#### **Preliminary Geotechnical and GeoHazards Report**

### **Big Rock Solar Site**SEC and NEC of Liebert Road and Wixom Road

El Centro, California

Prepared for:

92JT 8ME, LLC 211 Sutter Street, 6th Floor San Francisco, CA 94108





Prepared by:

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September 2017



September 1, 2017

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## Preliminary Geological and Geotechnical Hazard Evaluation Big Rock Solar Site SEC and NEC Liebert Road and Wixom Road El Centro, California LCI Project No. LE17083

Dear Mr. Kolta:

This preliminary geotechnical report and geologic hazards study is provided for preliminary site evaluation and permitting of the photo-voltaic solar farm at the approximately 342-acre project area located at the southeast and northeast corners of Liebert Road and Wixom Road approximately 8 miles west of El Centro, California.

#### **Scope of Work**

The scope of work consisted of a geologic and geotechnical hazards evaluation of the project site which addresses the following items:

- 1. General site geology.
- 2. Site location in relation to mapped earthquake faults and seismic zones.
- 3. Intensity of ground shaking at the site.
- 4. Potential for liquefaction, ground failure, and landslides at the site. No drilling was conducted to determine potential for liquefaction settlement or soil analysis.
- 5. Soil corrosivity.
- 6. Plant growth suitability of the site soils.
- 7. Preliminary pavement structural sections.
- 8. Potential for flooding at the site from manmade facilities (dams, canals, etc.) and from natural storms.
- 9. Other potential geologic and geotechnical hazards.

#### **Site Description**

The project site is located at the southeast and northeast corners of Liebert Road and Wixom Road approximately 8 miles west of El Centro, California. The project site consists of 342-acres comprised of seven (7) agricultural fields currently in crop production. Dirt field roads bisect the site. The Westside Main Canal (earthen) forms the southern property boundary. Liebert Road (unpaved) forms the western property boundary. Wixom Road (paved) forms the northern property boundary except for the 60-acre parcel located at the northeast corner of Wixom Road and Liebert Road. The Fig Drain, an approximately 10 foot deep irrigation drainage ditch, forms the eastern property boundary. There is an approximately 3 to 8 foot elevation difference between the northern and southwestern fields (lower) and the fields to the east (higher).

Agricultural fields are located around the perimeter of the project site. Dirt field roads are located along the margins and also cross the parcels. A rural residence and farm shop are located near the center of the northeast portion of the site. The adjacent properties are approximately the same elevation as the project sites.

#### **Site Geological Conditions**

Site Geology: The project site is located in the Imperial Valley portion of the Salton Trough physiographic province. The Salton Trough is a topographic and geologic structural depression resulting from large scale regional faulting. The trough is bounded on the northeast by the San Andreas Fault and Chocolate Mountains and the southwest by the Peninsular Range and faults of the San Jacinto Fault Zone. The Salton Trough represents the northward extension of the Gulf of California, containing both marine and non-marine sediments since the Miocene Epoch. Tectonic activity that formed the trough continues at a high rate as evidenced by deformed young sedimentary deposits and high levels of seismicity. Figure 1 shows the location of the site in relation to regional faults and physiographic features.

The Imperial Valley is directly underlain by lacustrine deposits, which consist of interbedded lenticular and tabular silt, sand, and clay. Based on Unified Soil Classification System, the permeability of these soils is expected to be low to moderate. The Late Pleistocene to Holocene lake deposits are probably less than 100 feet thick and derived from periodic flooding of the Colorado River which intermittently formed a fresh water lake (Lake Cahuilla).

Older deposits consist of Miocene to Pleistocene non-marine and marine sediments deposited during intrusions of the Gulf of California. Basement rock consisting of Mesozoic granite and Paleozoic metamorphic rocks are estimated to exist at depths between 15,000 - 20,000 feet.

Groundwater: The groundwater in the site area is brackish and typically encountered at a depth of between 5 to 10 feet below ground surface in the vicinity of the project site. There is uncertainty in the accuracy of short-term water level measurements, particularly in fine-grained soil. Groundwater levels may fluctuate with water elevation in the Westside Main Canal, precipitation, irrigation of adjacent properties, drainage, and site grading. The groundwater level noted should not be interpreted to represent an accurate or permanent condition.

Onsite Wastewater Disposal: The near surface soils at the project site generally consist of silts and silty sands having a moderate infiltration rate. The near surface soils are considered good in supporting onsite septic systems and leach fields for wastewater disposal. Site specific studies will be required to determine that County Environmental Health standards are met in regard to soil percolation rates and separation of leach fields from groundwater.

#### **Geological Hazards**

**Landsliding:** No ancient landslides are shown on geologic maps of the region and no indications of landslides were observed during our site investigation. The hazard of landsliding is unlikely due to the relatively planar topography of the project site.

*Volcanic hazards:* The site is not located proximal to any known volcanically active area and the risk of volcanic hazards is considered very low.

Tsunamis, seiches, and flooding: The site does not lie near any large bodies of water, so the threat of tsunami, seiches, or other seismically-induced flooding is considered unlikely. The project site is located in FEMA Flood Zone X, an area determined to be outside the 0.2% annual chance floodplain (FIRM Panel 06025C2050C). The Westside Main Canal forms a portion of the southern property boundary. The earthen canal supplies irrigation water to the western portion of the Imperial Valley. The canal embankments are approximately 3 to 4 feet above the elevation of the west side of the project site and the water level is slightly above the elevation of the project site.

**Expansive soil:** In general, much of the near surface soils within the project site consist of silts and silty sands having a low expansion potential. Clay soils may also be encountered within the near surface soils of the project site. A site specific geotechnical investigation will be required at this site to determine the extent and effect of expansive soils.

Corrosive Soils: The lacustrine site soils (lake bed deposits) are known to be corrosive. Typical remediation for the corrosive soil conditions consists of using concrete mixed with higher cement contents (6 sacks Type V Portland Cement) and low water-cement ratios (0.45 w/c ratio). Additionally, steel post corrosion protection is required, consisting of zinc coatings (galvanizing) or increased structural sections to compensate for metal loss due to corrosion.

Liquefaction/Seismic Settlements: Liquefaction is a potential design consideration because of possible saturated sandy substrata underlying the site. Liquefaction occurs when granular soil below the water table is subjected to vibratory motions, such as produced by earthquakes. With strong ground shaking, an increase in pore water pressure develops as the soil tends to reduce in volume. If the increase in pore water pressure is sufficient to reduce the vertical effective stress (suspending the soil particles in water), the soil strength decreases and the soil behaves as a liquid (similar to quicksand). Liquefaction can produce excessive settlement, ground rupture, lateral spreading, or failure of shallow bearing foundations.

Four conditions are generally required for liquefaction to occur:

- (1) the soil must be saturated (relatively shallow groundwater);
- (2) the soil must be loosely packed (low to medium relative density);
- (3) the soil must be relatively cohesionless (not clayey); and
- (4) groundshaking of sufficient intensity must occur to function as a trigger mechanism.

