18.0 Horizon Year 2049 Conditions

The year 2049 was selected as the horizon year because it is 30 years past the mid-point (year 2019) of the best estimate of when the project may be constructed. At the conclusion of the CUP term (estimated at year 2049), the Project entitlements require the Applicant to decommission the site and restore it to farmland uses in accordance with a future reclamation Plan. Implementation of the future reclamation plan is anticipated to generate traffic on the roads in the vicinity of the Project from trucks that will remove solar panels and other infrastructure from the site and install during a 7 month period ending at the end of 2049, agriculture related infrastructure. The traffic would also include the workers who travel to and from the site to perform the work. Nevertheless, after careful consideration of various methodologies for evaluating such traffic impacts, it is not possible to accurately forecast the traffic impacts for the following reasons:

- 1) There have been no solar projects decommissioned in Imperial County yet to provide a reference point for traffic impacts,
- 2) The near-term construction work force is based on the concentration of populations per the 2010 Census. The source and location of a horizon year 2049 construction work force cannot be estimated in the same manner; therefore, it would require speculation to determine where the construction work force would originate and the amount of workers from the local area (i.e. Imperial Valley) vs. the regional area (i.e. Los Angeles, San Diego, or Arizona),
- 3) Other solar projects on the cumulative project list in the vicinity may or may not be performing their own decommissioning phase activities at the same time. Many of these other solar projects have a 10 year extension option and it is not possible to estimate how many would exercise the option. Accordingly, only a guess could be made to as to when the other cumulative projects would initiate their own decommissioning phases and thus would add traffic to the horizon year background conditions, and
- 4) The horizon year traffic model for Imperial County does not have horizon year volumes for the study area roadways around the project site nor does the traffic model have data for decommissioning scenarios.

Therefore, after a thorough investigation for reliable data and having used our best efforts to find out and disclose all the information we reasonably can about traffic in the decommissioning phase, the only conclusion that can be drawn is that it is simply too speculative for evaluation.

19.0 Conclusions and Recommendations

The project is a solar photovoltaic energy-generating facility of approximately 250 megawatts of electricity on approximately 2,793 acres. The project is located approximately 8 miles west of the City of Calexico in the Mt. Signal area of Imperial Valley.

The project consists of a construction phase and operations phase. The construction phase will have the highest amount of workers and greatest amount of traffic while the operations phase will have approximately 15 fulltime personnel. Therefore, the higher and more conservative construction trip generation was used to determine potential project impacts. The worker and construction truck traffic was calculated at 664 ADT with 209 AM peak hour trips (203 inbound and 6 outbound) and 209 PM peak hour trips (6 inbound and 203 outbound).

The project may be phased over time; therefore, four possible phases were analyzed. This included the entire project being constructed early in 2013 (existing conditions scenario), the entire project being constructed on a typical schedule in 2016 (near-term scenario), the entire project being construct in 2019 (2014 + 5 year for a mid-point scenario of the CUP), and the entire project being delayed due to market forces until 2024 (long-term scenario).

Information on cumulative projects was obtained from the County of Imperial and confirmed with the County of Imperial EIR team to be current as of February 2014. Cumulative projects that are located in the immediate area around the project site (i.e. projects that are generally located south of I-8 and west of Clark Road) were included in this analysis.

Twelve scenarios were analyzed, that accounted for existing, project construction, cumulative projects, horizon year, and limited decommissioning conditions. Operational findings by scenario are summarized below:

- 1) Under existing year 2013 conditions, the study intersections, roadways, and freeway segments were calculated to operate at LOS C or better.
- 2) Under existing year 2013 + project construction conditions, the study intersections, roadways, and freeway segments were calculated to operate at LOS C or better with no significant direct project impacts.
- 3) Under existing year 2013 + project construction + cumulative conditions, the study intersections, roadways, and freeway segments were calculated to operate at LOS C or better with no cumulatively considerable impacts.
- 4) Under near-term year 2016 conditions, the study intersections, roadways, and freeway segments were calculated to operate at LOS C or better.
- 5) Under near-term year 2016 + project construction conditions, the study intersections, roadways, and freeway segments were calculated to operate at LOS C or better with no significant direct project impacts.
- 6) Under near-term year 2016 + project construction + cumulative conditions, the study intersections, roadways, and freeway segments were calculated to operate at LOS C or better with no cumulatively considerable impacts.

- 7) Under near-term year 2019 conditions, the study intersections, roadways, and freeway segments were calculated to operate at LOS C or better.
- 8) Under near-term year 2019 + project construction conditions, the study intersections, roadways, and freeway segments were calculated to operate at LOS C or better with no significant direct project impacts.
- 9) Under near-term year 2019 + project construction + cumulative conditions, the study intersections, roadways, and freeway segments were calculated to operate at LOS C or better with no cumulatively considerable impacts.
- 10) Under long-term year 2024 conditions, the study intersections, roadways, and freeway segments were calculated to operate at LOS C or better.
- 11) Under long-term year 2024 + project construction conditions, the study intersections, roadways, and freeway segments were calculated to operate at LOS C or better with no significant direct project impacts.
- 12) Under long-term year 2024 + project construction + cumulative conditions, the study intersections, roadways, and freeway segments were calculated to operate at LOS C or better with no cumulatively considerable impacts.

The project may be phased with construction occurring in years 2013, 2016, 2019, or 2024. As noted above for the various scenarios, there are no calculated traffic impacts under existing 2013 conditions, near-term 2016 conditions, mid-term 2019 conditions, or long-term 2024 conditions. Since these are no significant impact from long-term conditions of the entire project in these scenarios where other cumulative projects are generating traffic, we conclude that if the project were to be constructed either one CUP or a group of CUPs at a time phased out over the 10 years permitted by the Project's Development Agreement, then such phased-CUP construction would also not have a significant direct project impact or cumulatively considerable impact on traffic.

The year 2049 was selected as the horizon year because it is 30 years past the mid-point (year 2019) of the best estimate of when the project may be constructed. At the conclusion of the CUP term (estimated at year 2049), the Project entitlements require the Applicant to decommission the site and restore it to farmland uses in accordance with a future reclamation Plan. Implementation of the future reclamation plan is anticipated to generate traffic on the roads in the vicinity of the Project from trucks that will remove solar panels and other infrastructure from the site and install during a 7 month period ending at the end of 2049, agriculture related infrastructure. The traffic would also include the workers who travel to and from the site to perform the work. Nevertheless, after careful consideration of various methodologies for evaluating such traffic impacts, it is not possible to accurately forecast the traffic impacts. Therefore, after a thorough investigation for reliable data and having used our best efforts to find out and disclose all the information we reasonably can about traffic in the decommissioning phase, the only conclusion that can be drawn is that it is simply too speculative for evaluation.

20.0 References

Caltrans. December 2002. Guide for the Preparation of Traffic Impact Studies.

County of Imperial Department of Public Works. Dated March 12, 2007, revised June 29, 2007 and approved by the Board of Supervisors of the County of Imperial on August 7, 2007. *Traffic Study and Report Policy*.

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Imperial County Planning & Development Services Department. January 29, 2008. *Circulation and Scenic Highways Element*.

Trafficware, 2011. Synchro 8.0 computer software.

Transportation Research Board National Research Council Washington, D.C. 2000. *Highway Capacity Manual 2000*. CD ROM.

Appendix A

Excerpts from Imperial County's Traffic Study and Report Policy

COUNTY OF IMPERIAL

DEPARTMENT OF PUBLIC WORKS

TRAFFIC STUDY AND REPORT POLICY

Date: March, 12, 2007

Revised June 29, 2007

APPROVALS:

WILLIAM S. BRUNET, P. E.

DIRECTOR OF PUBLIC WORKS

ROAD COMMISSIONER

URG HEUBERGER

PLANNING DIRECTOR

necessary to develop a traffic report that determines whether the traffic study general criteria have been met.

In the case of significant development, it may be necessary to hold one or more scope of work meetings which would be attended by a ICPDS staff, the County Traffic Engineer or other County Advisory Staff, the individual who will be responsible for preparing the traffic study report and the Traffic and/or Civil Engineer responsible for the report and its recommendations. The individual preparing the traffic study should be familiar with the project site and the local conditions which may affect any final conclusions and recommendations.

Listed below are the basic criteria that will be used to make the determination for providing a complete traffic study as a part of the project review process. The criteria are not a complete or exhaustive list, but they are intended to define when such a report is to be prepared and to indicate the necessary components of the study report to be submitted.

1. General Criteria

- a. Any project that adds more than 8% of the total existing vehicle trips on the adjacent road system at full build-out of the project.
- b. Any project that generates more than 400 daily residential trip ends, 800 commercial or industrial trip ends or 200 peak hour trip ends, as determined by the average trip rates contained in the ITE Trip Generation Informational Report or the Imperial County local exceptions in Section 2.
- c. Any project that has the potential to degrade an existing road section, an existing signalized intersection, or an existing unsignalized intersection to below the existing level of service or to cause it to be lower than a level of service (LOS)

unit, unless it is for urban infill development, within one half mile of major retail and commercial developmentt.

