potential: ASCE JGGE, Vol., 130, No. 12, p. 1314-1340.
- Geologismiki, 2014, CLiq Computer Program, www.geologismiki.gr
- Jones, A. L., 2003, An Analytical Model and Application for Ground Surface Effects from Liquefaction, PhD. Dissertation, University of Washington, 362 p.
- McCrink, T. P., Pridmore, C. L., Tinsley, J. C., Sickler, R. R., Brandenberg, S. J., and Stewart, J. P., 2011, Liquefaction and Other Ground Failures in Imperial County, California, from the April 4, 2010, El Mayor—Cucapah Earthquake, CGS Special Report 220, USGS Open File Report 2011-1071, 84 p.
- Morton, P. K., 1977, Geology and mineral resources of Imperial County, California: California Division of Mines and Geology, County Report No. 7, 104 p.
- Rymer, M.J., Treiman, J.A., Kendrick, K.J., Lienkaemper, J.J., Weldon, R.J., Bilham, R., Wei, M., Fielding, E.J., Hernandez, J.L., Olson, B.P.E., Irvine, P.J., Knepprath, N., Sickler, R.R., Tong, .X., and Siem, M.E., 2011, Triggered surface slips in southern California associated with the 2010 El Mayor-Cucapah, Baja California, Mexico, earthquake: U.S. Geological Survey Open-File Report 2010-1333 and California Geological Survey Special Report 221, 62 p., available at http://pubs.usgs.gov/of/2010/1333/.
- U.S. Geological Survey (USGS), 1990, The San Andreas Fault System, California, Professional Paper 1515.
- U.S. Geological Survey (USGS), 2016, US Seismic Design Maps Web Application, available at http://geohazards.usgs.gov/designmaps/us/application.php

- Youd, T. L., 2005, Liquefaction-induced flow, lateral spread, and ground oscillation, GSA Abstracts with Programs, Vol. 37, No. 7, p. 252.
- Youd, T. L. and Garris, C. T., 1995, Liquefaction induced ground surface disruption: ASCE Geotechnical Journal, Vol. 121, No. 11.
- Zimmerman, R. P., 1981, Soil survey of Imperial County, California, Imperial Valley Area: U.S. Dept. of Agriculture Soil Conservation Service, 112 p.

# **Preliminary Geotechnical and GeoHazards Report**

# Laurel 3 Solar Site SEC Liebert Road and Wixom Road

El Centro, California

Prepared for:

90FI 8ME, LLC 211 Sutter Street, 6th Floor San Francisco, CA 94108





Prepared by:

Landmark Consultants, Inc. 780 N. 4th Street El Centro, CA 92243 (760) 337-1100

August 2017



August 31, 2017

780 N. 4th Street El Centro, CA 92243 (760) 370-3000 (760) 337-8900 fax

77-948 Wildcat Drive Palm Desert, CA 92211 (760) 360-0665 (760) 360-0521 fax

Mr. Daniel Kolta 90FI 8ME, LLC 211 Sutter Street, 6th Floor San Francisco, CA 94108

# Preliminary Geological and Geotechnical Hazard Evaluation Laurel 3 Solar Site North and South of Vaughn Road at Westside Road El Centro, California LCI Project No. LE17142

Dear Mr. Kolta:

This preliminary geotechnical report and geologic hazards study is provided for preliminary site evaluation and permitting of the photo-voltaic solar farm at the approximately 587-acre project area (APNs 051-330-001, a portion of 051-270-027, 051-270-047, a portion of 051-300-030, 051-300-039, 051-051-300-008, and 051-300-009) located both north and south of Vaughn Road and east and west of Westside Road approximately 8 miles west of El Centro, California.

# Scope of Work

The scope of work consisted of a geologic and geotechnical hazards evaluation of the project site which addresses the following items:

- 1. General site geology.
- 2. Site location in relation to mapped earthquake faults and seismic zones.
- 3. Intensity of ground shaking at the site.
- 4. Potential for liquefaction, ground failure, and landslides at the site. No drilling was conducted to determine potential for liquefaction settlement or soil analysis.
- 5. Soil corrosivity.
- 6. Plant growth suitability of the site soils.
- 7. Preliminary pavement structural sections.
- 8. Potential for flooding at the site from manmade facilities (dams, canals, etc.) and from natural storms.
- 9. Other potential geologic and geotechnical hazards.

# **Site Description**

The project site is located both north and south of Vaughn Road and east and west of Westside Road approximately 8 miles west of El Centro, California. The project site consists of approximately 587-acres comprised of two separate parcels each consisting of two (2) agricultural fields currently in crop production.

<u>Parcel 1:</u> Parcel 1 is comprised of three (3) agricultural fields totaling approximately 160 acres. The parcel is roughly square in plan view. The parcel is bounded on the south by Diehl Road, the north by Vaughn Road, and the west by Westside Road. A solar farm forms the eastern boundary of the site. A dirt field road and irrigation canal bisects the site in a north-south direction and the western portion of the site in an east-west direction.

<u>Parcel 2</u>: Parcel 2 is comprised of five (5) agricultural fields totaling approximately 427 acres. The parcel is irregular in plan view and elongate in the north-south direction. Vaughn Road bisects the parcel. Westside Road forms the eastern boundary of the northern portion of the site. Dirt field roads cross the site at the margins of the fields. The Dixie Drain, an open irrigation drainage ditch, forms a portion of the eastern margin of the northern portion of Parcel 2 and crosses through the north portion of the site. The Westside Main Canal forms the southern boundary of the site.

Agricultural fields are located around the perimeter of the project site. Dirt field roads are located along the margins and also cross the parcels. The adjacent properties are approximately the same elevation as the project sites.

# **Site Geological Conditions**

Site Geology: The project site is located in the Imperial Valley portion of the Salton Trough physiographic province. The Salton Trough is a topographic and geologic structural depression resulting from large scale regional faulting. The trough is bounded on the northeast by the San Andreas Fault and Chocolate Mountains and the southwest by the Peninsular Range and faults of the San Jacinto Fault Zone. The Salton Trough represents the northward extension of the Gulf of California, containing both marine and non-marine sediments since the Miocene Epoch. Tectonic activity that formed the trough continues at a high rate as evidenced by deformed young sedimentary deposits and high levels of seismicity. Figure 1 shows the location of the site in relation to regional faults and physiographic features.

The Imperial Valley is directly underlain by lacustrine deposits, which consist of interbedded lenticular and tabular silt, sand, and clay. Based on Unified Soil Classification System, the permeability of these soils is expected to be low to moderate. The Late Pleistocene to Holocene lake deposits are probably less than 100 feet thick and derived from periodic flooding of the Colorado River which intermittently formed a fresh water lake (Lake Cahuilla).

Older deposits consist of Miocene to Pleistocene non-marine and marine sediments deposited during intrusions of the Gulf of California. Basement rock consisting of Mesozoic granite and Paleozoic metamorphic rocks are estimated to exist at depths between 15,000 - 20,000 feet.

Groundwater: The groundwater in the site area is brackish and typically encountered at a depth of between 5 to 10 feet below ground surface in the vicinity of the project site. There is uncertainty in the accuracy of short-term water level measurements, particularly in fine-grained soil. Groundwater levels may fluctuate with water elevation in the Westside Main Canal, precipitation, irrigation of adjacent properties, drainage, and site grading. The groundwater level noted should not be interpreted to represent an accurate or permanent condition.