All of these conditions may exist to some degree at this site. Liquefaction settlement and ground fissures were noted along the Westside Main Canal in the area of the project site after the April 4, 2010 magnitude 7.2M<sub>w</sub> El Mayor-Cucapah Earthquake. McCrink and others (2011) reported several liquefaction related failures to the embankment of the Westside Main Canal along the southern margin of the project site. Ground fissures and sand boils were noted along the embankments of the Westside Main Canal.

#### Seismic Hazards

The project site is located in the seismically active Imperial Valley of southern California with numerous mapped faults of the San Andreas Fault System traversing the region. The San Andreas Fault System is comprised of the San Andreas, San Jacinto, and Elsinore Fault Zones in southern California. The Imperial fault represents a transition from the more continuous San Andreas fault to a more nearly echelon pattern characteristic of the faults under the Gulf of California (USGS, 1990). We have performed a computer-aided search of known faults or seismic zones that lie within a 62 mile (100 kilometer) radius of the project site (Table 1).

A fault map illustrating known active faults relative to the site is presented on Figure 1, *Regional Fault Map*. A legend for the regional fault map is presented on Figure 2. The criterion for fault classification adopted by the California Geological Survey defines Earthquake Fault Zones along active or potentially active faults. An active fault is one that has ruptured during Holocene time (roughly within the last 11,000 years). A fault that has ruptured during the last 1.8 million years (Quaternary time), but has not been proven by direct evidence to have not moved within Holocene time is considered to be potentially active. A fault that has not moved during both Pleistocene and Holocene time (that is no movement within the last 1.8 million years) is considered to be inactive.

Review of the current Alquist-Priolo Earthquake Fault Zone maps (CGS, 2000a) indicates that the nearest mapped Earthquake Fault Zone is an unnamed fault located approximately 2.1 miles west of the substation project site. Geologic mapping by the USGS (Rymer and others, 2011) of the Imperial Valley after the April 4, 2010 magnitude 7.2M<sub>w</sub> El Mayor-Cucapah Earthquake indicates movement along several known and unknown faults west of the project site. Surface rupture on these faults is possible from future seismic events in the area.

The nearest mapped major Earthquake Fault Zone is the Laguna Salada fault located approximately 9.4 miles southwest of the site and the Superstition Hills fault located approximately 8.5 miles northeast of the substation site.

*Groundshaking*. The primary seismic hazard at the project site is the potential for strong groundshaking during earthquakes along the Brawley Seismic Zone, Imperial, Brawley, East Highline, and Elmore Ranch Faults (Figure 2).

<u>Site Acceleration:</u> The project site is considered likely to be subjected to moderate to strong ground motion from earthquakes in the region. Ground motions are dependent primarily on the earthquake magnitude and distance to the seismogenic (rupture) zone. Accelerations also are dependent upon attenuation by rock and soil deposits, direction of rupture and type of fault; therefore, ground motions may vary considerably in the same general area.

CBC General Ground Motion Parameters: The 2016 CBC general ground motion parameters are based on the Risk-Targeted Maximum Considered Earthquake (MCE<sub>R</sub>). The U.S. Geological Survey "U.S. Seismic Design Maps Web Application" (USGS, 2017) was used to obtain the site coefficients and adjusted maximum considered earthquake spectral response acceleration parameters. The site soils have been classified as Site Class D (stiff soil profile).

Design spectral response acceleration parameters are defined as the earthquake ground motions that are two-thirds (2/3) of the corresponding MCE<sub>R</sub> ground motions. Design earthquake ground motion parameters are provided in Table 2. A Risk Category II was determined using Table 1604.5 and the Seismic Design Category is D since S<sub>1</sub> is less than 0.75.

The Maximum Considered Earthquake Geometric Mean (MCE<sub>G</sub>) peak ground acceleration (PGA<sub>M</sub>) value was determined from the "U.S. Seismic Design Maps Web Application" (USGS, 2017) for liquefaction and seismic settlement analysis in accordance with 2016 CBC Section 1803.5.12 and CGS Note 48 (PGA<sub>M</sub> =  $F_{PGA}*PGA$ ). A PGA<sub>M</sub> value of 0.50g has been determined for this project site.

*Surface Rupture:* The project site does not lie within a State of California, Alquist-Priolo Earthquake Fault Zone. Surface fault rupture at the project site is considered to be low.

#### **Other Hazards**

*Hazardous Materials:* The site is not located in proximity to any known hazardous materials (methane gas, tar seeps, hydrogen sulfide gas), and the risk of hazardous materials is considered very low.

**Radon 222 Gas:** Radon gas is not believed to be a potential hazard at the site. A report titled "California Statewide Radon Survey-Screening Results", dated November 1990 and published by the California State Department of Health Services, notes that Southern California showed a low risk of elevated radon levels, based on 2-day tests conducted from January through April 1990. Some of the reported testing was performed in Imperial County; however, no data was observed as being at or near the project site.

*Naturally occurring asbestos:* The site is not located in proximity to any known naturally occurring asbestos, and the risk of naturally occurring asbestos is considered very low.

*Hydrocollapse:* The site is dominantly underlain by clays that are not expected to collapse with the addition of water to the site. The risk of hydrocollapse is considered very low.

**Regional Subsidence:** Regional subsidence due to geothermal resource activities has not been documented in the area west of the New River; therefore, the risk of regional subsidence is considered low.

#### Conclusion

This preliminary report was prepared according to the generally accepted *geotechnical* engineering standards of practice that existed in Imperial County at the time the report was prepared. No express or implied warranties are made in connection with our services.

Our research did not reveal conditions that would preclude implementation of the proposed project provided site specific geotechnical investigations are conducted prior to site development to provide geotechnical criteria for the design and construction of this project.

We appreciate the opportunity to provide our findings and professional opinions regarding geologic and geotechnical hazards at the site. If you have any questions or comments regarding our findings, please call our office at (760) 370-3000.

CERTIFIED ENGINEERING GEOLOGIST

Sincerely Yours;

Landmark Consultants, Inc.

Steven K Williams, PG, CEG

Senior Engineering Geologist

Jeffrey O. Lyon, PE

President

Table 1
Summary of Characteristics of Closest Known Active Faults

Fault Name	Approximate Distance (miles)  Approxima Distance (ki		Maximum Moment Magnitude (Mw)	Fault Length (km)	Slip Rate (mm/yr)
Unnamed 1*	2.1	3.3			
Yuha*	3.8	6.1			
Unnamed 2*	4.1	6.5			
Shell Beds	7.3	11.6			
Yuha Well *	7.8	12.5			
Superstition Hills	8.5	13.7	6.6	23 ± 2	4 ± 2
Laguna Salada	9.4	15.0	7	67 ± 7	3.5 ± 1.5
Superstition Mountain	10.4	16.6	6.6	24 ± 2	5 ± 3
Vista de Anza*	10.7	17.1			
Borrego (Mexico)*	12.7	20.4			
Imperial	13.5	21.7	7	62 ± 6	20 ± 5
Painted Gorge Wash*	14.3	22.9			
Brawley *	15.1	24.1			
Ocotillo*	15.4	24.6			
Rico *	18.3	29.3			
Elsinore - Coyote Mountain	19.1	30.6	6.8	39 ± 4	4 ± 2
Pescadores (Mexico)*	20.5	32.8			
Cerro Prieto *	21.8	34.9			
Elmore Ranch	22.0	35.2	6.6	29 ± 3	1 ± 0.5
Cucapah (Mexico)*	22.8	36.5			
San Jacinto - Borrego	25.5	40.7	6.6	29 ± 3	4 ± 2
San Andreas - Coachella	42.5	68.0	7.2	96 ± 10	25 ± 5

<sup>\*</sup> Note: Faults not included in CGS database.