- Existing traffic on the adjacent road system and projected traffic on the adjacent road system, projected for a minimum of five (5) years, to project build-out, or both, depending on the project and the area; larger projects or high traffic generation may require future year build-out, currently Year 2030. Future CMP TIA reports would require additional traffic projection information.
- c. Traffic projections on the adjacent road system for both the project and "normal background growth" (demonstrated growth, as detailed in the general plan, or as agreed upon with County staff). Normally, traffic will be projected to Year 2030 or later for an updated future year condition.
- d. Traffic projections shall include the additional impact of undeveloped land or new development within an area surrounding the proposed development site (project) as agreed to by the County Director of Public Works, the County Planning Director and advisory staff.
- e. Projected impacts on intersections adjacent to or within the defined impact area of the project, using intersection capacity analysis Highway Capacity Manual Operations Delay Method. Right turn-on-red volumes and changes in signal timing can be incorporated in a signalized intersection analysis, but any signal timing changes must be specifically identified in the study recommendations with additional cautions or impact conclusions identified if the timing changes are not

m. Traffic counts, calculations, other basic information, and supporting data shall be included in an Appendix to the report or provided as a separate Technical Appendix. All actual traffic count data will be provided to the County in a useful summary form, digital and paper format, as specified by the County.

3. Analysis Methodology

The build-up method of traffic analysis will be followed, showing:

- a. Existing traffic;
- b. Existing traffic and normal background growth (rate and time to be agreed to by County staff);
- c. Existing traffic and normal background growth (see C. 3. b. above) and project build-out traffic;
- d. Existing traffic and normal background growth (see C. 3. b. above) and new development traffic (see C. 3. b. above);
- e. Existing traffic and 5 year normal background growth (see b. above) and new development (see b. above) and project build out, if longer than 5 years to build out of project.

If the study period to build-out is longer than 5 years, the future projection time period appropriate for a new development will be determined by the County staff. Significant projects may require a future projection time period of 20 years or General Plan build out. The future year is currently year 2030 as of the date of adopting this Policy. State Highway traffic projections will usually be carried to the year 2030 or to Caltrans current policy and procedures.

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Excerpts from Caltrans' Guide for the Preparation of Traffic Impact Studies



GUIDE FOR THE PREPARATION

OF

TRAFFIC IMPACT STUDIES

STATE OF CALIFORNIA DEPARTMENT OF TRANSPORTATION

December 2002

D. Travel Forecasting (Transportation Modeling)

The local or regional traffic model should reflect the most current land use and planned improvements (i.e., where programming or funding is secured). When a general plan build-out model is not available, the closest forecast model year to build-out should be used. If a traffic model is not available, historical growth rates and current trends can be used to project future traffic volumes. The TIS should clearly describe any changes made in the model to accommodate the analysis of a proposed project.

V. TRAFFIC IMPACT ANALYSIS METHODOLOGIES

Typically, the traffic analysis methodologies for the facility types indicated below are used by Caltrans and will be accepted without prior consultation. When a State highway has saturated flows, the use of a micro-simulation model is encouraged for the analysis (please note however, the micro-simulation model must be calibrated and validated for reliable results). Other analysis methods may be accepted, however, consultation between the lead agency, Caltrans and those preparing the TIS is recommended to agree on the data necessary for the analysis.

- A. <u>Freeway Segments</u> Highway Capacity Manual (HCM)*, operational analysis
- B. Weaving Areas Caltrans Highway Design Manual (HDM)
- C. <u>Ramps and Ramp Junctions</u> HCM*, operational analysis or Caltrans HDM, Caltrans Ramp Metering Guidelines (most recent edition)
- D. Multi-Lane Highways HCM*, operational analysis
- E. Two-lane Highways HCM*, operational analysis
- F. Signalized Intersections⁸ HCM*, Highway Capacity Software**, operational analysis, TRAFFIXTM**, Synchro**, see footnote 8
- G. <u>Unsignalized Intersections</u> HCM*, operational analysis, Caltrans Traffic Manual for signal warrants if a signal is being considered
- H. <u>Transit</u> HCM*, operational analysis
- I. Pedestrians HCM*
- J. Bicvcles HCM*

K. <u>Caltrans Criteria/Warrants</u> – Caltrans Traffic Manual (stop signs, traffic signals, freeway lighting, conventional highway lighting, school crossings)

L. <u>Channelization</u> – Caltrans guidelines for Reconstruction of Intersections, August 1985, Ichiro Fukutome

*The most current edition of the Highway Capacity Manual, Transportation Research Board, National Research Council, should be used.

**NOTE: Caltrans does not officially advocate the use of any special software. However, consistency with the HCM is advocated in most but not all cases. The Caltrans local development review units utilize the software mentioned above. If different software or analytical techniques are used for the TIS then consultation between the lead agency, Caltrans and those preparing the TIS is recommended. Results that are significantly different than those produced with the analytical techniques above should be challenged.

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⁸ The procedures in the Highway Capacity Manual "do not explicitly address operations of closely spaced signalized intersections. Under such conditions, several unique characteristics must be considered, including spill-back potential from the downstream intersection to the upstream intersection, effects of downstream queues on upstream saturation flow rate, and unusual platoon dispersion or compression between intersections. An example of such closely spaced operations is signalized ramp terminals at urban interchanges. Queue interactions between closely spaced intersections may seriously distort the procedures in" the HCM.

Appendix C
Excerpts from Imperial County's Circulation and Scenic Highways Element

CIRCULATION AND SCENIC HIGHWAYS ELEMENT

Prepared by:
Imperial County Planning & Development Services Department
801 Main Street
El Centro, CA 92243

in collaboration with the

Imperial County Public Works Department 155 South 11th Street El Centro, CA 92243

WILLIAM S. BRUNET, P.E. Director of Public Works

JURG HEUBERGER, AICP Planning & Development Services Director

Approved by: Board of Supervisors January 29, 2008

TABLE 5 IMPERIAL COUNTY STANDARD STREET CLASSIFICATION AVERAGE DAILY VEHICLE TRIPS

Road	Level of Service (LOS)					
Class	X-Section	Α	В	С	D	E
Expressway	154/210	30,000	42,000	60,000	70,000	80,000
Prime Arterial	106/136	22,200	37,000	44,600	50,000	57,000
Minor Arterial	82/102	14,800	24,700	29,600	33,400	37,000
Major Collector	64/84	13,700	22,800	27,400	30,800	34,200
(Collector)						
Minor Collector	40/70	1,900	4,100	7,100	10,900	16,200
(Local Collector)						
Local County	40/60	*	*	<1,500	*	*
(Residential)						
Local County	40/60	*	*	<200	*	*
(Residential Cul-de- Sac or Loop Street)						
Major Industrial Collector – (Industrial)	76/96	5,000	10,000	14,000	17,000	20,000
Industrial Local	44/64	2,500	5,000	7,000	8,500	10,000

^{*} Levels of service are not applied to residential streets since their primary purpose is to serve abutting lots, not carry through traffic. Levels of service normally apply to roads carrying through traffic between major trip generators and attractors.

Table 5 was originally developed for the County of San Diego by the San Diego County Department of Public Works in 1985 and compares ADT to levels of service (LOS) for various roadway classifications. Proposed functional classifications were then inserted into this table and right-of-way widths adjusted to match County of Imperial standards.

Transition Areas

The Circulation and Scenic Highways Element is the graphical reference guide which shows the present and planned street system, along with the classification of those streets. It is important to note that where there is a change from one classification to another along a certain street, the transition will occur in mid-block areas to preclude non-continuing lanes and intersections. The design criteria (design, speed, curve radii, etc.) for the higher classification shall generally take precedence through the transition area.