Onsite Wastewater Disposal: The near surface soils at the project site generally consist of silts and silty sands having a moderate infiltration rate. The near surface soils are considered good in supporting onsite septic systems and leach fields for wastewater disposal. Site specific studies will be required to determine that County Environmental Health standards are met in regard to soil percolation rates and separation of leach fields from groundwater.

# Geological Hazards

**Landsliding:** No ancient landslides are shown on geologic maps of the region and no indications of landslides were observed during our site investigation. The hazard of landsliding is unlikely due to the relatively planar topography of the project site.

*Volcanic hazards:* The site is not located proximal to any known volcanically active area and the risk of volcanic hazards is considered very low.

**Tsunamis, seiches, and flooding:** The site does not lie near any large bodies of water, so the threat of tsunami, seiches, or other seismically-induced flooding is considered unlikely. The project site is located in FEMA Flood Zone X, an area determined to be outside the 0.2% annual chance floodplain (FIRM Panels 06025C2050C and 06025C1700C).

**Expansive soil:** In general, much of the near surface soils within the project site consist of silty clays and clay having a moderate to high expansion potential. A site specific geotechnical investigation will be required at this site to determine the extent and effect of expansive soils.

Corrosive Soils: The lacustrine site soils (lake bed deposits) are known to be corrosive. Typical remediation for the corrosive soil conditions consists of using concrete mixed with higher cement contents (6 sacks Type V Portland Cement) and low water-cement ratios (0.45 w/c ratio). Additionally, steel post corrosion protection is required, consisting of zinc coatings (galvanizing) or increased structural sections to compensate for metal loss due to corrosion.

Liquefaction/Seismic Settlements: Liquefaction is a potential design consideration because of possible saturated sandy substrata underlying the site. Liquefaction occurs when granular soil below the water table is subjected to vibratory motions, such as produced by earthquakes. With strong ground shaking, an increase in pore water pressure develops as the soil tends to reduce in volume. If the increase in pore water pressure is sufficient to reduce the vertical effective stress (suspending the soil particles in water), the soil strength decreases and the soil behaves as a liquid (similar to quicksand). Liquefaction can produce excessive settlement, ground rupture, lateral spreading, or failure of shallow bearing foundations.

Four conditions are generally required for liquefaction to occur:

- (1) the soil must be saturated (relatively shallow groundwater);
- (2) the soil must be loosely packed (low to medium relative density);
- (3) the soil must be relatively cohesionless (not clayey); and
- (4) groundshaking of sufficient intensity must occur to function as a trigger mechanism.

All of these conditions may exist to some degree at this site. Liquefaction settlement and ground fissures were noted along the Westside Main Canal in the area of the project site after the April 4, 2010 magnitude 7.2M_w El Mayor-Cucapah Earthquake. McCrink and others (2011) reported several liquefaction related failures to the embankment of the Westside Main Canal southwest of the project site.

# Seismic Hazards

The project site is located in the seismically active Imperial Valley of southern California with numerous mapped faults of the San Andreas Fault System traversing the region. The San Andreas Fault System is comprised of the San Andreas, San Jacinto, and Elsinore Fault Zones in southern California. The Imperial fault represents a transition from the more continuous San Andreas fault to a more nearly echelon pattern characteristic of the faults under the Gulf of California (USGS, 1990). We have performed a computer-aided search of known faults or seismic zones that lie within a 62 mile (100 kilometer) radius of the project site (Table 1).

A fault map illustrating known active faults relative to the site is presented on Figure 1, Regional Fault Map. A legend for the regional fault map is presented on Figure 2. The criterion for fault classification adopted by the California Geological Survey defines Earthquake Fault Zones along active or potentially active faults. An active fault is one that has ruptured during Holocene time (roughly within the last 11,000 years). A fault that has ruptured during the last 1.8 million years (Quaternary time), but has not been proven by direct evidence to have not moved within Holocene time is considered to be potentially active. A fault that has not moved during both Pleistocene and Holocene time (that is no movement within the last 1.8 million years) is considered to be inactive.

Review of the current Alquist-Priolo Earthquake Fault Zone maps (CGS, 2000a) indicates that the nearest mapped Earthquake Fault Zone is an unnamed fault located approximately 0.8 miles southwest of the project site. Geologic mapping by the USGS (Rymer and others, 2011) of the Imperial Valley after the April 4, 2010 magnitude 7.2M_w El Mayor-Cucapah Earthquake indicates movement along several known and unknown faults west of the project site. Surface rupture on these faults is possible from future seismic events in the area.

The nearest mapped major Earthquake Fault Zone is the Laguna Salada fault located approximately 7.7 miles southwest of the site and the Superstition Hills fault located approximately 9.6 miles northeast of the project site.

*Groundshaking*. The primary seismic hazard at the project site is the potential for strong groundshaking during earthquakes along the Superstition Hills, Imperial, Cerro Prieto, and Laguna Salada faults (Figure 2).

<u>Site Acceleration</u>: The project site is considered likely to be subjected to moderate to strong ground motion from earthquakes in the region. Ground motions are dependent primarily on the earthquake magnitude and distance to the seismogenic (rupture) zone. Accelerations also are dependent upon attenuation by rock and soil deposits, direction of rupture and type of fault; therefore, ground motions may vary considerably in the same general area.

<u>CBC General Ground Motion Parameters:</u> The 2016 CBC general ground motion parameters are based on the Risk-Targeted Maximum Considered Earthquake (MCE_R). The U.S. Geological Survey "U.S. Seismic Design Maps Web Application" (USGS, 2017) was used to obtain the site coefficients and adjusted maximum considered earthquake spectral response acceleration parameters. The site soils have been classified as Site Class D (stiff soil profile).

Design spectral response acceleration parameters are defined as the earthquake ground motions that are two-thirds (2/3) of the corresponding MCE_R ground motions. Design earthquake ground motion parameters are provided in Table 2. A Risk Category II was determined using Table 1604.5 and the Seismic Design Category is D since  $S_1$  is less than 0.75.

The Maximum Considered Earthquake Geometric Mean (MCE_G) peak ground acceleration (PGA_M) value was determined from the "U.S. Seismic Design Maps Web Application" (USGS, 2017) for liquefaction and seismic settlement analysis in accordance with 2016 CBC Section 1803.5.12 and CGS Note 48 (PGA_M =  $F_{PGA}*PGA$ ). A PGA_M value of 0.50g has been determined for this project site.

Surface Rupture: The project site does not lie within a State of California, Alquist-Priolo Earthquake Fault Zone. Surface fault rupture at the project site is considered to be low. The nearest mapped earthquake fault zone is located approximately 0.8 miles southwest of the project site. This is an unnamed fault that was mapped after the 2010 7.2M_w El Mayor-Cucapah Earthquake.

# Other Hazards

*Hazardous Materials:* The site is not located in proximity to any known hazardous materials (methane gas, tar seeps, hydrogen sulfide gas), and the risk of hazardous materials is considered very low.

**Radon 222 Gas:** Radon gas is not believed to be a potential hazard at the site. A report titled "California Statewide Radon Survey-Screening Results", dated November 1990 and published by the California State Department of Health Services, notes that Southern California showed a low risk of elevated radon levels, based on 2-day tests conducted from January through April 1990. Some of the reported testing was performed in Imperial County; however, no data was observed as being at or near the project site.