Table 2 2016 California Building Code (CBC) and ASCE 7-10 Seismic Parameters

CBC Reference Table 20.3-1

Soil Site Class: **D** T Latitude: 32.7360 N

Latitude: 32./360 N Longitude: -115.7075 W

Risk Category: I Seismic Design Category: D

#### Maximum Considered Earthquake (MCE) Ground Motion

Mapped MCE <sub>R</sub> Short Period Spectral Response	$S_s$	1.500 g	Figure 1613.3.1(1)	)
Mapped MCE <sub>R</sub> 1 second Spectral Response	$S_1$	0.600 g	Figure 1613.3.1(2)	)
Short Period (0.2 s) Site Coefficient	$\mathbf{F_a}$	1.00	Table 1613.3.3(1)	
Long Period (1.0 s) Site Coefficient	$\mathbf{F_v}$	1.50	Table 1613.3.3(2)	
7 . 1D	C	1.500	F * C	_

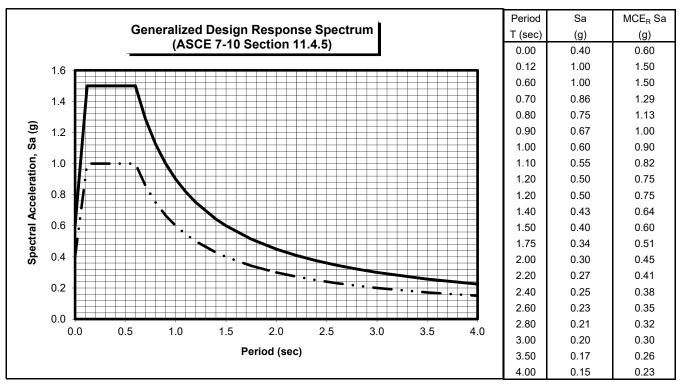
MCE<sub>R</sub> Spectral Response Acceleration Parameter (0.2 s)  $S_{MS}$  1.500 g =  $F_a * S_s$  Equation 16-37 MCE<sub>R</sub> Spectral Response Acceleration Parameter (1.0 s)  $S_{MI}$  0.900 g =  $F_v * S_1$  Equation 16-38

#### **Design Earthquake Ground Motion**

Design Spectral Response Acceleration Parameter (0.2 s)	$S_{DS}$	1.000 g	$= 2/3*S_{MS}$	Equation 16-39
Design Spectral Response Acceleration Parameter (1.0 s)	$S_{D1}$	0.600 g	$= 2/3*S_{M1}$	Equation 16-40
	$T_{\rm L}$	8.00 sec		ASCE Figure 22-12
	an.	0.10	0.240 /0	

 $T_{O}$  0.12 sec =0.2\* $S_{D1}/S_{DS}$  $T_{S}$  0.60 sec = $S_{D1}/S_{DS}$ 

Peak Ground Acceleration  $PGA_M$  0.50 g ASCE Equation 11.8-1

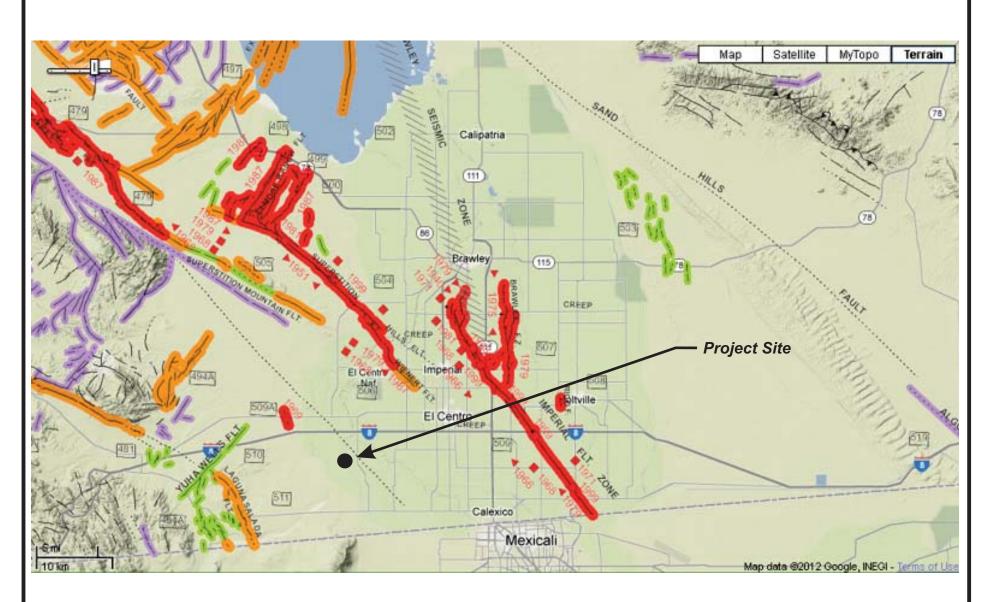


Design Response Spectra MCE<sub>R</sub> Response Spectra



Source: California Geological Survey 2010 Fault Activity Map of California http://www.quake.ca.gov/gmaps/FAM/faultactivitymap.html#





Source: California Geological Survey 2010 Fault Activity Map of California http://www.quake.ca.gov/gmaps/FAM/faultactivitymap.html#



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#### **EXPLANATION**

Fault traces on land are indicated by solid lines where well located, by dashed lines where approximately located or inferred, and by dotted lines where concealed by younger rocks or by lakes or bays. Fault traces are queried where continuation or existence is uncertain. Concealed faults in the Great Valley are based on maps of selected subsurface horizons, so locations shown are approximate and may indicate structural trend only. All offshore faults based on seismic reflection profile records are shown as solid lines where well defined, dashed where inferred, queried where uncertain.

#### FAULT CLASSIFICATION COLOR CODE (Indicating Recency of Movement)

Fault along which historic (last 200 years) displacement has occurred and is associated with one or more of the following:

- (a) a recorded earthquake with surface rupture. (Also included are some well-defined surface breaks caused by ground shaking during earthquakes, e.g. extensive ground breakage, not on the White Wolf fault, caused by the Arvin-Tehachapi earthquake of 1952). The date of the associated earthquake is indicated. Where repeated surface ruptures on the same fault have occurred, only the date of the latest movement may be indicated, especially if earlier reports are not well documented as to location of ground breaks.
- (b) fault creep slippage slow ground displacement usually without accompanying earthquakes.
- (c) displaced survey lines.