Appendix D

Excerpts from Caltrans' Guide for the Preparation of Traffic Impact Studies



GUIDE FOR THE PREPARATION

OF

TRAFFIC IMPACT STUDIES

STATE OF CALIFORNIA DEPARTMENT OF TRANSPORTATION

December 2002

Transition between LOS "C" and LOS "D" Criteria

(Reference Highway Capacity Manual)

BASIC FREEWAY SEGMENTS @ 65 mi/hr

LOS	Maximum Density (pc/mi/ln)	Minimum Speed (mph)	Maximum v/c	Maximum Service Flow Rate (pc/hr/ln)
A	11	65.0	0.30	710
В	18	65.0	0.50	1170
C	26	64.6	0.71	1680
 D	35	59.7	0.89	2090
E	45	52.2	1.00	2350

SIGNALIZED INTERSECTIONS and RAMP TERMINALS

LOS	Control Delay per Vehicle (sec/veh)	
A	≤ 10	
В	> 10 - 20	
 C	> 20 - 35	l
 D	> 35 - 55	
E	> 55 - 80	
F	> 80	

MULTI-LANE HIGHWAYS @ 55 mi/hr

LOS	Maximum Density (pc/mi/ln)	Minimum Speed (mph)	Maximum v/c	Maximum Service Flow Rate (pc/hr/ln)
Α	11	55.0	0.29	600
В	18	55.0	0.47	990
C	26	54.9	0.68	1430
D	35	52.9	0.88	1850
E	41	51.2	1.00	2100

Dotted line represents the transition between LOS "C" and LOS "D"

Appendix E
Excerpts from Imperial County's Circulation and Scenic Highways Element

CIRCULATION AND SCENIC HIGHWAYS ELEMENT

Prepared by:
Imperial County Planning & Development Services Department
801 Main Street
El Centro, CA 92243

in collaboration with the

Imperial County Public Works Department 155 South 11th Street El Centro, CA 92243

WILLIAM S. BRUNET, P.E. Director of Public Works

JURG HEUBERGER, AICP Planning & Development Services Director

Approved by: Board of Supervisors January 29, 2008 The County Director of Public Works shall review these transition areas and provide guidance in achieving this policy.

c. New or enlarged Roads:

Local Roads

The County shall require all new developments to provide for local roads to serve the direct access needs of abutting property. These streets should be designed with a discontinuous pattern to discourage through traffic. They generally should not intersect with arterial street classifications. Typical design features include two travel lanes with parking on both sides of the street. Local roads include loop streets and cul-de-sacs.

Regional Roads (Roads beyond the actual development project)

The County shall require that all new developments participate in the improvement of regional roads that may be impacted by the proposed development. The extent to which a project impacts regional roads is generally determined by a traffic study. In some cases however the County may have predetermined improvement requirements for certain road segments or road intersections. The new developments will be required to either make certain regional improvements or in the alternative contribute a "fair share" towards the cost of such improvements.

d. Level of Service Standards

As the County continues to grow, transportation demand management and systems management will be necessary to preserve and increase available roadway "capacity". Level of Service (LOS) standards are used to assess the performance of a street or highway system and the capacity of a roadway.

An important goal when planning the transportation system is to maintain acceptable levels of service along the federal and state highways and the local roadway network. To accomplish this, the California Department of Transportation (Caltrans), Imperial County and local agencies adopt minimum levels of service to determine future infrastructure needs.

Imperial County must provide and maintain a highway system with adequate capacity and acceptable levels of service to accommodate projected travel demands associated with the projected population growth within the Land Use Element. This can be accomplished by establishing minimum service levels for the designated street and conventional state highway system. Strategies that result in improvements to the transportation system, coupled with local job creation, will allow County residents to have access to a wide range of job opportunities within reasonable commute times.

The County's goal for an acceptable traffic service standard on an ADT basis and during AM and PM peak periods for all County-Maintained Roads shall be LOS C for all street segment links and intersections. These service values are defined by the 1985 or 2000 edition of the *Highway Capacity* Manual or any subsequent edition thereof. This policy shall acknowledge that the aforementioned level of service standards may not be obtainable on some existing facilities where abutting development precludes acquisition of additional right-of-way needed for changes in facility classification.

In order to achieve the level of service goals in the previous policy, the County shall develop and institute a long-range funding program in which new land development shall bear the major burden of the associated costs and improvement requirements.

e. Design Standards

The County shall adopt design standards for all streets in accordance with their functional classifications and recognized design guidelines. In developing these standards, the County shall consider the design standards of Caltrans and the American Association of State and Highway Transportation Officials (AASHTO). All streets within the County shall be designed in accordance with the adopted County of Imperial Design Standards. Typical cross sections and design criteria for the various street classifications are shown as an attachment to this document.

f. Private Streets

The County may permit construction of private streets within individual development projects (gated community). providing the following are addressed:

- They are designed geometrically and structurally to meet County standards.
- Only project occupants are served (gated community).
- Emergency vehicle access requirements are satisfied.
- The streets do not provide a direct through route between public streets.
- The Homeowners Associations and/or property owners provide an acceptable program for financing regular street maintenance.
- If the private street is permitted with a waiver of any of the above standards, any future requests to make the private street a public street shall require that all adjacent property owners provide and pay for all improvements and right of way required to bring the street to current public street or road standards. This includes road width, right of way widths and structural section. In no circumstance shall the County pay for any costs to upgrade a private street to public street standards if the above-mentioned requirements were waived at the request of the original developer or subdivider.

Appendix F

Traffic Impact Significance Criteria from Imperial area EIRs

4.6.2 Impact Significance Criteria

Significance Criteria

The significance criteria summarized in Table 4.6-2 by Linscott, Law and Greenspan Engineers is based upon the City of El Centro and the County of Imperial's goal for intersections and roadway segments to operate at LOS C or better. In general, a degradation in LOS from LOS C or better to LOS D or worse is considered a significant direct impact. A cumulative impact can occur if the intersection or segment LOS is already operating below City/County standards and the project increases the delay by more than 2 seconds or the v/c ratio by more than 0.02.

Table 4.6-2 Significance Criteria							
	INTERSECTIONS						
Existing	Existing + Project	Existing + Project + Cumulative Projects	Impact Type				
LOS ¹ C or better	LOS C or better	LOS C or better	None				
LOS C or better	LOS D or worse	-	Direct				
LOS D	LOS E or F	-	Direct				
LOS E	LOS F	-	Direct				
Any LOS	Project does not degrade LOS and adds > 2.0 seconds of delay	LOS E or worse	Cumulative				
Any LOS	Project does not degrade LOS and adds < 2.0 seconds of delay		None				
	SEGMENTS						
Existing	Existing + Project	Existing + Project + Cumulative Projects	Impact Type				
LOS C or better	LOS C or better	LOS C or better	None				
LOS C or better	LOS D or worse	-	Direct ²				
LOS D	LOS E or F	-	Direct				
LOS E	LOS F	-	Direct				
Any LOS	LOS E or worse and $v/c^3 > 0.02$	LOS E or worse	Cumulative				
Any LOS	LOS E or worse and $v/c^3 < 0.02$	Any LOS	None				

Source: Linscott, Law & Greenspan, Engineers (July 2004)

Notes:

- 1. LOS: Level of Service
- 2. Exception: post-project segment operation is D and intersections along segment are D or better, no significant impact.
- 3. V/C: Volume to Capacity Ratio

In addition the project would have a significant impact if:

• It would substantially increase hazards due to a design feature (e.g., sharp curves or dangerous intersections) or incompatible uses (e.g., farm equipment).

TABLE 5.1
SIGNIFICANCE CRITERIA

Intersections							
Existing	Existing + Project	Existing + Project + Cumulative Projects	Impact Type				
LOS C or better	LOS C or better	LOS C or better	None				
LOS C or better	LOS C or better and project adds < 2.0 seconds of delay	LOS D or worse	None				
LOS C or better	LOS C or better and project adds > 2.0 seconds of delay	LOS D or worse	Cumulative				
LOS C or better	LOS D or worse	LOS D or worse	Direct				
LOS D	LOS D and project adds < 2.0 seconds of delay	LOS D or worse	None				
LOS D	LOS D and project adds > 2.0 seconds of delay	LOS D or worse	Cumulative				
LOS D	LOS E or F	LOS E or F	Direct				
LOS E	LOS E and project adds < 2.0 seconds of delay	LOS E or F	None				
LOS E	LOS E and project adds > 2.0 seconds of delay	LOS E or F	Cumulative				
LOS E	LOS F	LOS F	Direct				
LOS F	Project add < 2.0 seconds of delay	LOS F	None				
LOS F	Project adds 2.0 to 9.9 seconds of delay	LOS F	Cumulative				
LOS F	Project adds 10.0 or more seconds of delay	LOS F	Direct				
	Segments						
Existing	Existing + Project	Existing + Project + Cumulative Projects	Impact Type				
LOS C or better	LOS C or better	LOS C or better	None				
LOS C or better	LOS or better and project increases V/C by < 0.02	LOS D or worse	None				
LOS C or better	LOS C or better and project increase V/C by >0.02	LOS D or worse	Cumulative				
LOS C or better	LOS D or worse	LOS D or worse	Direct ¹				
LOS D	LOS D and project increases V/C by < 0.02	LOS D or worse	None				
LOS D	LOS D and project increases V/C by > 0.02	LOS D or worse	Cumulative				
LOS D	LOS E or F	LOS E or F	Direct				
LOS E	LOS E and project increases V/C by < 0.02	LOS E or F	None				
LOS E	LOS E and project increases V/C by > 0.02	LOS E or F	Cumulative				
LOS E	LOS F	LOS F	Direct				
LOS F	Project increases V/C by < 0.02	LOS F	None				
LOS F	Project increases V/C by > 0.02 and < 0.09	LOS F	Cumulative				
LOS F	Project increases V/C by > 0.09	LOS F	Direct				

Notes: LOS = Level of Service; V/C = Volume to Capacity Ratio; ¹ Exception: If Existing + Project segment operation is $LOS \ D$ and intersections along segment are $LOS \ D$ or better, then there is no significant impact.