*Naturally occurring asbestos:* The site is not located in proximity to any known naturally occurring asbestos, and the risk of naturally occurring asbestos is considered very low.

*Hydrocollapse:* The site is dominantly underlain by clays that are not expected to collapse with the addition of water to the site. The risk of hydrocollapse is considered very low.

**Regional Subsidence:** Regional subsidence due to geothermal resource activities has not been documented in the area west of the New River; therefore, the risk of regional subsidence is considered low.

# Conclusion

This preliminary report was prepared according to the generally accepted *geotechnical* engineering standards of practice that existed in Imperial County at the time the report was prepared. No express or implied warranties are made in connection with our services.

Our research did not reveal conditions that would preclude implementation of the proposed project provided site specific geotechnical investigations are conducted prior to site development to provide geotechnical criteria for the design and construction of this project.

We appreciate the opportunity to provide our findings and professional opinions regarding geologic and geotechnical hazards at the site. If you have any questions or comments regarding our findings, please call our office at (760) 370-3000.

CERTIFIED ENGINEERING GEOLOGIST

CEG 2261

No. 31921 EXPIRES 12-31-18

Sincerely Yours;

Landmark Consultants, Inc.

Steven K. Williams, PG, CEG Senior Engineering Geologist

Jeffrey O. Lyon, PE

President

Table 1
Summary of Characteristics of Closest Known Active Faults

Fault Name	Approximate Distance (miles)	Approximate Distance (km)	Maximum Moment Magnitude (Mw)	Fault Length (km)	Slip Rate (mm/yr)
Unnamed 1*	0.8	1.3			
Yuha*	2.7	4.3			
Shell Beds	5.2	8.3			
Unnamed 2*	5.6	9.0	,		
Yuha Well *	5.7	9.2			
Laguna Salada	7.7	12.4	7	67 ± 7	$3.5 \pm 1.5$
Vista de Anza*	8.7	14.0			
Superstition Hills	9.6	15.4	6.6	23 ± 2	4 ± 2
Superstition Mountain	10.3	16.5	6.6	24 ± 2	5 ± 3
Painted Gorge Wash*	12.3	19.7			
Ocotillo*	13.3	21.3			
Borrego (Mexico)*	13.8	22.1			
Imperial	15.2	24.3	7	62 ± 6	20 ± 5
Brawley *	16.7	26.7			
Elsinore - Coyote Mountain	17.0	27.3	6.8	39 ± 4	4 ± 2
Rico *	20.2	32.3			
Elmore Ranch	20.8	33.3	6.6	29 ± 3	1 ± 0.5
Pescadores (Mexico)*	21.8	34.9			
Cerro Prieto *	23.6	37.7			
San Jacinto - Borrego	23.8	38.1	6.6	29 ± 3	4 ± 2
Cucapah (Mexico)*	24.3	38.9			
San Jacinto - Anza	41.8	66.9	7.2	91 ± 9	12 ± 6

^{*} Note: Faults not included in CGS database.

Regional Fault Map

Figure 1





Source: California Geological Survey 2010 Fault Activity Map of California http://www.quake.ca.gov/gmaps/FAM/faultactivitymap.html#

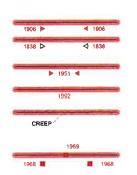
# **EXPLANATION**

Fault traces on land are indicated by solid lines where well located, by dashed lines where approximately located or inferred, and by dotted lines where concealed by younger rocks or by lakes or bays. Fault traces are queried where continuation or existence is uncertain. Concealed faults in the Great Valley are based on maps of selected subsurface horizons, so locations shown are approximate and may indicate structural trend only. All offshore faults based on seismic reflection profile records are shown as solid lines where well defined, dashed where inferred, queried where uncertain.

# FAULT CLASSIFICATION COLOR CODE (Indicating Recency of Movement)

Fault along which historic (last 200 years) displacement has occurred and is associated with one or more of the following:

- (a) a recorded earthquake with surface rupture. (Also included are some well-defined surface breaks caused by ground shaking during earthquakes, e.g. extensive ground breakage, not on the White Wolf fault, caused by the Arvin-Tehachapi earthquake of 1952). The date of the associated earthquake is indicated. Where repeated surface ruptures on the same fault have occurred, only the date of the latest movement may be indicated, especially if earlier reports are not well documented as to location of ground breaks.
- (b) fault creep slippage slow ground displacement usually without accompanying earthquakes.
- (c) displaced survey lines.



A triangle to the right or left of the date indicates termination point of observed surface displacement. Solid red triangle indicates known location of rupture termination point. Open black triangle indicates uncertain or estimated location of rupture termination point.

Date bracketed by triangles indicates local fault break.

No triangle by date indicates an intermediate point along fault break.

Fault that exhibits fault creep slippage. Hachures indicate linear extent of fault creep. Annotation (creep with leader) indicates representative locations where fault creep has been observed and recorded.

Square on fault indicates where fault creep slippage has occured that has been triggered by an earthquake on some other fault. Date of causative earthquake indicated. Squares to right and left of date indicate terminal points between which triggered creep slippage has occurred (creep either continuous or intermittent between these end points).

_____

Holocene fault displacement (during past 11,700 years) without historic record. Geomorphic evidence for Holocene faulting includes sag ponds, scarps showing little erosion, or the following features in Holocene age deposits: offset stream courses, linear scarps, shutter ridges, and triangular faceted spurs. Recency of faulting offshore is based on the interpreted age of the youngest strata displaced by faulting.

______

Late Quaternary fault displacement (during past 700,000 years). Geomorphic evidence similar to that described for Holocene faults except features are less distinct. Faulting may be younger, but lack of younger overlying deposits precludes more accurate age classification.

Quaternary fault (age undifferentiated). Most faults of this category show evidence of displacement sometime during the past 1.6 million years; possible exceptions are faults which displace rocks of undifferentiated Plio-Pleistocene age. Unnumbered Quaternary faults were based on Fault Map of California, 1975. See Bulletin 201, Appendix D for source data.

Pre-Quaternary fault (older that 1.6 million years) or fault without recognized Quaternary displacement. Some faults are shown in this category because the source of mapping used was of reconnaissnce nature, or was not done with the object of dating fault displacements. Faults in this category are not necessarily inactive.

LANDMARK
Geo-Engineers and Geologists
Project No.: LE17142

**Fault Map Legend** 

Figure 3a

# ADDITIONAL FAULT SYMBOLS

	bar and ball on downthown side (relative or apparent).
	Arrows along fault indicate relative or apparent direction of lateral movement.
	Arrow on fault indicates direction of dip.
<b></b>	Low angle fault (barbs on upper plate). Fault surface generally dips less than 45° but locally may have been subsequently steepened. On offshore faults, barbs simply indicate a reverse fault regardless of steepness of dip.
	OTHER SYMBOLS
	Numbers refer to annotations listed in the appendices of the accompanying report. Annotations include fault name, age of fault displacement, and pertinent references including Earthquake Fault Zone maps where a fault has been zoned by the Alquist-Prioto Earthquake Fault Zoning Act. This Act requires the State Geologist to delineate zones to encompass faults with Holocene displacement.
	Structural discontinuity (offshore) separating differing Neogene structural domains. May indicate discontinuities between basement rocks,
	Brawley Seismic Zone, a linear zone of seismicity locally up to 10 km wide associated with the releasing step between the Imperial and San Andreas faults.