A triangle to the right or left of the date indicates termination point of observed surface displacement. Solid red triangle indicates known location of rupture termination point. Open black triangle indicates uncertain or estimated location of rupture termination point.

Date bracketed by triangles indicates local fault break.

No triangle by date indicates an intermediate point along fault break.

Fault that exhibits fault creep slippage. Hachures indicate linear extent of fault creep. Annotation (creep with leader) indicates representative locations where fault creep has been observed and recorded.

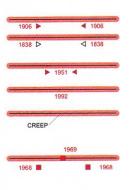
Square on fault indicates where fault creep slippage has occured that has been triggered by an earthquake on some other fault. Date of causative earthquake indicated. Squares to right and left of date indicate terminal points between which triggered creep slippage has occurred (creep either continuous or intermittent between these end points).

Holocene fault displacement (during past 11,700 years) without historic record. Geomorphic evidence for Holocene faulting includes sag ponds, scarps showing little erosion, or the following features in Holocene age deposits: offset stream courses, linear scarps, shutter ridges, and triangular faceted spurs. Recency of faulting offshore is based on the interpreted age of the youngest strata displaced by faulting.

Late Quaternary fault displacement (during past 700,000 years). Geomorphic evidence similar to that described for Holocene faults except features are less distinct. Faulting may be younger, but lack of younger overlying deposits precludes more accurate age classification.

Quaternary fault (age undifferentiated). Most faults of this category show evidence of displacement sometime during the past 1.6 million years; possible exceptions are faults which displace rocks of undifferentiated Plio-Pleistocene age. Unnumbered Quaternary faults were based on Fault Map of California, 1975. See Bulletin 201, Appendix D for source data.

Pre-Quaternary fault (older that 1.6 million years) or fault without recognized Quaternary displacement. Some faults are shown in this category because the source of mapping used was of reconnaissnce nature, or was not done with the object of dating fault displacements. Faults in this category are not necessarily inactive.



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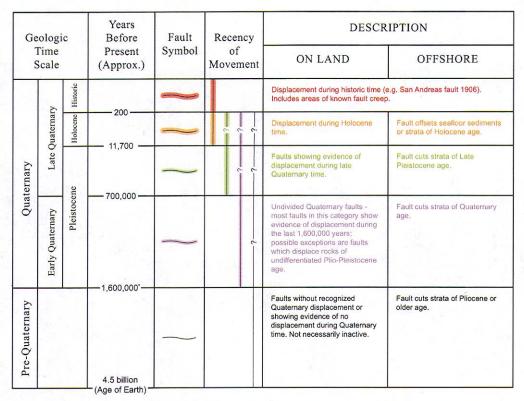
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Fault Map Legend

**Figure** 3a

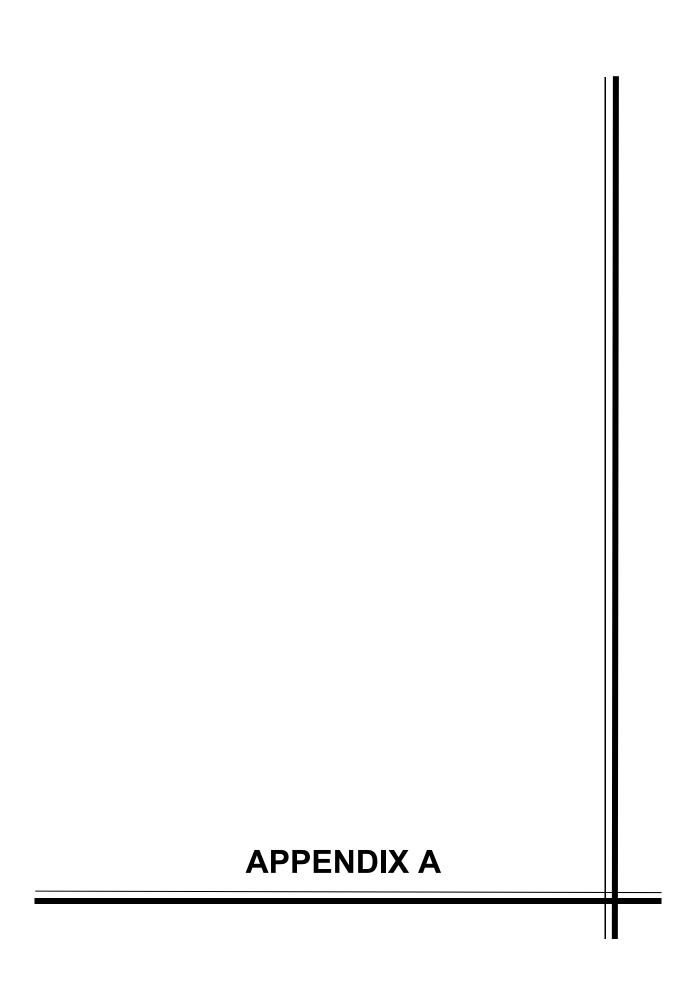
#### ADDITIONAL FAULT SYMBOLS

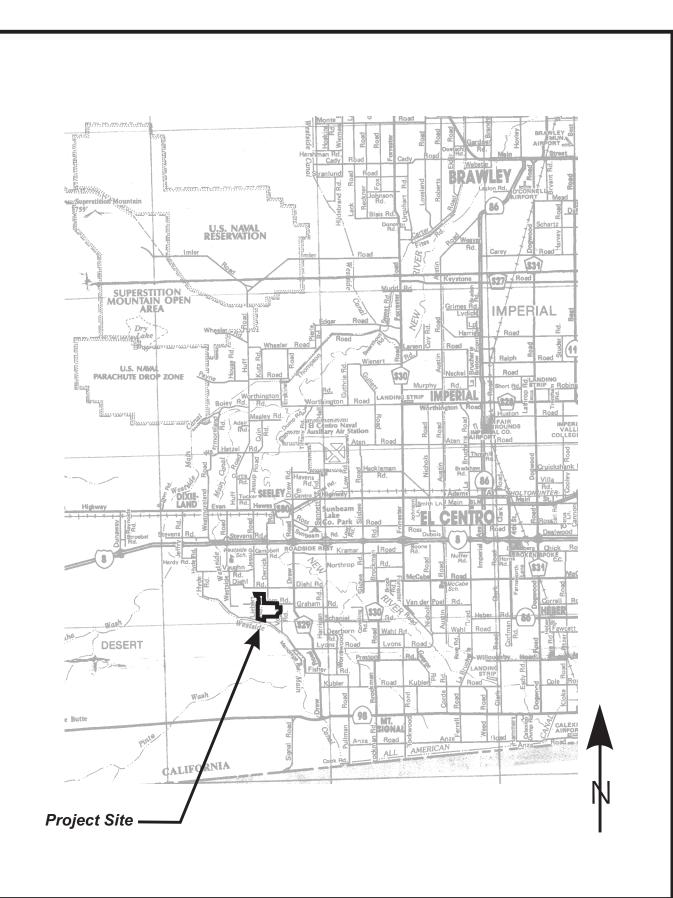
	Bar and ball on downthrown side (relative or apparent).
	Arrows along fault indicate relative or apparent direction of lateral movement.
	Arrow on fault indicates direction of dip.
	Low angle fault (barbs on upper plate). Fault surface generally dips less than 45° but locally may have been subsequently steepened. On offshore faults, barbs simply indicate a reverse fault regardless of steepness of dip.
	OTHER SYMBOLS
491	Numbers refer to annotations listed in the appendices of the accompanying report. Annotations include fault name, age of fault displacement, and pertinent references including Earthquake Fault Zone maps where a fault has been zoned by the Alquist-Priolo Earthquake Fault Zoning Act. This Act requires the State Geologist to delineate zones to encompass faults with Holocene displacement.
	Structural discontinuity (offshore) separating differing Neogene structural domains. May indicate discontinuities between basement rocks.
	Brawley Seismic Zone, a linear zone of seismicity locally up to 10 km wide associated with the releasing step between the Imperial and San Andreas faults.