In addition to the above listed projects, the Lerno/Verhaegen project was recently submitted and is currently starting the CEQA process. This project is listed for information purposes but cannot be analyzed in cumulative terms. The following is a brief description based on the limited information available for this project.

Lerno-Verhaegen Specific Plan is proposed to be a mixed-use development of 2,708 dwelling units. The project consists of 680 acres on the west side of the City of El Centro. The project includes a zone change, Tentative Map, an amendment of the City's General Plan and an annexation.

Individual traffic assignments were completed for each cumulative project. Figure 2-7 depicts the total cumulative project traffic volumes in the area. Figure 2-8 shows the existing + project + cumulative projects traffic volumes for the vicinity. Appendix D of this Mitigated Negative Declaration contains the individual cumulative project traffic assignments.

Significance Criteria

The significance criteria summarized in Table 2-7 by Linscott, Law and Greenspan, engineers is based upon the County of Imperial's goal for intersections and roadway segments to operate at LOS C or better. Intersections or segments operating at LOS D, E or F are unacceptable and therefore constitute a significant impact.

Table 2-7 – Significance Criteria								
	INTERSECTIONS							
Existing	Existing + Project	Existing + Project + Cumulative Projects	Impact Type					
LOS ¹ C or better	LOS C or better	LOS C or better	None					
LOS C or better	LOS D or worse	-	Direct					
LOS D	LOS E or F	-	Direct					
LOS E	LOS F	-	Direct					
Any LOS	Project does not degrade LOS and adds > 2.0 seconds of delay	LOS E or worse	Cumulative					
Any LOS	Project does not degrade LOS and adds < 2.0 seconds of delay	Any LOS	None					
	SEGMENTS							
Existing	Existing + Project	Existing + Project + Cumulative Projects	Impact Type					
LOS C or better	LOS C or better	LOS C or better	None					
LOS C or better	LOS D or worse	-	Direct ²					
LOS D	LOS E or F	=	Direct					
LOS E	LOS F	-	Direct					
Any LOS	LOS E or worse and $v/c^3 > 0.02$	LOS E or worse	Cumulative					
Any LOS	LOS E or worse and $v/c^3 < 0.02$	Any LOS	None					

Source: LL&G, July 2004.

Notes:

- 1. LOS: Level of Service
- 2. Exception: post-project segment operation is D and intersections along segment are D or better, no significant impact.
- 3. V/C: Volume to Capacity Ratio

TABLE 5-1
SIGNIFICANCE CRITERIA

Intersections						
Existing	Existing + Project	Existing + Project + Cumulative Projects	Impact Type			
LOS ^a C or better	LOS C or better	LOS C or better	None			
LOS C or better	LOS D or worse	_	Direct			
LOS D	LOS D and adds 2.0 seconds or more of delay	LOS D or worse	Cumulative			
LOS D	LOS E or F		Direct			
LOSE	LOS F		Direct			
LOS F	LOS F and delay increases by ≥ 10.0 seconds	LOS F	Direct			
Any LOS	Project does not degrade LOS and adds 2.0 to 9.9 seconds of delay	LOS E or worse	Cumulative			
Any LOS	Project does not degrade LOS and adds < 2.0 seconds of delay	Any LOS	None			
	SEGMENTS	Section 1995				
Existing	Existing + Project	Existing + Project + Cumulative Projects	Impact Type			
LOS C or better	LOS C or better	LOS C or better	None			
LOS C or better	LOS C or better and $v/c^b > 0.02$	LOS D or worse	Cumulative			
LOS C or better	LOS D or worse		Direct			
LOS D	LOS D and v/c > 0.02	LOS D or worse	Cumulative			
LOS D	LOS E or F		Direct			
LOS E	LOS F	_	Direct			
LOS F	LOS F and v/c increases by > 0.09	LOS F	Direct			
Any LOS	LOS E or worse and v/c 0.02 to 0.09	LOS E or worse	Cumulative			
Any LOS	LOS E or worse and v/c < 0.02	Any LOS	None			

Source: Linscott, Law & Greenspan, Engineers

Footnotes:

b. Volume to Capacity Ratio

a. Level of Service

Appendix G

Excerpts from Imperial County Circulation Element

CIRCULATION AND SCENIC HIGHWAYS ELEMENT

Prepared by:
Imperial County Planning & Development Services Department
801 Main Street
El Centro, CA 92243

in collaboration with the

Imperial County Public Works Department 155 South 11th Street El Centro, CA 92243

WILLIAM S. BRUNET, P.E. Director of Public Works

JURG HEUBERGER, AICP Planning & Development Services Director

Approved by: Board of Supervisors January 29, 2008

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(County of Imperial)



LEGEND

- Interstate (0-Lanes)

- massu- (U-Lanes)
- massu- (U-Lanes)
- Prime Arteriol (G-Lanes divided)
- Minor Arteriol (G-Lanes divided)
- Major Callestor-Collector (4-Lanes)

- Minor Collector-Local Collector (2-Lanes)

---- - Proposed Future Alignment

- Cities & Towns

MEXICO

Imperial County Circulation Element Plan Southern Section (SHEET 4 of 4)



Figure 1c

Segment Location	2003 Classification	Year 2002 ADT Volume ^a	Year 2005 ADT Volume ^a	Year 2025 ADT Volume ^c	25 Year Total Growth Factor ^d	Year 2050 ADT Volume	Year 2050 Recommended Classification (# of Lanes)	2050 LOS ^e
Meloland/SR-115	Major Collector						Major Collector (4)	
Albright Road								
SR-111/SR-115 SR-115/Butters	Minor Collector Major Collector						Minor Collector (2) Major Collector (4)	
Anderholt Road	wajor Concotor						Wajor Collector (1)	
Evan Hewes (S-80)/Hunt	Minor Collector						Minor Collector (2)	
Hunt/Carr Andre Road	Major Collector						Major Collector (4)	
Forrester/End	Minor Collector						Minor Collector (2)	
Anza Road								
Pulliam/Rockwood Rockwood/Calexico	Local Prime Arterial						Minor Collector (2) Prime Arterial (6-divided)	
Calexico/Barbara Worth	Prime Arterial						Prime Arterial (6-divided)	
Aten Road							Ì	
End/Forrester Engrector/Austin	Minor Collector						Minor Collector (2)	
Forrester/Austin East Imperial City Limits/Dogwood	Minor Arterial Prime Arterial	7,300	8,450	39,000	1.13	44,500	Minor Arterial (6-divided) Prime Arterial (6-divided)	С
Dogwood/SR-111	Prime Arterial	.,500	5, 100	20,000		,000	Prime Arterial (6-divided)	
Proposed/SR-111/River	None						Prime Arterial (6-divided)	
Austin Road McCabe/Wahl	Local					1	Prime Arterial (6-divided)	
Proposed Wahl/SR-98	None						Prime Arterial (6-divided)	
Evan Hewes Hwy/McCabe	Major Collector						Prime Arterial (6-divided)	
Aten/Evan Hewes Hwy	Minor Arterial						Prime Arterial (6-divided)	
Keystone/Aten SR-86/Keystone	Major Collector Minor Collector						Prime Arterial (6-divided) Prime Arterial (6-divided)	
Bannister Road	THE COMMON OF						· ······o / iitonai (o aiviaoa)	
SR-86/Brandt	Major Collector						Major Collector (4)	
Barbara Worth Road Zenos/Evan Hewes (S-80)	Minor Collector						Major Collector (4)	
Evan Hewes Hwy/Anza	Major Collector						Major Collector (4)	
Baughman Road	_							
Garvey/Lack Lack/SR-86	Minor Collector Major Collector						Minor Collector (2) Major Collector (4)	
Bell Road	iviajor Collector						iviajor Collector (4)	
Alamo/Evan Hewes Hwy	Minor Collector						Minor Collector (2)	
Bennett Road	Miner Cellenter		ſ			1	Minan Callantan (0)	
Havens/Ross Best Road	Minor Collector						Minor Collector (2)	
Rutherford/Brawley	Minor Arterial						Minor Arterial (4)	
Blair Road								
Pound/Sinclair Peterson/Lindsey	Minor Collector Major Collector						Minor Collector (2) Major Collector (4)	
Lindsey/SR-115	Major Collector						Major Collector (4)	
SR-115/Yocum	Local						Major Collector (4)	
Blais Road	Miner Cellenter		ſ			1	Minor College	
Wieman/Forrester Boarts Road (S26)	Minor Collector						Minor Collector	
Westmorland/Kalin	Major Collector						Major Collector (4)	
Boley Road								
Westmorland/Huff Bonds Corner Road	Minor Collector						Minor Collector (2)	
Holtville/I-8	Major Collector						Major Collector (4)	
I-8/SR-98	Minor Arterial						Minor Arterial (4)	
Bonesteele Road	Minor O-II						Minor Collector (0)	
Kumberg/SR-98 Bornt Road	Minor Collector						Minor Collector (2)	
Verde School/SR-98	Minor Collector						Minor Collector (2)	
Bowker Road			ı				, ,	
		1				1	Major Collegtor (4)	1
Evan Hewes Hwy/I-8 I-8/SR-98	Major Collector Minor Arterial						Major Collector (4) Expressway (6)	+