Geologic		С	Years Before	Fault	Recency	DESCRIPTION				
	Time Scale		Present (Approx.)	Symbol	of Movement	ON LAND	OFFSHORE			
	۲	Historic	200	~		Displacement during historic time (of Includes areas of known fault creep				
	Late Quaternary	Повоселе	200-		-2	Oreplacement during Holocene- tine.	Fault offsets seafoor sediments or strata of holocene age			
Quaternary	Late Q	9	11,700		-7-	Faults showing evidence of anothersment our rigilitie Quinternary time	Fault collistrate of Late Presticeme age			
Quat	Early Quatemary	Pleistocene	<del></del>		-3-	Undivided Quaternary faults most faults in this series category show excended a displacement during the last 1.600,000 years; passable acceptions are faults which displace rocks of undifferentiated Psin-Plassocietie age.	Fault cuts strata of Quaternary age			
Pre-Quaternary			— 1,600,000°—			Faults without recognized Quaternary displacement or showing evidence of no displacement during Quaternary time. Not necessarily inactive.	Fault cuts strata of Pliocene or older age.			
			4.5 billion (Age of Earth)							

^{*} Quaternary now recognized as extending to 2.6 Ma (Walker and Geissman, 2009). Quaternary faults in this map were established using the previous 1.6 Ma criterion.



Fault Map Legend

Figure 3b

Table 2
2016 California Building Code (CBC) and ASCE 7-10 Seismic Parameters

CBC Reference Table 20.3-1

Soil Site Class: **D** 

Latitude: 32.7435 N

Longitude: -115.7424 W

Risk Category: I Seismic Design Category: D

# Maximum Considered Earthquake (MCE) Ground Motion

Mapped MCE _R Short Period Spectral Response	$S_s$	1.500 g	Figure 1613.3.1(1)
Mapped MCE _R 1 second Spectral Response	$S_1$	0.600 g	Figure 1613.3.1(2)
Short Period (0.2 s) Site Coefficient	$\mathbf{F_a}$	1.00	Table 1613.3.3(1)
Long Period (1.0 s) Site Coefficient	$\mathbf{F_v}$	1.50	Table 1613.3.3(2)

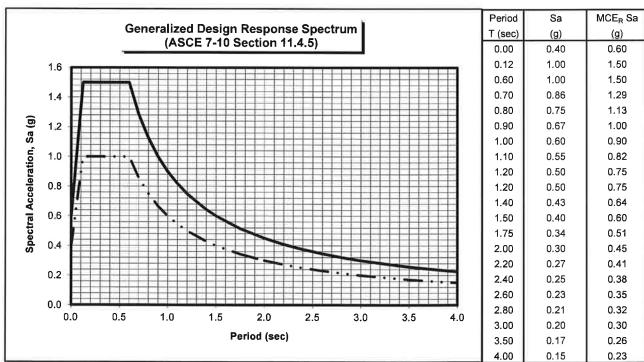
MCE_R Spectral Response Acceleration Parameter (0.2 s)  $S_{MS}$  1.500 g =  $F_a * S_s$  Equation 16-37 MCE_R Spectral Response Acceleration Parameter (1.0 s)  $S_{M1}$  0.900 g =  $F_v * S_1$  Equation 16-38

# **Design Earthquake Ground Motion**

Design Spectral Response Acceleration Parameter (0.2 s)	$S_{DS}$	$1.000 \text{ g} = 2/3 * S_{MS}$	Equation 16-39
Design Spectral Response Acceleration Parameter (1.0 s)	$S_{D1}$	$0.600 \text{ g} = 2/3 \text{*} \text{S}_{\text{M1}}$	Equation 16-40
	$T_L$	8.00 sec	ASCE Figure 22-12
	T	0.120.2*5 /5	

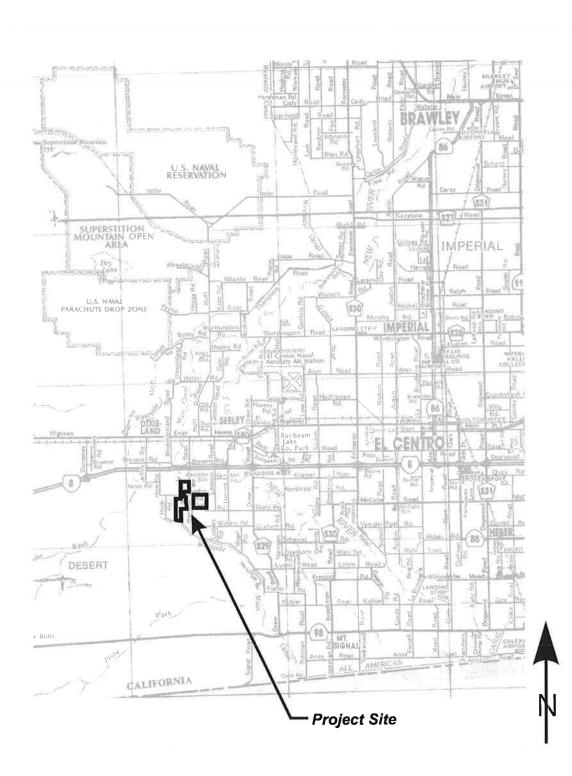
 $T_{o}$  0.12 sec =0.2* $S_{D1}/S_{DS}$  $T_{s}$  0.60 sec = $S_{D1}/S_{DS}$ 