<sup>\*</sup> Quaternary now recognized as extending to 2.6 Ma (Walker and Geissman, 2009). Quaternary faults in this map were established using the previous 1.6 Ma criterion.





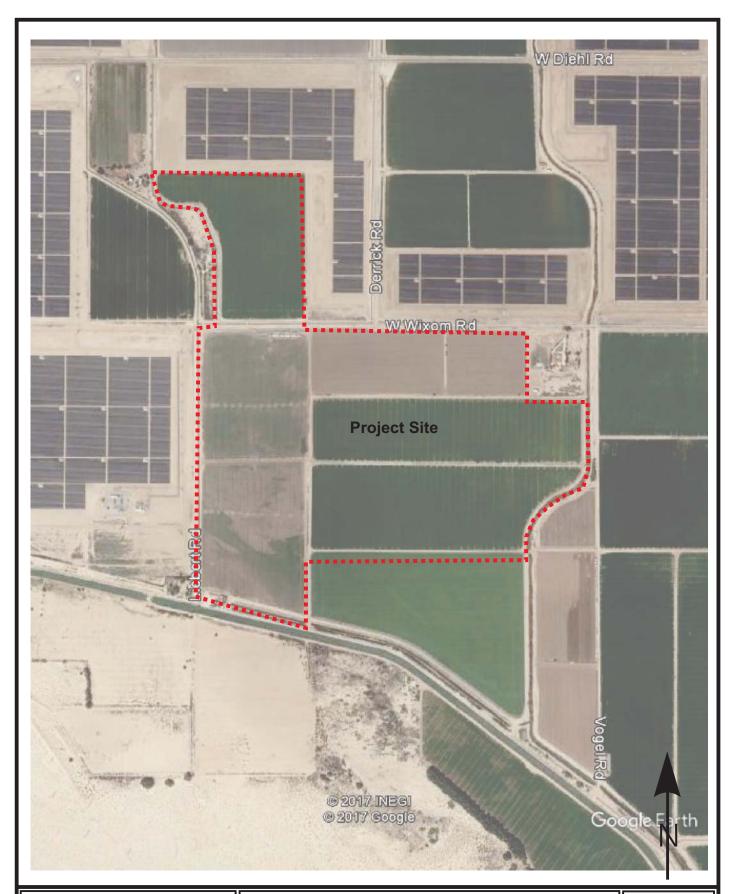


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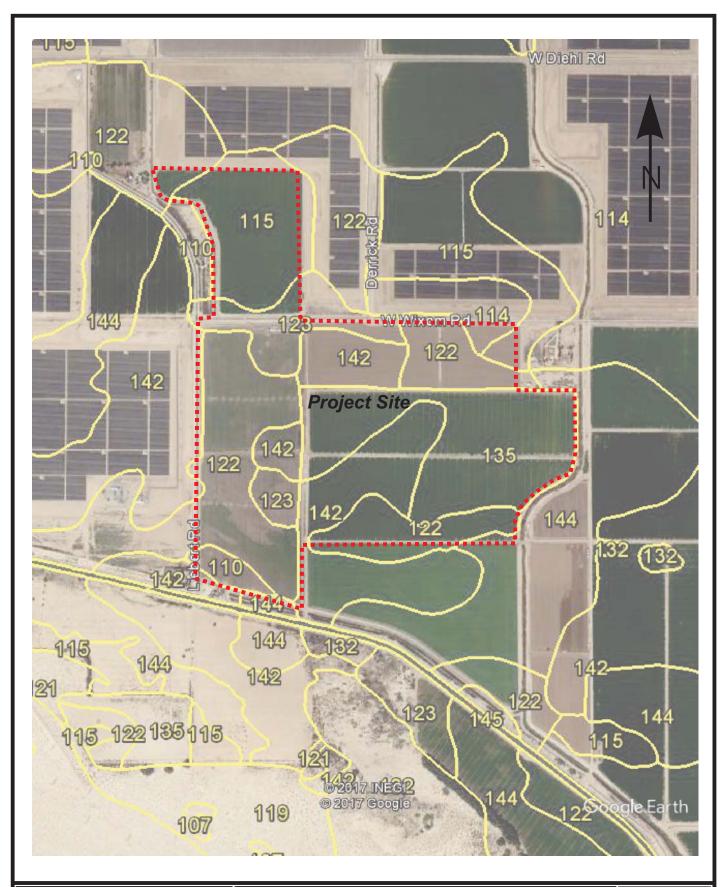
Project No.: LE17083

**Vicinity Map** 



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Site Map



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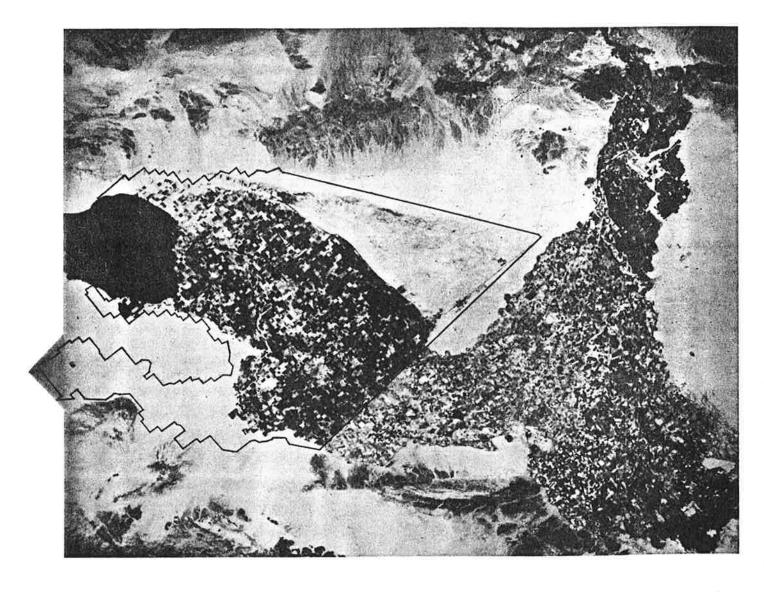
Geo-Engineers and Geologists