Segment Location	2003 Classification	Year 2002 ADT Volume ^a	Year 2005 ADT Volume ^a	Year 2025 ADT Volume ^c	25 Year Total Growth Factor ^d	Year 2050 ADT Volume	Year 2050 Recommended Classification (# of Lanes)	2050 LOS°
Bowles Road Riley/Lyerly	Minor Collector						Minor Collector (2)	
Boyd Road	Willion Collector						Willion Collector (2)	
Wiest/SR-78	Local						Minor Collector (2)	
SR-115/Highline	Local						Minor Collector (2)	
Highline/End	Minor Collector						Minor Collector (2)	
Brandt Road							M: 0 II (0)	
Sinclair/Lindsey Lindsey/Eddins	Local Minor Collector						Minor Collector (2) Minor Collector (2)	+
Eddins/Webster	Minor Collector						Minor Collector (2)	+
Bridenstein Road	Willion Collector						William Collector (2)	
Proposed SR-78/Hartshorn							Minor Collector (2)	
Hartshorn/Bonds Corner	Minor Collector						Minor Collector (2)	
Brockman Road (S30)						,		
McCabe/SR-98	Major Collector						Major Collector (4)	
Butters Road (S32) Gonder/SR-78	Prime Arterial						Prime Arterial (6)	Α
Bowles/Albright	Local						Major Collector (4)	A
Albright/SR-78	Major Collector						Major Collector (4)	
Cady Road	major comocio.						major concotor (1)	
Pellett/SR-86	Major Collector						Major Collector (4)	
Cambell Road						,		
Jessup/Derrick	Major Collector						Major Collector (4)	
Derrick/Drew	Major Collector						Major Collector (4)	
Carey Road SR-86/Dogwood	Minor Collector						Minor Collector (2)	
Carr Road	WIII OF COILECTOR						Willion Collector (2)	
Barbara Worth/SR-7	Major Collector						Minor Arterial (4)	
Carter Road	,							
Kalin/Forrester	Minor Collector						Major Collector (4)	
Casey Road	I w						A	
Dickerman/SR-78	Minor Collector						Minor Collector (2)	
SR-78/Worthington Proposed Worthington/Norrish	Minor Collector None						Major Collector (4) Major Collector (4)	
Chick Road	None						iviajor conector (4)	
El Centro/Pitzer	Prime Arterial						Prime Arterial (6)	
Pitzer/Barbara Worth	Major Collector						Major Collector (4)	
Clark Road								
El Centro/SR-98	Minor Arterial						Minor Arterial (4)	
North El Centro City Limits/Worthington	Major Collector	2,100	2,430 930	12,550	1.64	21,000	Major Collector (4)	B A
Worthington/Larsen Cole Road	Minor Collector	800	930	6,220	1.64	10,500	Major Collector (4)	А
Dogwood/Calexico	Prime Arterial						Prime Arterial (6-divided)	
East Calexico City Limits/SR-98	Minor Arterial	9,700	11,230	18,340	1.64	30,500	Prime Arterial (6-divided)	В
Connelly Road								
Vencill/Van Der Linden	Minor Collector						Minor Collector (2)	
Cooley Road							(2)	
Worthington/Gillett	Minor Collector						Minor Collector (2)	
Corn Road Bowles/Eddins	Minor Collector						Minor Collector (2)	
Correll Road	WILLOU COLLECTOR						Willion Collector (2)	
Dogwood/SR 111	Minor Arterial						Minor Arterial (4)	
Cross Road							()	
Imperial (City)/Villa	Minor Collector						Minor Collector (2)	
Davis Road								
Gillespie/Schrimpf	Major Collector					<u> </u>	Major Collector (4)	1
Proposed Schrimpf/Sinclair	Major Collector	<u> </u>					Major Collector (4)	
Dearborn Road Harrigan/Wormwood	Minor Collector						Minor Collector (2)	
Derrick Road	WILLOW COMECTOR						WILLOT COLLECTOR (2)	
Evan Hewes Hwy/Wixom	Minor Collector						Minor Collector (2)	
Dickerman Road SR-115/Butters	Minor Collector						Minor Collector (2)	
OIX-110/DUILEIS	IVIII IOI COIIECIOI					1	willion Collector (2)	

Segment Location	2003 Classification	Year 2002 ADT Volume ^a	Year 2005 ADT Volume ^a	Year 2025 ADT Volume ^c	25 Year Total Growth Factor ^d	Year 2050 ADT Volume	Year 2050 Recommended Classification (# of Lanes)	2050 LOS ^e
Diehl Road Westside/Drew	Minor Collector						Minor Collector (2)	
Drew/Harrigan	Major Collector						Prime Arterial (6)	
Proposed Harrigan/Silsbee	Major Collector						Prime Arterial (6)	
Dietrich Road								
Rutherford/Shank	Minor Collector						Major Collector (4)	
Proposed Shank/SR-78 Doetsch Road	None						Major Collector (4)	
Elder/SR-86	Minor Collector						Minor Collector (2)	
Dogwood Road (S31)*							\ /	
Proposed Lindsey/Hovley	None						Prime Arterial (6-divided)	
Brawley/SR-98 Dowden Road	Prime Arterial						Prime Arterial (6-divided)	
Proposed Forrester/Gentry	None						Local Collector (2)	
Gentry/Kershaw	None						Prime Arterial (6)	
Kershaw/Butters	Minor Collector						Prime Arterial (6)	
Drew Road (S29) Evan Hewes/SR-98	Drimo Artorial						Drimo Artorial (6 divided)	
Dunaway Road	Prime Arterial						Prime Arterial (6-divided)	
I-8/Evan Hewes Hwy	Major Collector	900	1,040	2,756	1.64	4,500	Major Collector (4)	Α
Eady Road						,		
Willoughby/Cole	Minor Collector						Minor Collector (2)	
Eddins Road (S30) Gentry/SR-111(Calipatria City Limits)	Major Collector						Major Collector (4)	
Edgar Road	Major Collector						Major Concotor (1)	
Pierle/Forrester	Minor Collector						Minor Collector (2)	
Elder Road	MinanOallaatan						Minan Callanton (0)	
Doetsch/Cady English Road	Minor Collector						Minor Collector (2)	
Sinclair/Wilkins	Minor Collector						Minor Collector (2)	
Erskine Road							\	
Wheeler/Payne	Minor Collector						Minor Collector	
Evan Hewes Hwy (S80) Imperial Hwy/El Centro	Prime Arterial						Prime Arterial (6-divided)	
El Centro/SR-115	Prime Arterial						Prime Arterial (6-divided)	
SR-115/End	Prime Arterial						Prime Arterial (6-divided)	
Fawcett Road								
Dogwood/Meadows Ferrell Road	Minor Collector						Major Collector (4)	
Kubler/SR-98	Major Collector						Major Collector (4)	
SR-98/Anza	Minor Collector						Minor Collector (2)	
Fifield Road								
SR-78/Streiby	Minor Collector						Minor Collector (2)	
Fisher Road Drew/Pulliam	Minor Collector						Minor Collector (2)	
Flett Road	William Collector	l					William Collector (2)	
Wilkinson/Wirt	Minor Collector						Minor Collector (2)	
Forrester Road (S30)	Mana						Drive - Autorial (C. divide d)	
Proposed Sinclair/Walker Walker/Westmorland	None Major Collector	-					Prime Arterial (6-divided) Prime Arterial (6-divided)	
Westmorland/McCabe	Prime Arterial						Prime Arterial (6-divided)	
McCabe/Hime	Minor Collector						Prime Arterial (6-divided)	
Proposed Hime/River	Minor Collector	4.55	4.00-	0.00		45.000	Prime Arterial (6-divided)	_
North Westmorland City Limits/Gentry Foulds Road	Major Collector	1,200	1,390	9,000	1.64	15,000	Prime Arterial (6-divided)	Α
Pellett/Lack	Minor Collector						Minor Collector (2)	
Fredericks Road	3000101						23.100101 (2)	
Loveland/SR-111	Minor Collector						Minor Collector (2)	
Frontage Road	Maior C-II						Major Cells -t (4)	
Ross/Brawley (City) Garst Road	Major Collector						Major Collector (4)	
Sinclair/McDonald	Minor Collector						Minor Collector (2)	
Garvey Road	Minor Collector						Minor Collector (2)	