Peak Ground Acceleration PGA_M 0.50 g ASCE Equation 11.8-1

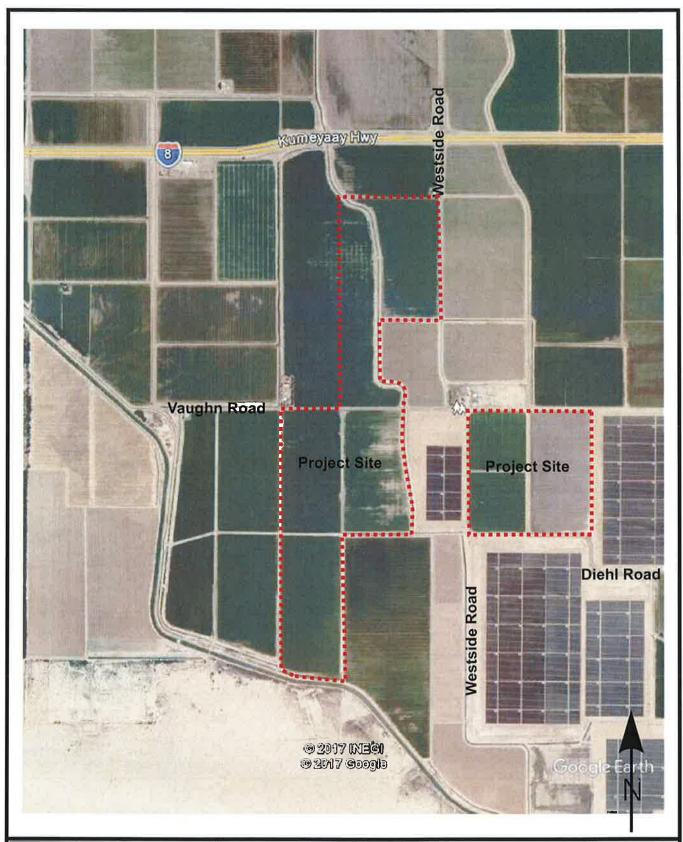


Design Response Spectra MCE_R Response Spectra

# **APPENDIX A**



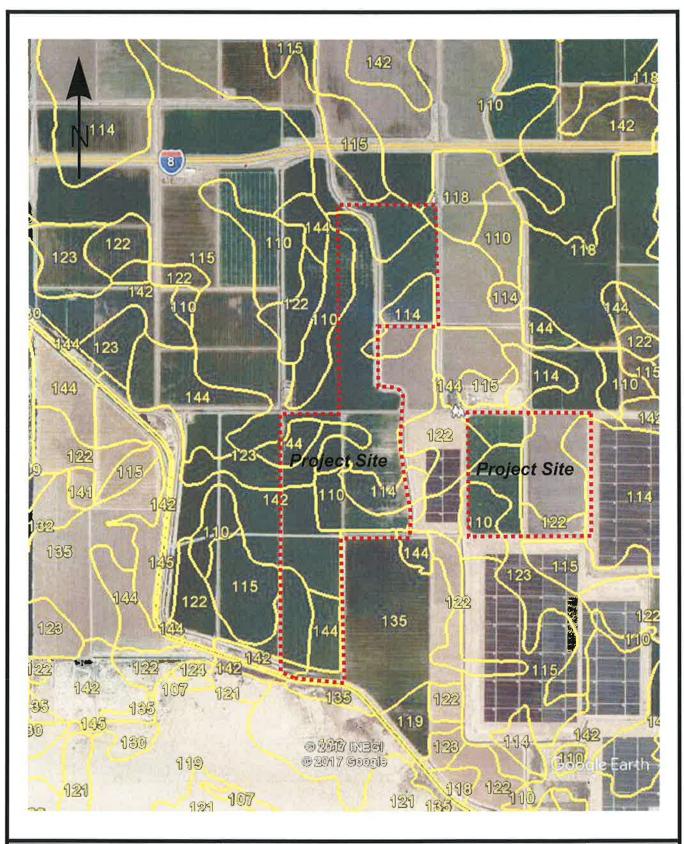
**Vicinity Map** 



LANDMARK
Geo-Engineers and Geologists

Project No.: LE17142

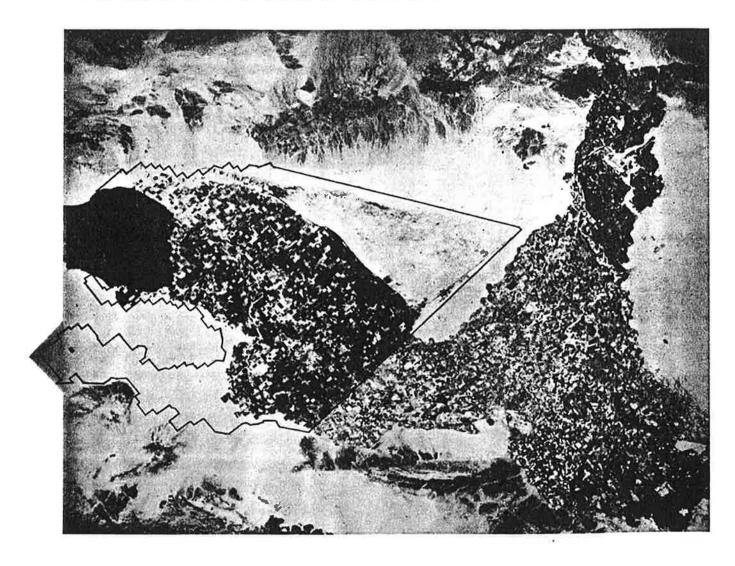
Site Map



Soil Survey Map

# Soil Survey of

# IMPERIAL COUNTY CALIFORNIA IMPERIAL VALLEY AREA



United States Department of Agriculture Soil Conservation Service
in cooperation with
University of California Agricultural Experiment Station
and
Imperial Irrigation District

TABLE 11.--ENGINEERING INDEX PROPERTIES

[The symbol > means more than. Absence of an entry indicates that data were not estimated]

Soil name and	Depth	USDA texture	Classif	I	Frag- ments	P		ge pa <b>ss</b> : number		Liquid	Plas-
map symbol			Unified		> 3  inches	4	10	40	200	limit	ticity index
	<u> In</u>				Pot					Pat	
100 Antho		Loamy fine sand Sandy loam, fine sandy loam.		A-2, A-2, A-4	0			75-85  50-60			N P N P
101#:								0-			
Antho		Loamy fine sand Sandy loam, fine sandy loam.	SM	A-2 A-2, A-4	0			75-85 50-60			N P N P
Superstition		Fine sand Loamy fine sand, fine sand, sand,		A-2 A-2	0 0			70-85 70-85		==	N P N P
102*. Badland											
103 Carsitas		Gravelly sand Gravelly sand, gravelly coarse sand, sand.	SP, SP-SM			60-90 60-90			0-10 0-10	===	N P N P
104 <b>*</b> Fluvaquents											
105 Glenbar	113-60	Clay loam Clay loam, silty clay loam.	CL CL	A-6 A-6	0 0	100 100		90-100 90-100		35-45 35-45	
106 Glenbar		Clay loam Clay loam, silty clay loam.		A-6, A-7 A-6, A-7		100 100		90-100 90-100		35-45 35-45	15-25 15-25
107 <b>*</b> Glenbar	0-13	Loam	ML, CL-ML, CL	A – 4	0	100	100	100	70-80	20-30	NP-10
	13-60	Clay loam, silty clay loam.		A-6, A-7	0	100	100	95-100	<b>75-</b> 95	35-45	15-30
108 Holtville	14-22 122-60	LoamClay, silty clay Silt loam, very fine sandy loam.	CL, CH	A = 4 A = 7 A = 4	0 0 0	100 100 100	100	85-100 95-100 95-100	85-95	25-35 40-65 25-35	NP-10 20-35 NP-10
109 Holtville	17-24 124-35	Clay, silty clay Silt loam, very fine sandy	CL, CH	A-7 A-7 A-4	0 0 0	100 100 100	100	95-100 95-100 95-100	85-95	40-65	20-35 20-35 NP-10
	W	loam. Loamy very fine sand, loamy fine sand.	SM, ML	A-2, A-4	0	100	100	75-100	20 <b>-</b> 55		ΝP
110 Holtville	117-24	Silty clay Clay, silty clay Silt loam, very fine sandy loam.	CH, CL	A-7 A-7 A-4	0 0 0	100 100 100		95-100 95-100 95-100	85-95	40-65 40-65 25-35	20-35 20-35 NP-10
	35-60	Loam, very fine sand, loamy fine sand.	SM, ML	A-2, A-4	0	100	100	75-100	20-55		ΝP

See footnote at end of table.