Project No.: LE17083

Soil Survey Map

#### **Soil Survey of**

# IMPERIAL COUNTY CALIFORNIA IMPERIAL VALLEY AREA



United States Department of Agriculture Soil Conservation Service
in cooperation with
University of California Agricultural Experiment Station
and
Imperial Irrigation District

TABLE 11.--ENGINEERING INDEX PROPERTIES

[The symbol > means more than. Absence of an entry indicates that data were not estimated]

Soil name and	Depth	USDA texture	Classif	·	Frag- ments					Liquid	Plas-
map symbol			Unified	AASHTO	> 3 inches	4	10	40	200	limit	ticity index
	In				Pct					Pot	
100 Antho		Loamy fine sand Sandy loam, fine sandy loam.	100	A-2 A-2, A-4	0 0	100 9 <b>0-1</b> 00	100 75-95	75-85 50-60		===	N P N P
101*:						İ					
Antho		Loamy fine sand Sandy loam, fine sandy loam.	SM	A-2  A-2,   A-4	0		100 75 <b>-</b> 95				NP NP
Superstition		Fine sand Loamy fine sand, fine sand, sand.		A-2 A-2	0		95-100 95-100			==	N P N P
102*. Badland											
103 Carsitas	0-10 10-60	Gravelly sand Gravelly sand, gravelly coarse sand, sand.	SP, SP-SM	A-1, A-2 A-1			50 <b>-</b> 85 50 <b>-</b> 85		0-10 0-10	==	N P N P
104 <b>*</b> Fluvaquents			9 9 9								
105 Glenbar	13-60	Clay loam Clay loam, silty clay loam.	CL CL	A-6 A-6	0 0	100 100		90-100 90-100		35-45 35-45	15 <b>-</b> 30 15 <b>-</b> 30
106 Glenbar	113-60	Clay loam Clay loam, silty clay loam.		A-6, A-7 A-6, A-7		100 100		90 <b>-</b> 100 90 <b>-</b> 100		35-45 35-45	15-25 15-25
107* Glenbar	0-13		ML, CL-ML, CL	A – 4	0	100	100	100	70-80	20-30	NP-10
		Clay loam, silty clay loam.		A-6, A-7	0	100	100	95-100	75-95	35-45	15-30
108Holtville	14-22 22-60	LoamClay, silty clay Silt loam, very fine sandy loam.	CL, CH	A – 4 A – 7 A – 4	0 0 0	100 100 100	100	85-100 95-100 95-100	85-95	25-35 40-65 25-35	NP-10 20-35 NP-10
	17 <b>-</b> 24 24 <b>-</b> 35	Clay, silty clay Silt loam, very fine sandy	CL, CH	A-7 A-7 A-4	0			95-100 95-100 95-100	85-95	40-65	20-35 20-35 NP-10
		loam. Loamy very fine sand, loamy fine sand.	SM, ML	A-2, A-4	0	100	100	75-100	20-55		ΝP
110 Holtville	17→24   24→35	Silty clay Clay, silty clay Silt loam, very fine sandy loam.	CH, CL	A-7 A-7 A-4	0 0 0	100 100 100	100	95-100 95-100 95-100	85-95	40-65 40-65 25-35	20-35 20-35 NP-10
		Loamy very fine sand, loamy fine sand.	SM, ML	A-2, A-4	0	100	100	75 <b>-</b> 100	20-55		NΡ

See footnote at end of table.

TABLE 11.--ENGINEERING INDEX PROPERTIES--Continued

Soil name and	Depth	USDA texture	Classifi		Frag-	Pe	rcentag sieve n		Liquid	Plas-	
map symbol	pepu	ODDA GEXCUIE	Unified	AASHTO	> 3	4	10	40	200	limit	
	<u>In</u>				Pet					Pct	-11122-
111*: Holtville	10-22 22-60	Silty clay loam Clay, silty clay Silt loam, very fine sandy loam.	CL, CH	A-7 A-7 A-4	0 0 0	100 100 100	100	95-100 95-100 95-100	85-95	40-65 40-65 25-35	20-35 20-35 NP-10
Imperial	12-60	Silty clay loam  Silty clay loam,   silty clay,   clay.		A-7 A-7	0 0	100 100	100 100		85 <b>-</b> 95 85 <b>-</b> 95	40-50 50-70	10-20 25-45
112 Imperial	12-60	Silty clay Silty clay loam, silty clay, clay.		A-7 A-7	0	100 100	100 100		85-95 85-95	50-70 50-70	25-45 25-45
113Imperial	12-60 1	Silty clay  Silty clay,   clay, silty   clay loam.		A-7 A-7	0	100 100	100 100		85 <b>-</b> 95 85 <b>-</b> 95	50-70 50-70	25-45 25-45
114 Imperial	12-60 	Silty clay  Silty clay loam,   silty clay,   clay.		A-7 A-7	0	100 100	100 100		85-95 85-95		25-45 25-45
115 <b>*:</b> Imperial	12 <b>-</b> 60	Silty clay loam Silty clay loam, silty clay, clay.	CL CH	A-7 A-7	0	100 100	<b>1</b> 00 100		85 <b>-</b> 95  85 <b>-</b> 95	40-50 50-70	10-20 25-45
Glenbar		Silty clay loam Clay loam, silty clay loam.		A-6, A-7		100 100		90-100 90-100		7	15-25 15-25
116*: Imperial	0-13 13-60	Silty clay loam Silty clay loam, silty clay, clay.	CL CH	A – 7   A – 7	0	100 100	100 100		85-95 85-95		10-20 25-45
Glenbar		Silty clay loam Clay loam, silty clay loam.		A-6, A-7	0	100 100		90 <b>-</b> 100 90-100		35-45 35-45	15-25 15-30
117, 118 Indio	0-12 12-72	LoamStratified loamy very fine sand to silt loam.	ML ML	A – 4 A – 4	0	95-100 95-100				20 <b>-</b> 30 20 <b>-</b> 30	NP-5 NP-5
119*: Indio	0-12 12-72	Loam	IML	A – 4 A – 4	0	95-100 95-100				20-30	NP-5 NP-5
Vint		Loamy fine sand Loamy sand, loamy fine sand.	SM SM	A-2 A-2	0	95-100 95-100					N P N P
120* Laveen	0-12 12-60	Loam Loam, very fine sandy loam.	ML, CL-ML	A – 4 A – 4	0	100 195-100	95-100 85-95	75-85 70-80	55-65 55-65	20-30 15 <b>-</b> 25	NP-10 NP-10

See footnote at end of table.