Segment Location	2003 Classification	Year 2002 ADT Volume ^a	Year 2005 ADT Volume ^a	Year 2025 ADT Volume ^c	25 Year Total Growth Factor ^d	Year 2050 ADT Volume	Year 2050 Recommended Classification (# of Lanes)	2050 LOS°
Gentry Road Sinclair/Walker	Major Callagtor						Major Collector (4)	
Gillespie Road	Major Collector						Major Collector (4)	
Davis/Wilkins	Minor Collector						Minor Collector (2)	
Gillett Road	Miner Cellecter	1					Miner Cellenter (0)	
Cooley/Bowker Gonder Road	Minor Collector						Minor Collector (2)	
Proposed New River/SR-115	None						Major Collector (4)	
SR-115/Butters	Local						Minor Collector (2)	
Butters/Green Green/Highline	Minor Collector Major Collector						Minor Collector (2) Major Collector (4)	
Gowling Road	Major Collector						iviajor conector (4)	
Norrish/Zenos	Minor Collector						Major Collector (4)	
Green Road	Major Collector						Major Callagtor (4)	
SR-78/Gonder Griffin Road	Major Collector						Major Collector (4)	
Wiest/SR-115	Minor Collector						Minor Collector (2)	
Grumbles Road								
James/Meloland Gullett Road	Minor Collector						Minor Collector (2)	
Worthington/Aten	Minor Collector						Minor Collector (2)	
Gutherie Road								
Wienert/Worthington	Minor Collector						Minor Collector (2)	
Proposed Worthington/Hackleman Hackleman Road	Minor Collector						Minor Collector (2)	
Low/Forrester	Minor Collector						Minor Collector (2)	
Hardy Road							()	
Dunaway/Jeffrey	Major Collector						Major Collector (4)	
Jeffrey/Hyde Hyde/Jessup	Major Collector Major Collector						Major Collector (4) Major Collector (4)	
Harrigan Road	Major Collector						Iviajor Conector (4)	
Diehl/Dearborn	Minor Collector						Minor Collector (2)	
Harris Road							M : 0 (1)	
Austin/SR-86 SR-86/McConnel	Local Major Collector						Major Collector (4) Major Collector (4)	
McConnell/Highline	Minor Collector						Major Collector (4)	
Hart Road								
Wiest/SR-115	Minor Collector						Minor Collector (2)	
Hartshorn Road Bridenstein/Proposed Bridenstein	Minor Collector						Minor Collector	
Haskell Road	Willion Collector						William Collector	
Evan Hewes Hwy/End	Minor Collector						Minor Collector (2)	
Hastain Road Taecker/SR-78	Minor Collector						Minor Collector (2)	
Young/Dickerman	Minor Collector Minor Collector						Minor Collector (2) Minor Collector (2)	
Havens Road								
Haskell/Bennett	Minor Collector						Minor Collector (2)	
Hetzel Road Westmorland/Huff	Minor Collector						Minor Collector (2)	
Heber Road	WIII IOI COILECTOI						Willion Collector (2)	
La Brucherie/SR-86	Local						Minor Collector (2)	
SR-111/Anderholt	Minor Arterial	N/A	2,040	16,700	1.64	27,500	Prime Arterial (6-divided)	В
Anderholt/Keffer Keffer/Vencill	Major Collector Minor Collector						Major Collector (4) Major Collector (4)	
Highline Road (S33)							major Odirottor (4)	
Proposed SR-78/Gonder	None						Major Collector (4)	
Gonder/Kavanuagh	Major Collector						Major Collector (4)	1
Proposed Kavanaugh/I-8 Holt Road. (\$32)	None						Major Collector (4)	
Gonder/Holtville city limits	Prime Arterial						Prime Arterial (6-divided)	
Hoskins Road						1		
SR-86/Steiner Hovley Road	Minor Collector						Minor Collector	
Rutherford/Brawley	Major Collector						Major Collector (4)	

Segment Location Huff Road	2003 Classification	Year 2002 ADT Volume ^a	Year 2005 ADT Volume ^a	Year 2025 ADT Volume ^c	25 Year Total Growth Factor ^d	Year 2050 ADT Volume	Year 2050 Recommended Classification (# of Lanes)	2050 LOS ^e
Imler/Evan Hewes Hwy	Major Collector						Major Collector (4)	
Hunt Road Barbara Worth/Bonds Corner Bonds Corner/Van Der Linden	Major Collector Minor Collector						Major Collector (4) Minor Collector (2)	
Huston Road Dogwood/McConnell	Minor Collector						Minor Collector (2)	
Imler Road Huff/Forrester International Road	Major Collector						Major Collector (4)	
Noffsinger/Pound Irvine Road	Minor Collector						Minor Collector (2)	
Shank/End James Road Paleb/Even House House	Minor Collector						Minor Collector (2)	
Ralph/Evan Hewes Hwy Jasper Road Calexico/Anderholt	Minor Collector Major Collector						Minor Collector (2) Expressway (6)	
Proposed Anderholt/ SR-7 Jeffery Road	None						Expressway (6)	
Evan Hewes Hwy/Hardy Kaiser Road Wirt/Albright	Minor Collector Minor Collector						Minor Collector (2) Minor Collector (2)	
Kalin (S26) Sinclair/SR-78/86	Major Collector						Major Collector (4)	
SR-78/86/Webster Kamm Road	Minor Collector						Minor Collector (4)	
River/SR-115 SR-115/Holt Keffer Road	Local Minor Collector						Prime Arterial (6) Major Collector (4)	
SR-98/King Kershaw Road	Major Collector						Major Collector (4)	
Yocum/Rutherford Keystone Road (S27)	Minor Collector						Minor Collector (2)	
Forrester/SR-111 SR-111/Highline King Road	Prime Arterial Major Collector						Expressway (6) Expressway (6)	
Orchard/Keffer Kloke Road	Major Collector						Major Collector (4)	
Willoughby/Calexico Kramar Road	Major Collector						Major Collector (4)	
Drew/Forrester Kubler Road Drew/Clark	Major Collector Minor Collector						Major Collector (4) Minor Collector (2)	
Kumberg Road Bonesteele/Miller	Minor Collector						Minor Collector (2)	
La Brucherie Road El Centro city limits/Kubler Larsen/Murphy	Major Collector Minor Collector						Major Collector (4) Minor Collector (2)	
Murphy/Imperial city limits Lack Road	Minor Collector						Minor Collector (2)	
Lindsey/Blais Larsen Road	Minor Collector						Minor Collector (2)	
Forrester/SR-86 SR-86/Clark	Major Collector Minor Collector						Major Collector (4) Minor Collector (2)	
Lavigne Road SR-98/Bowker Proposed Bowker/Barbara Worth	Prime Arterial Prime Arterial						Prime Arterial (6) Prime Arterial (6)	
Liebert Road Wixom/Rd 8018	Minor Collector						Minor Collector (2)	
Proposed Road 8018/SR-98 Lindsey Road Lack/Wiest	Minor Collector Minor Collector						Minor Collector (2) Minor Collector (2)	
Lack/West Loveland Road Fredericks/Monte	Minor Collector						Minor Collector (2) Minor Collector (2)	
Low Road Hackleman/Evan Hewes Hwy	Minor Collector						Minor Collector (2)	