TABLE 11.--ENGINEERING INDEX PROPERTIES--Continued

Soil name and	Depth	USDA texture	Classification		Frag-		rcentag sieve n	Liquid	Plas-		
map symbol			Unified	AASHTO	> 3 inches	4	10	40	200	limit	ticity index
	10-22 22-60	Silty clay loam Clay, silty clay Silt loam, very fine sandy loam.	CL, CH	A-7 A-7 A-4	0 0 0 0	100 100 100	100	95-100 95-100 95-100	85-95	Pot 40-65 40-65 25-35	20-35 20-35 NP-10
Imperial	12-60	Silty clay loam Silty clay loam, silty clay, clay.	CL CH	A-7 A-7	0	100 100	100 100		85-95 85-95	40-50 50-70	10-20 25-45
112 Imperial	12 <b>-</b> 60	Silty clay Silty clay loam, silty clay, clay.	СН СН	A-7 A-7	0 0	100 100	100 100		85-95 85 <b>-</b> 95	50-70 50 <b>-</b> 70	25-45 25-45
113 Imperial	12-60	Silty clay Silty clay, clay, silty clay loam.	СН СН	A-7 A-7	0	100 100	100 100		85-95 85 <b>-</b> 95	50-70 50-70	25=45 25=45
	12-60	Silty clay Silty clay loam, silty clay, clay.		A-7 A-7	0 0	100 100	100 100	100 100	85-95 85-95	50 <b>-7</b> 0 50-70	25-45 25-45
115*: Imperial	12-60 	Silty clay loam Silty clay loam, silty clay, clay.		A-7 A-7	0	100 100	100 100	100 100	85-95 85-95	40-50 50-70	10-20 25-45
Glenbar	113-60	Silty clay loam Clay loam, silty clay loam.		A-6, A-7   A-6, A-7		100 100				35-45 35-45	15-25 15-25
116*: Imperial		Silty clay loam Silty clay loam, silty clay, clay.		A - 7   A - 7	0	100 100	100 100		85-95 85 <b>-</b> 95		10-20 25-45
Glenbar	113-60	Silty clay loam Clay loam, silty clay loam.		A-6, A-7 A-6	0	100 100		90-100 90-100	70-95 70-95	35-45 35-45	15-25 15-30
	112-72	Loam	ML	A-4 A-4		95-100 95-100					NP-5 NP-5
119*: Indio		LoamStratified loamy very fine sand to silt loam.	IML	A - 4   A - 4 	0	95-100 95-100	95-100 95-100	85-100 85-100	75-90 75-90	20-30	NP-5 NP-5
Vint		Loamy fine sand Loamy sand, loamy fine sand.	SM SM	A-2 A-2	0	95-100 95-100	95-100	70-80	20-30		NP NP
120* Laveen	0-12 12-60	Loam Loam, very fine sandy loam.	ML, CL-ML	A – 4 A – 4	0 0	100 95-100	95-100 85-95	75-85 170-80	55-65 55-65	20-30 15 <b>-</b> 25	NP-10 NP-10

See footnote at end of table.

TABLE 11. -- ENGINEERING INDEX PROPERTIES -- Continued

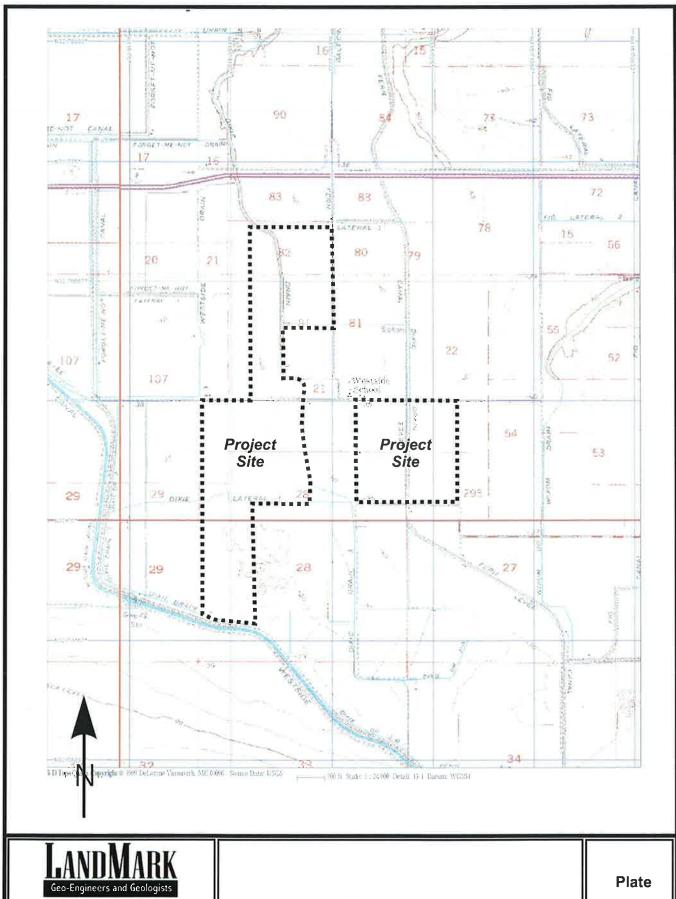
0.43	Darth	HEDA touture	Classif				Frag-	Percentage passing				Liquid	Plas-
Soil name and map symbol	Depth	USDA texture	Un:	ified	AASI	OTE	> 3			40	200	limit	ticity index
	In						inches Pot	4	10	40	200	Pet	Index
121 Meloland	0-12 12-26	Fine sand Stratified loamy fine sand to			A-2, A-4	A-3	0	95 <b>-</b> 100	90 <b>-</b> 100 100			25 <b>-</b> 35	NP NP-10
	i	silt loam.	CL,	СН	A-7		0	100	100	95-100	85 <b>-</b> 95	40-65	20-40
	0-12	Very fine sandy	ML		A-4		0	95-100	95-100	95-100	55-85	25 <b>-</b> 35	NP-10
Meloland	1	loam. Stratified loamy fine sand to	ML		A-4		0	100	100	90-100	50-70	25-35	NP-10
		silt loam. Clay, silty clay, silty clay loam.	сн,	CL	A-7		0	100	100	95-100	85-95	40-65	20-40
123*: Meloland	0 10		l MT		A-4		0	195-100	95-100	95-100	  55 <b>-</b> 85	25-35	NP-10
weloland	12-26	Stratified loamy fine sand to			A-4		Ö				50-70		NP-10
		clay, silty	сн,	CL	A-7		0	100	100	95-100	85-95	40-65	20-40
	38-60	clay loam. Stratified silt loam to loamy fine sand.	SM,	ML	A-4		0	100	100	75 <b>-</b> 100	35-55	25 <b>-</b> 35	NP-10
Holtville	112-24 24-36	Clay, silty clay Silt loam, very fine sandy	CH,	CL	A-4 A-7 A-4		0 0 0	100 100 100	100	85-100 95-100 95-100	85-95	25-35 40-65 25-35	NP-10 20-35 NP-10
	36-60	loam. Loamy very fine sand, loamy fine sand.	SM,	ML	A-2,	A = 4	0	100	100	75-100	20-55		NР
124, 125 Niland	123-60	Gravelly sand Silty clay, clay, clay loam.	SM, CL,	SP-SM CH	A-2, A-7	A-3	0	90-100	70-95 100			40-65	ИР 20-40
126 Niland	0-23 23-60	Fine sand Silty clay	SM, CL,	SP-SM CH	A-2, A-7	A - 3	0	90-100				40 <b>-</b> 65	NP 20-40
127Niland		Loamy fine sand Silty clay		СН	A-2 A-7		0	90→100 100	90 <b>-</b> 100 100			40-65	NP 20-40
128*: Niland		Gravelly sand Silty clay, clay, clay loam.			A-2, A-7	A-3	0				5-25 80-100	40-65	NP 20-40
Imperial		Silty clay Silty clay loam, silty clay, clay.			A-7 A-7		0	100	100 100	100 100	85 <b>-</b> 95 85 <b>-</b> 95	50-70 50-70	25-45 25-45
129*: Pits		İ										×	
130, 131 Rositas	0-27	Sand	SP-	-SM	A-1	,	0	100	80-100	40-70	5-15		NP
	27-60	Sand, fine sand, loamy sand.	SM,	SP-SM	A-2 A-3, A-2 A-1		0	100	80-100	40-85	5-30		N P

See footnote at end of table.