TABLE 11.--ENGINEERING INDEX PROPERTIES--Continued

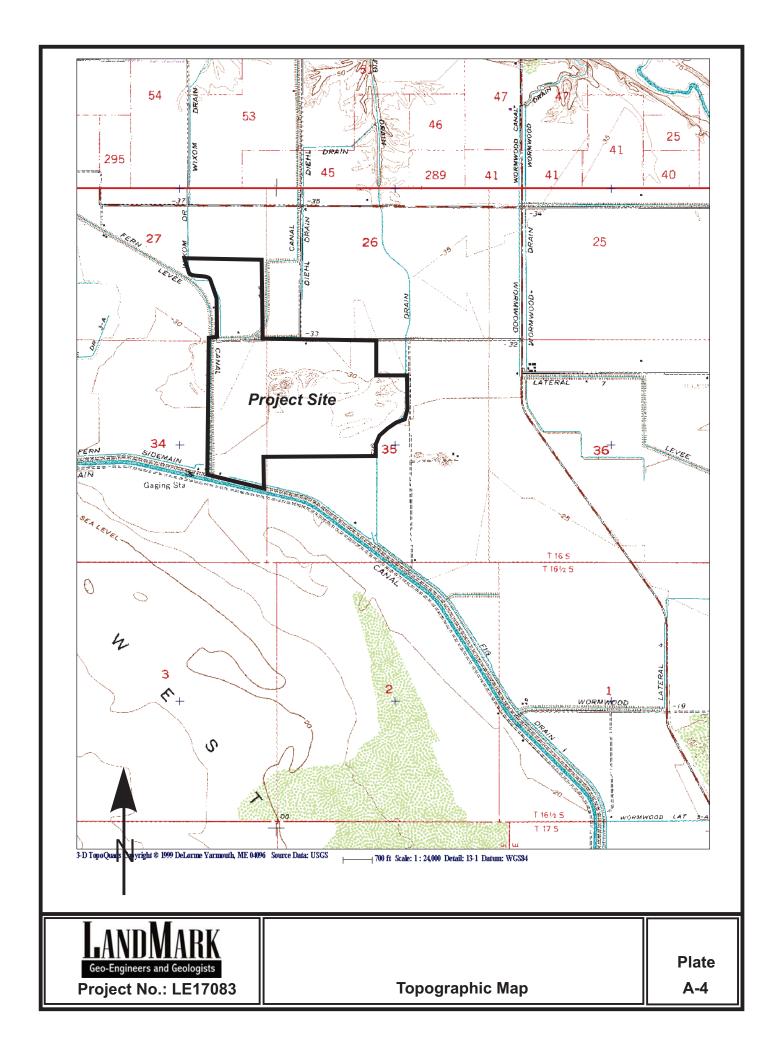
Soil name and	Depth	USDA texture	Classifi				Frag- ments	Pε		ge passi number		Liquid	Plas-
map symbol	S S P G M		Uni	ified	AASH	OTE	> 3  inches	4	10	40	200	limit	ticity index
	<u>In</u>						Pot		9			Pet	
121 Meloland	0-12 12-26	Fine sand Stratified loamy fine sand to	SM,	SP-SM	A-2, A-4	A-3		95 <b>–</b> 100 100		75-100 90-100		25 <b>-</b> 35	NP NP-10
		silt loam. Clay, silty clay, silty clay loam.	CL,	СН	A-7		0	100	100	95-100	85-95	40-65	20-40
122	0-12		ML		A-4		0	95-100	95-100	95⊸100	55-85	25-35	NP-10
Meloland	1 1	loam.  Stratified loamy   fine sand to	ML		A-4		0	100	100	90-100	50 <b>-</b> 70	25-35	NP-10
	26-71	silt loam. Clay, silty clay, silty clay loam.	сн,	CL	A-7		0	100	100	95-100	85 <b>-</b> 95	40-65	20-40
123*: Meloland	0 12	1 000	! мт		A-4		0	  95 <b>-</b> 100	95 <b>~</b> 100	95-100	  55 <b>-</b> 85	25 <b>-</b> 35	NP-10
Metotand	12-26	Stratified loamy fine sand to silt loam.	ML		A – 4		0				50-70		NP-10
	26-38	Clay, silty clay, silty clay loam.	сн,	CL	A-7		0	100	100	95-100	85-95	40-65	20-40
	38-60	Stratified silt loam to loamy fine sand.	SM,	ML	A-4		0	100	100	75-100	35 <b>-</b> 55	25 <b>-</b> 35	NP-10
Holtville	112-24	Clay, silty clay Silt loam, very fine sandy	CH,	CL	A-4   A-7   A-4		0	100 100 100	100	195-100	55 <b>-</b> 95  85 <b>-</b> 95  55 <b>-</b> 85	40-65	NP-10 20-35 NP-10
	36-60	loam. Loamy very fine sand, loamy fine sand.	SM,	ML	A-2,	A - 4	0	100	100	75-100	20-55		NР
124, 125 Niland		Gravelly sand Silty clay, clay, clay loam.	SM, CL,	SP-SM CH	A-2, A-7	A-3	0	90-100					NP 20-40
126 Niland	0-23	Fine sand Silty clay	SM,	SP-SM CH	A-2, A-7	A - 3	0	90-100			5 <b>-</b> 25 80 <b>-</b> 95		NP 20-40
127Niland	0-23 23-60	Loamy fine sand Silty clay	SM CL,	СН	A-2 A-7		0	90-100		50-65  85-100		40-65	NP 20-40
128*: Niland		Gravelly sand Silty clay, clay, clay loam.	SM,		A-2, A-7	A - 3	0	90-100 100	70-95 100	50-65 85-100		40-65	NP 20-40
Imperial	0-12	Silty clay Silty clay loam, silty clay, clay.	CH		A-7 A-7		0	100 100	100 100	100 100	85 <b>-</b> 95 85 <b>-</b> 95	50 <b>-</b> 70 50 <b>-</b> 70	25-45 25-45
129*: Pits													
130, 131 Rositas	0-27	Sand	SP-	-SM	A-3, A-1 A-2	,	0	100	80-100	40-70	5-15		NP
	27-60	Sand, fine sand, loamy sand.	SM,	SP-SM		2,	0	100	80-100	40-85	5-30		N P