Segment Location	2003 Classification	Year 2002 ADT Volume ^a	Year 2005 ADT Volume ^a	Year 2025 ADT Volume ^c	25 Year Total Growth Factor ^d	Year 2050 ADT Volume	Year 2050 Recommended Classification (# of Lanes)	2050 LOS ^e
Bowles/Eddins	Minor Collector						Minor Collector (2)	
Lyons Road Drew/Nichols	Minor Collector						Major Collector (4)	
Proposed Nichols/La Brucherie	None						Major Collector (4)	
Main ST (Niland)								
SR-111/Blair Martin Road	Major Collector						Major Collector (4)	
Baughman/7th	Minor Collector						Minor Collector (2)	
7th/Bannister	Local						Minor Collector (2)	
Mead Road Dogwood/McConnell	Minor Collector						Minor Collector (2)	
Meadows Road	Willion Collector						Millior Collector (2)	
Heber/Calexico (City)	Major Collector						Major Collector (4)	
Meloland Road	Miner Cellector	1				1	Minor College (0)	
Worthington/Correll Proposed Correll/SR-98	Minor Collector Minor Collector						Minor Collector (2) Minor Collector (2)	
McCabe Road	Anna. Comocior						0001(12)	
Silsbee/La Brucherie	Major Collector	N1/2	000	47.070	4.01	00.500	Prime Arterial (6-divided)	_
La Brucherie/SR-111 SR-111/SR-7	Minor Arterial Major Collector	N/A	200	17,270	1.64	28,500	Prime Arterial (6-divided) Prime Arterial (6-divided)	В
McConnell Road	iviajor concetor						Tilline Attendi (o divided)	
SR-78/Evan Hewes Hwy	Major Collector						Major Collector (4)	
McDonald Road Garst/SR-111	Minor Collector						Minor Collector (2)	
SR-111 TO Rd 8041	Minor Collector						Minor Collector (2)	
McKim Road) /	
Harris/Ralph	Minor Collector						Minor Collector (2)	
Miller Road (S33) I-8/Kumberg	Minor Collector						Minor Collector (2)	
I-8/SR-115	Major Collector	200	230	5,250	1.64	9,000	Major Collector (4)	Α
SR-115/Kavanaugh	Major Collector	100	120	5,300	1.64	9,000	Major Collector (4)	Α
Monte Road Pellett/Loveland	Minor Collector						Minor Collector (2)	
Neckel Road	minor consoco.						1111161 CONCOTO! (2)	
Austin/Clark	Minor Collector						Minor Collector (2)	
Nichols Road McCabe/Lyons	Minor Collector						Minor Collector (2)	
Noffsinger Road	miner consector						er eeneeter (2)	
SR-111/McDonald	Minor Collector						Minor Collector (2)	
Norrish Road Gowling/Holt	Minor Collector						Minor Collector (2)	
Holt/Highline	Local						Major Collector (4)	
Highline/End	Major Collector						Major Collector (4)	
Orchard Road (S32)/ SR 7	Major Collector	700	910	50.740	1.12	E7 E00	Everesousy (6)	С
King/McCabe McCabe/l-8	Major Collector	900	810 1,040	49,000	1.13	57,500 56,000	Expressway (6) Expressway (6)	C
Holtville/I-8	Minor Arterial		,-	-,			Prime Arterial (6-divided)	
I-8/Connelly	Major Collector						Major Collector (4)	
Orr Road Baughman/SR-86	Minor Collector						Minor Collector (2)	
Park Road	Willion Collector						Willion Collector (2)	
Proposed Dowden/Williams	None						Major Collector (4)	
Williams/Rutherford Proposed Rutherford/Dietrich	Minor Collector None						Major Collector (4) Major Collector (4)	1
Parker Road	INOTIC						iviajoi Collectol (4)	
Ross/Gilllett	Minor Collector						Minor Collector (2)	
Payne Road	Minor Collector						Minor Collector (2)	
Huff/Erskine Pellett Road	Minor Collector						Minor Collector (2)	4
Foulds/Monte	Minor Collector						Minor Collector (2)	
Proposed Monte/Imler	Minor Collector						Minor Collector (2)	
Pickett Road								

TABLE 3 IMPERIAL COUNTY PROJECTED STREET SEGMENT CONFIGURATIONS AND **VOLUMES** (continued)

Segment Location	2003 Classification	Year 2002 ADT Volume ^a	Year 2005 ADT Volume ^a	Year 2025 ADT Volume ^c	25 Year Total Growth Factor ^d	Year 2050 ADT Volume	Year 2050 Recommended Classification (# of Lanes)	2050 LOS°
Pierle Road Edgar/Wheeler	Minor Collector						Minor Collector(2)	
Proposed Jasper/Willoughby	None						Major Collector (4)	
Chick/SR-86	Major Collector						Major Collector (4)	
SR-86/Jasper	Minor Collector						Major Collector (4)	
Pound Road Davis/International	Major Collector						Major Collector (4)	
International/Noffsinger	Minor Collector						Minor Collector (2)	+
Pulliam Road							· /	
Fisher/ SR-98	Minor Collector						Minor Collector (2)	
Ralph Road Imperial (City)/Dogwood	Major Collector						Major Collector (4)	
Dogwood/Mckim	Minor Collector						Minor Collector (2)	+
Riley Road							` ,	
Bowles/Eddins	Minor Collector						Minor Collector	
Rockwood Road Proposed River/Lyons	Minor Collector						Prime Arterial (6)	
Lyons SR-98	Minor Collector						Prime Arterial (6)	+
ŚR-98/Anza	Major Collector						Major Collector	
Ross Road	Maior College	4.500	4.740	0.040	4.04	4.000	Maior Callantan (4)	^
Drew/Bennett Drew/Austin	Major Collector Major Collector	1,500	1,740	2,310	1.64	4,000	Major Collector (4) Major Collector (4)	Α
El Centro/SR-111	Minor Arterial						Minor Arterial (4)	
SR-111/Mets	Local	N/A	560	2,120	1.64	3,500	Minor Collector (2)	В
Ruegger Road							M: 0 !! . (0)	
Kalin/SR-111 Rutherford Road (S26)	Minor Collector						Minor Collector (2)	
Proposed Banister/Kalin							Major Collector (4)	
Kalin/Butters	Major Collector						Major Collector (4)	
Butters/Irvine	Minor Collector						Minor Collector (2)	
Schartz Road Proposed SR-86/Dogwood	None						Major Collector (4)	
Dogwood/McConnell	Minor Collector						Major Collector (4)	+
Proposed McConnell/River	None						Major Collector (4)	
Seybert Road	Miner Cellester						Mines Cellester	
Taecker/SR-78 Shank Road	Minor Collector						Minor Collector	
Best/SR-115	Minor Arterial						Minor Arterial (4)	
SR-115/Irvine	Minor Collector						Minor Collector (2)	
Silsbee Road	Miner Cellester						Miner Cellenter (0)	
Evan Hewes Hwy/McCabe Sinclair Road	Minor Collector						Minor Collector (2)	
Gentry/SR-111	Major Collector						Prime Arterial (6-divided)	
SR-111/Weist	Minor Collector						Minor Collector (2)	
Slayton Road	Miner Celle ster						Miner Cellenter (0)	
Worthington/Holtville (City) Snyder Road	Minor Collector						Minor Collector (2)	
Worthington/Bonds Corner Road	Minor Collector						Minor Collector (2)	
Stahl Road							· /	
McConnell/End	Minor Collector						Minor Collector (2)	
Streiby Road Fifield/Wiest	Minor Collector						Minor Collector (2)	
Taecker Road	WILLION CONCERO						Willion Collector (2)	
Seybert/Hastain	Minor Collector						Minor Collector (2)	
Titsworth Road	Minor C-II/						Minor Cellt (0)	
Butters/End Townsend Road	Minor Collector						Minor Collector (2)	
SR-115/Holt	Minor Collector						Minor Collector (2)	
Vail Road							· ,	
Lack/Kalin	Minor Collector						Minor Collector (2)	
Van Der Linden Hunt/Connelly	Minor Collector						Minor Collector (2)	
	I WILLION CONCUID			1		1	IVIII IOI CONECIOI (2)	1
Vencill Road								

TABLE 3 IMPERIAL COUNTY PROJECTED STREET SEGMENT CONFIGURATIONS AND **VOLUMES** (continued)

Segment Location	2003 Classification	Year 2002 ADT Volume ^a	Year 2005 ADT Volume ^a	Year 2025 ADT Volume ^c	25 Year Total Growth Factor ^d	Year 2050 ADT Volume	Year 2050 Recommended Classification (# of Lanes)	2050 LOS ^e
Verde School Road Keffer/Bornt	Minor Collector						Minor Collector (2)	
Villa Road							(_)	
Dogwood/Cooley	Minor Collector						Minor Collector (2)	
Wahl Road Nichols/Clark	Minor Collector						Minor Collector (2)	
Walker Road	Willion Collector						Millior Collector (2)	
Gentry/End	Major Collector						Major Collector (4)	
Gentry/Brandt	Minor Collector						Minor Collector (2)	
Ware Road	Major Callagtor						Major Callagtor (4)	
Fawcett/Willoughby Weaver Road	Major Collector						Major Collector (4)	
Kalin/SR-86	Minor Collector						Minor Collector (2)	
Webster Road								
Kalin/Brandt	Minor Collector						Minor Collector (2)	
Westmorland Road Boley/Evan Hewes Hwy	Minor Collector						Minor Collector (2)	
Westside Road	IVIIIIOI COILECTOI						IVIIIIOI COIIECIOI (2)	_
Evan Hewes Hwy/End	Minor Collector						Minor Collector (2)	
Wheeler Road								
Erskine/Pierle	Minor Collector						Minor Collector (2)	
Wieman Road Steiner/Cady	Minor Collector						Minor Collector (2)	
Wienert Road	WIII IOI COIIECIOI						Willion Collector (2)	
Guthrie/Forrester	Minor Collector						Minor Collector (2)	
Wiest Road								
SR-78/Griffin	Minor Collector						Minor Collector (2)	
Griffin/Boyd McDonald/SR-115	Local Minor Collector						Minor Collector (2) Minor Collector (2)	
Wilkins Road	Willion Collector						WINTON CONCOLOR (2)	
English/Cuff	Minor Collector						Minor Collector (2)	
Wilkinson Road								
Brandt/SR-111 Wiest/Flett	Minor Collector Minor Collector						Minor Collector (2) Minor Collector (2)	1
Willoughby Road	Willion Collector						Millior Collector (2)	
Proposed La Brucherie/Clark	none						Major Collector (4)	
Clark/Dogwood	Minor Collector						Major Collector (4)	
Dogwood/Kloke	Major Collector						Major Collector (4)	
Wirt Road Wiest/Kaiser	Minor Collector						Minor Collector (2)	
Wixom Road	William Collector						William Collector (2)	
Liebert/Drew	Minor Collector						Minor Collector (2)	
Wormwood Road							M: 0 II ((0)	
Dearborn/Fisher Worthington Road (S28)	Minor Collector						Minor Collector (2)	
Huff/Highline	Major Collector						Major Collector (4)	
Yocum Road	,,,						()	
Proposed Dogwood/Lyerly	none						Major Collector (2)	
Lyerly/Kershaw	Minor Collector						Major Collector (4)	
Kershaw/Blair Young Road	Local						Major Collector (4)	_
SR-111/Blair	Minor Collector						Minor Collector (2)	
Zenos Road								
Barbara Worth/Holtville (City)	Minor Collector						Minor Collector (2)	
State Route 78 S.DImperial County Line/Junction SR-86	State Hwy	N/A	920	8,104	1.64	13,500	Collector (4)	Α
SR-111/SR-115N	State Hwy	N/A	3,950	10,592	1.64	17,500	Collector (4)	В
SR-115N/SR-115S	State Hwy	N/A	3,100	13,447	1.64	22,500	Collector (4)	В
115S/Glamis	State Hwy	N/A	1,950	7,340	1.64	12,500	Collector (4)	Α
Glamis/Olgilby	State Hwy	N/A	1,850	4,909	1.64	8,500	Collector (4)	A
Olgilby/Palo Verde, Fourth Palo Verde, Fourth/Imperial County Line	State Hwy State Hwy	N/A N/A	2,000 2,000	5,307 5,307	1.64 1.64	9,000 9,000	Collector (4) Collector (4)	A