TABLE 11.--ENGINEERING INDEX PROPERTIES--Continued

Soil name and	Depth	USDA texture	Classif	1	Frag- ments	P	ercenta sieve		Liquid	Plas-	
map symbol			Unified		> 3  inches	4	10	40	200	limit	ticity index
	In				Pet					Pot	
132, 133, 134, 135- Rositas	0-9	Fine sand	ISM	A-3, A-2	0	100	80 <b>-1</b> 00	50 <b>-</b> 80	10 <b>-</b> 25 		NP
	9-60	Sand, fine sand,   loamy sand.	SM, SP-SM	A-3, A-2, A-1	0	100	80-100	40-85	5-30		N P
136 Rositas		Loamy fine sand  Sand, fine sand,   loamy sand.	SM SM, SP-SM	A-1, A-2 A-3, A-2, A-1	0		80 <b>-</b> 100 80 <b>-</b> 100				NP NP
137Rositas	0-12 12-60	Silt loam Sand, fine sand, loamy sand.	ML SM, SP-SM	A-4  A-3,   A-2,   A-1	0 0		100 80 <b>-</b> 100			20-30	NP-5 NP
138 <b>*:</b> Rositas	0-4 4-60	Loamy fine sand Sand, fine sand, loamy sand.	SM SM, SP-SM	A-1, A-2 A-3, A-2, A-1	0		80-100 80-100			===	NP NP
Superstition	0-6 6-60	Loamy fine sand Loamy fine sand, fine sand, sand.	SM SM	A-2 A-2	0		95-100 95-100				NP NP
139 Superstition	0-6 6-60	Loamy fine sand Loamy fine sand, fine sand, sand.	SM SM	A-2 A-2	0		95 <b>-1</b> 00 95 <b>-</b> 100				NP NP
140#: Torriorthents								1			
Rock outerop											
141*: Torriorthents											
Orthids											
142 Vint		Loamy very fine sand.	SM, ML	A-4	0	100	100	  85 <b>–</b> 95	40-65	15-25	NP-5
71110			SM	A-2	0	95-100	95-100	70-80	20-30		NP
143 Vint	0-12	Fine sandy loam	ML, CL-ML, SM,	A-4	0	100	100	75-85	45-55	15-25	NP-5
	12-60	Loamy sand, loamy fine sand.	SM-SC SM	A-2	0	95-100	95-100	  70-80 	20-30		NР
144*: Vint	0.10	Vanu čina	cu v		_	100	100	05.05	1	15 25	NO 5
	N [	Very fine sandy loam.	- 1	A-4	0	100		85-95 	i	15-25	NP-5
	40-60	Loamy fine sand Silty clay		A-2   A-7	0		95⊸100 100			40-65	NP 20 <del>-</del> 35
Indio	0-12	Very fine sandy	ML	A-4	0	95-100	95-100	85 <b>-1</b> 00	75-90	20-30	NP-5
	1	loam. Stratified loamy very fine sand to silt loam.	ML	A-4	0	95-100	95-100	85-100	75-90	20-30	NP-5
		Silty clay	CL, CH	A-7	0	100	100	95-100	85-95	40-65	20-35

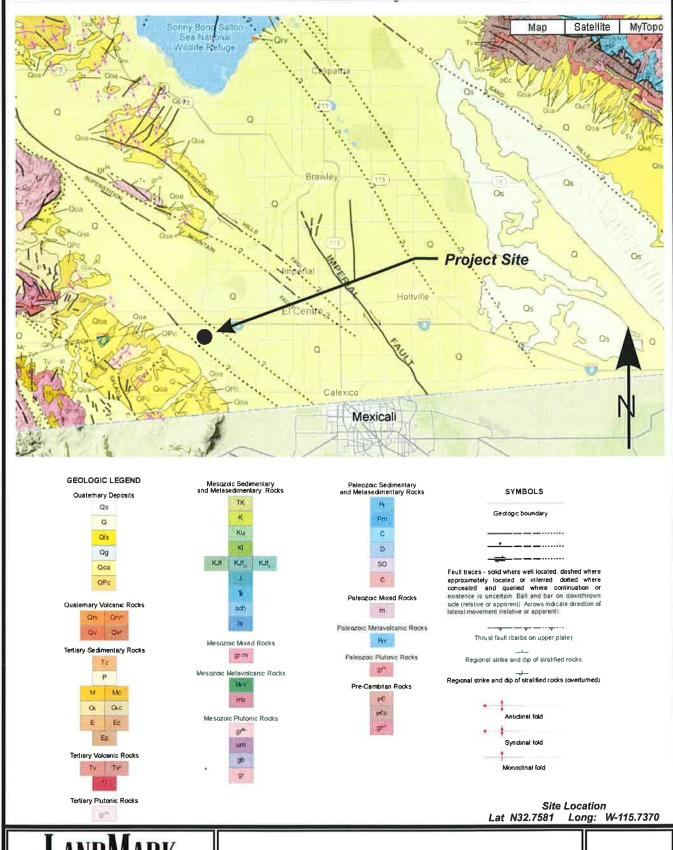
ullet See description of the map unit for composition and behavior characteristics of the map unit.



Project No.: LE17142

Topographic Map

**A-4** 

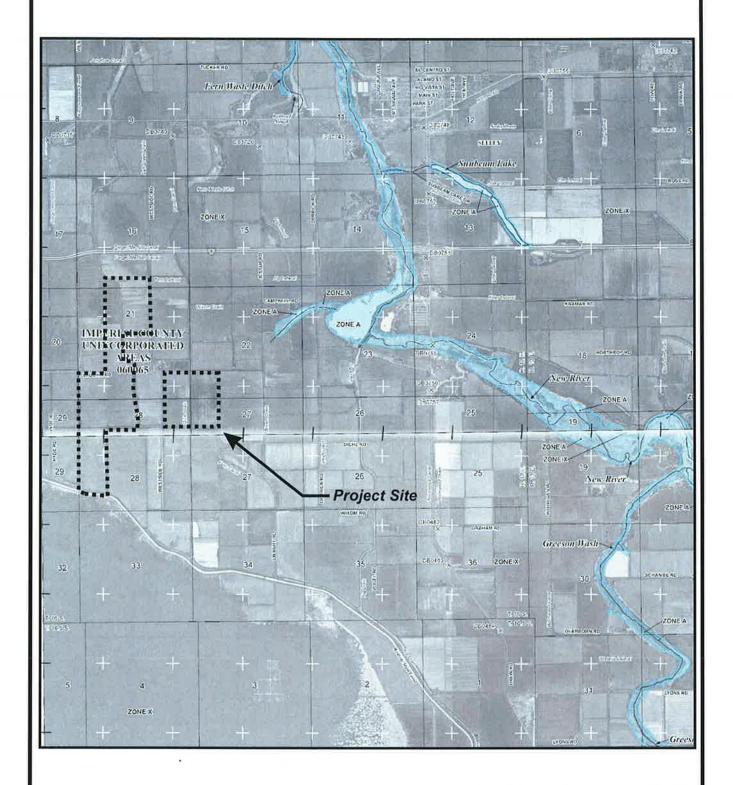


LANDMARK
Geo-Engineers and Geologists

Project No.: LE17142

Regional Geologic Map

Plate A-5



Reference: Federal Emergency Management Agency (FEMA) Imperial County-Panel Numbers 06025C1770C and 06025C2050C