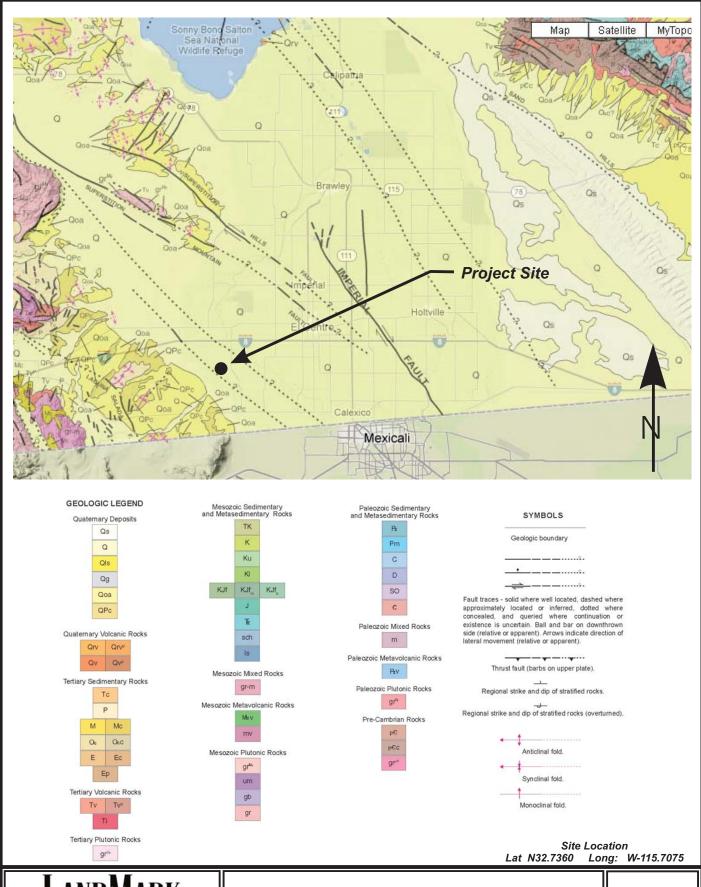
See footnote at end of table.

TABLE 11.--ENGINEERING INDEX PROPERTIES--Continued

Soil name and	Depth	USDA texture		ication	Frag=  ments	i P	Percentage passing sieve number				Plas-
map symbol			Unified	AASHTO	> 3  inches	4	10	40	200	Liquid   limit	ticity
	I <u>In</u>				Pet		į			Pet	
132, 133, 134, 135- Rositas	0-9	Fine sand	SM	A-3,	0	100	80-100	50-80	10-25		NP
	9-60	Sand, fine sand, loamy sand.	SM, SP-SM	•	0	100	80-100	40-85	5-30		NP
136 Rositas	0-4 4-60	Loamy fine sand  Sand, fine sand,   loamy sand.	SM, SP-SM	A-1, A-2 A-3, A-2, A-1	0	100 100	80-100 80-100			==	NP NP
137Rositas		Silt loam  Sand, fine sand,   loamy sand.	ISM, SP-SM	A-4 A-3, A-2, A-1	0	100 100	100 80 <b>–</b> 100	  90-100  40-85 		20-30	NP-5 NP
138*: Rositas	0-4 4-60	Loamy fine sand Sand, fine sand, loamy sand.	SM SM, SP-SM	A-1, A-2 A-3, A-2, A-1	0	100 100	80-100 80-100			==	NP NP
Superstition		Loamy fine sand Loamy fine sand, fine sand, sand.		A-2 A-2	0		95-100 95-100			==	NP NP
139 Superstition	6-60	Loamy fine sand Loamy fine sand, fine sand, sand.		A-2 A-2	0 0		95-100 95-100			===	N P N P
140*: Torriorthents											
Rock outcrop											
141 <b>*:</b> Torriorthents											
Orthids											
142    Vint		Loamy very fine sand.	SM, ML	A-4	0	100	100	85-95	40-65	15-25	NP-5
		Loamy fine sand	SM	A-2	0	95-100	95-100	70-80	20-30		ΝP
143  Vint	0-12	Fine sandy loam	ML, CL-ML, SM,	A-4	0	100	100	75-85	45 <b>-</b> 55	15-25	NP-5
	12-60	Loamy sand, loamy fine sand.	SM-SC	A-2	0	95-100	95-100	70-80	20-30		ΝP
144*:										- 1	
1	4	Very fine sandy loam.		A-4 i	0	100	100	85-95	40-65	15-25	NP-5
	10-40   40-60	Loamy fine sand Silty clay	SM CL, CH	A-2 A-7		95 <b>-</b> 100 100	95~100 100			40-65	NP 20-35
Indio	0-12	Very fine sandy	ML	A-4	0	95-100	95-100	85-100	75-90	20-30	NP-5
	1	loam. Stratified loamy very fine sand	ML	A-4	0	95-100	95-100	85-100	75-90	20-30	NP-5
	40 <b>-</b> 72	to silt loam. Silty clay	CL, CH	A-7	0	100	100	95-100	85-95	40-65	20-35

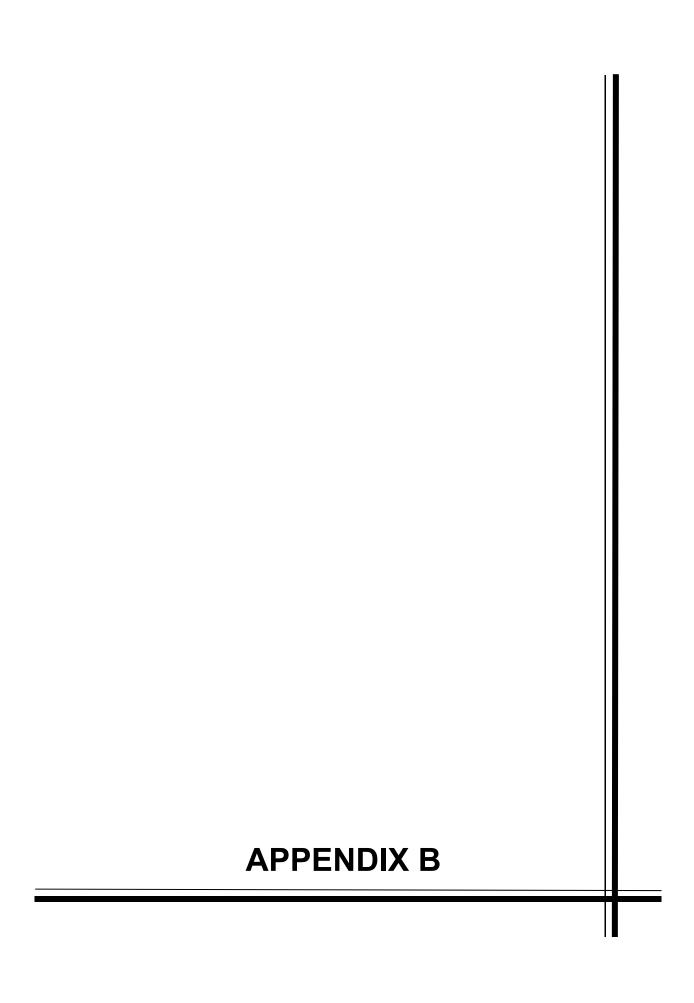
 $<sup>{}^{*}</sup>$  See description of the map unit for composition and behavior characteristics of the map unit.





Geo-Engineers and Geologists
Project No.: LE17083

Regional Geologic Map



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