Project No.: LE17142

Flood Insurance Rate Map (FIRM)

Plate A-6

## **LEGEND**



SPECIAL FLOOD HAZARD AREAS SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD

The 1% annual flood (100-year flood), also known as the base flood, is the flood that has a 1% chance of being equaled or exceeded in any given year. The Special Flood Hazard Area is the area subject to flooding by the 1% annual chance flood. Areas of Special Flood Hazard include Zones A, AE, AH, AO, AR, A99, V, and VE. The Base Flood Elevation Is the water-surface elevation of the 1% annual chance flood.

ZONE A No Base Flood Elevations determined.

ZONE AE Base Flood Elevations determined.

ZONE AH Flood depths of 1 to 3 feet (usually areas of ponding); Base Flood

Elevations determined.

ZONE AO Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities also

ZONE AR Special Flood Hazard Area formerly protected from the 1% annual chance flood by a flood control system that was subsequently decertified. Zone AR

indicates that the former flood control system is being restored to provide

protection from the 1% annual chance or greater flood.

**ZONE A99** Area to be protected from 1% annual chance flood by a Federal flood

protection system under construction; no Base Flood Elevations determined

ZONE V Coastal flood zone with velocity hazard (wave action); no Base Flood

Elevations determined.

Coastal flood zone with velocity hazard (wave action); Base Flood

Elevations determined.



ZONE VE

## FLOODWAY AREAS IN ZONE AE

The floodway is the channel of a stream plus any adjacent floodplain areas that must be kept free of encroachment so that the 1% annual chance flood can be carried without substantial increases in flood heights.

## OTHER FLOOD AREAS

ZONE X

Areas of 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood.

## OTHER AREAS

ZONE X

Areas determined to be outside the 0.2% annual chance floodplain.

ZONE D

Areas in which flood hazards are undetermined, but possible.

COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS

OTHERWISE PROTECTED AREAS (OPAs)

CBRS areas and OPAs are normally located within or adjacent to Special Flood Hazard Areas.

1% annual chance floodplain boundary

0.2% annual chance floodplain boundary

Floodway boundary

Zone D boundary

CBRS and OPA boundary

Boundary dividing Special Flood Hazard Area Zones and boundary dividing Special Flood Hazard Areas of different Base Flood Elevations, flood depths or flood velocities.

- 513 ~~~

Base Flood Elevation line and value; elevation in feet®

Base Flood Elevation value where uniform within zone; elevation (EL 987)

* Referenced to the North American Vertical Datum of 1988

(3)----(23)

Cross section line

Transect line

Geographic coordinates referenced to the North American Datum of 1983 (NAD 83), Western Hemisphere

87°07'45", 32°22'30" 2476000mN

1000-meter Universal Transverse Mercator grid values, zone

600000 FT

5000-foot grid ticks: California State Plane coordinate system, zone VI (FIPSZONE 0406), Lambert Conformal Conic projection

DX5510 x

Bench mark (see explanation in Notes to Users section of this

FIRM panel)

●M1.5

River Mile

# **APPENDIX B**

## REFERENCES

- California Building Standards Commission, 2017, 2016 California Building Code. California Code of Regulations, Title 24, Part 2, Vol. 2 of 2.
- California Division of Mines and Geology (CDMG), 1996, California Fault Parameters: available at <a href="http://www.consrv.ca.gov/dmg/shezp/fltindex.html">http://www.consrv.ca.gov/dmg/shezp/fltindex.html</a>.
- California Geological Survey (CGS), 2016, Fault Activity Map of California <a href="http://www.quake.ca.gov/gmaps/FAM/faultactivitymap.html#">http://www.quake.ca.gov/gmaps/FAM/faultactivitymap.html#</a>.
- California Geological Survey (CGS), 2016, Alquist-Priolo Earthquake Fault Zone Maps. <a href="http://maps.conservation.ca.gov/cgs/informationwarehouse/index.html?map=regulatorymaps">http://maps.conservation.ca.gov/cgs/informationwarehouse/index.html?map=regulatorymaps</a>
- Cetin, K. O., Seed, R. B., Der Kiureghian, A., Tokimatsu, K., Harder, L. F., Jr., Kayen, R. E., and Moss, R. E. S., 2004, Standard penetration test-based probabilistic and deterministic assessment of seismic soil liquefaction potential: ASCE JGGE, Vol., 130, No. 12, p. 1314-1340.
- Geologismiki, 2014, CLiq Computer Program, www.geologismiki.gr
- Jones, A. L., 2003, An Analytical Model and Application for Ground Surface Effects from Liquefaction, PhD. Dissertation, University of Washington, 362 p.
- McCrink, T. P., Pridmore, C. L., Tinsley, J. C., Sickler, R. R., Brandenberg, S. J., and Stewart, J. P., 2011, Liquefaction and Other Ground Failures in Imperial County, California, from the April 4, 2010, El Mayor—Cucapah Earthquake, CGS Special Report 220, USGS Open File Report 2011-1071, 84 p.
- Morton, P. K., 1977, Geology and mineral resources of Imperial County, California: California Division of Mines and Geology, County Report No. 7, 104 p.
- Rymer, M.J., Treiman, J.A., Kendrick, K.J., Lienkaemper, J.J., Weldon, R.J., Bilham, R., Wei, M., Fielding, E.J., Hernandez, J.L., Olson, B.P.E., Irvine, P.J., Knepprath, N., Sickler, R.R., Tong, .X., and Siem, M.E., 2011, Triggered surface slips in southern California associated with the 2010 El Mayor-Cucapah, Baja California, Mexico, earthquake: U.S. Geological Survey Open-File Report 2010-1333 and California Geological Survey Special Report 221, 62 p., available at http://pubs.usgs.gov/of/2010/1333/.
- U.S. Geological Survey (USGS), 1990, The San Andreas Fault System, California, Professional Paper 1515.
- U.S. Geological Survey (USGS), 2016, US Seismic Design Maps Web Application, available at http://geohazards.usgs.gov/designmaps/us/application.php

- Youd, T. L., 2005, Liquefaction-induced flow, lateral spread, and ground oscillation, GSA Abstracts with Programs, Vol. 37, No. 7, p. 252.
- Youd, T. L. and Garris, C. T., 1995, Liquefaction induced ground surface disruption: ASCE Geotechnical Journal, Vol. 121, No. 11.
- Zimmerman, R. P., 1981, Soil survey of Imperial County, California, Imperial Valley Area: U.S. Dept. of Agriculture Soil Conservation Service, 112 p.