TABLE 3 IMPERIAL COUNTY PROJECTED STREET SEGMENT CONFIGURATIONS AND VOLUMES (continued)

Segment Location State Route 86	2003 Classification	Year 2002 ADT Volume ^a	Year 2005 ADT Volume ^a	Year 2025 ADT Volume ^c	25 Year Total Growth Factor ^d	Year 2050 ADT Volume	Year 2050 Recommended Classification (# of Lanes)	2050 LOS°
Imperial County Line/Desert Shores	State Hwv	N/A	12,900	21,138	1.28	27,500	Minor Arterial (4)	С
Desert Shores/Brawley Ave.	State Hwy	N/A	12,900	20,319	1.28	26,500	Collector (4)	C
Brawley Ave./S. Marina	State Hwy	N/A	13,400	21,957	1.28	28,500	Minor Arterial (4)	C
S. Marina/Air Park	State Hwy	N/A	12,100	19,827	1.64	33,000	Prime Arterial (6-divided)	В
Air Park/SR-78 West	State Hwy	N/A	10,800	17,697	1.64	29,500	Minor Arterial (4)	C
SR-78 West/Lack	State Hwy	N/A	10,800	17,890	1.64	29,500	Minor Arterial (4)	C
Lack/West Westmorland City Limits	State Hwy	N/A	10,200	19,650	1.64	32,500	Prime Arterial (6-divided)	В
E Westmorland C. Limits/W Brawley C. Limits	State Hwy	N/A	14,000	19,440	1.64	32,000	Prime Arterial (6-divided)	В
South Brawley City Limits/Legion	State Hwy	N/A	21.400	28,300	1.13	32,500	Prime Arterial (6-divided)	В
Legion/Keystone	State Hwy	N/A	19,100	27,940	1.13	32,000	Prime Arterial (6-divided)	В
Keystone/Imperial Ave.	State Hwy	N/A	14,700	27,940	1.13	32,000	Prime Arterial (6-divided)	В
I-8/McCabe	State Hwy	N/A	21,500	24,890	1.13	32,000	Prime Arterial (6-divided)	В
McCabe/Heber	State Hwy	N/A	7,100	26,100	1.28	33,500	Prime Arterial (6-divided)	В
Heber/Dogwood	State Hwy	N/A	7,100	26,100	1.28	33,500	Prime Arterial (6-divided)	В
	State Hwy	N/A	5,200	26,000	1.28	33,500		В
Dogwood/SR-111 South Imperial City Limits/North El Centro City Limits	State Hwy	N/A	6.500	27,980	1.13	32,000	Prime Arterial (6-divided) Prime Arterial (6-divided)	В
State Route 98	State riwy	IN/A	0,300	27,900	1.13	32,000	Prime Arteriai (6-divided)	Ь
Imperial Hwy/Drew	State Hwy	N/A	2,300	1,730	1.64	3,000	Local Collector (2)	В
Drew/Clark	State Hwy	N/A	3,800	5,350	1.64	9,000	Collector (4)	A
Clark/Dogwood	State Hwy	N/A	4,550	8,800	1.64	14,500	Collector (4)	В
Dogwood/West Calexico City Limits		N/A					Prime Arterial (6-divided)	В
,	State Hwy	N/A	9,800	24,180	1.64	31,500	Prime Arterial (6-divided) Prime Arterial (6-divided)	В
East Calexico City Limits/Barbara Worth Barbara Worth/Bonds Corner	State Hwy State Hwy	N/A	24,400 16.300	26,000	1.64 1.64	33,500		В
		N/A	- 1	26,000		33,500	Prime Arterial (6-divided)	_
Bonds Corner/E. Highline Canal E. Highline Canal/I-8	State Hwy	N/A	4,500 2,200	770 250	1.64 1.64	1,500 500	Local Collector (2)	A
State Route 111	State Hwy	IN/A	2,200	250	1.04	500	Local Collector (2)	А
North Calexico City Limits	State Hwy	N/A	50,000	97,570	1.13	111,000	Freeway (8)	С
Heber/McCabe	State Hwy	N/A	33,500	98,650	1.13	112,000	Freeway (8)	C
McCabe/I-8	State Hwy	N/A	37,000	90,830	1.13	103,000	Freeway (8)	C
I-8/Evan Hewes Hwy	State Hwy	N/A	16.300	52.980	1.13	60,500	Expressway (6)	D
Evan Hewes/Aten		N/A	14,100	60,200	1.13	68,500	, , , ,	D
	State Hwy State Hwy	N/A	11,300	58,160	1.13	66,000	Expressway (6)	D
Aten/Worthington Worthington/Keystone	State Hwy	N/A	10,600	58,710	1.13	67,000	Expressway (6)	D
Keystone/E. Junction 78	State Hwy	N/A	9,300	57,590	1.13	65,500	Expressway (6) Expressway (6)	D
,	State Hwy	N/A	9,500	18,510	1.13	30,500	Prime Arterial (6-divided)	В
North Brawley City Limits/Rutherford	State Hwy	N/A	6,600	18,560	1.64	30,500	Prime Arterial (6-divided) Prime Arterial (6-divided)	В
Rutherford/South Calipatria City Limits North Calipatria City Limits/Sinclair	State Hwy	N/A	5.700	15,640	1.64	26.000	Minor Arterial (4)	C
. ,		N/A	5,700	13,532	1.64	22,500		В
Sinclair/Niland Ave Niland Ave/English	State Hwy State Hwy	N/A	3,700	9,817	1.64	16,500	Collector (4) Collector (4)	В
English/Bombay Beach	State Hwy	N/A	2,300	6,103	1.64	10,500	Collector (4)	A
Bombay Beach/Imperial-Riverside County line	State Hwy	N/A	1,900	5,041	1.64	8,500	Collector (4)	A
State Route 115	State Hwy	IN/A	1,900	3,041	1.04	0,300	Collector (4)	А
	Ctoto I luni	NI/A	4.050	4.440	1.01	7 000	Lead Callester (2)	
Junction I-8/East Holtville City Limits West Holtville City Limits West Hungtion Even Howes Hung	State Hwy State Hwy	N/A N/A	1,850	4,140 8,320	1.64	14,000	Local Collector (2)	В
West Holtville City Limits/West Junction Evan Hewes Hwy West Junction Evan Hewes Hwy/SR-78		N/A	6,600		1.04	14,000	Collector (4) Prime Arterial (6-divided)	В
,	State Hwy		2,850	27,870		32,000 22,500	\ /	В
SR-78/Rutherford	State Hwy	N/A	990	13,450	1.64		Minor Arterial (4)	_
Rutherford/Wirt	State Hwy	N/A	1,650	9,720	1.64	16,000	Collector (4) Collector (4)	B
Wirt/East Calipatria City Limits	State Hwy	N/A	1,150	9,240	1.64	15,500	Collector (4)	В
State Route 186 I-8/International Border	State Hwy	N/A					State Hwy	

Notes:

- * See Table 1 regarding additional right-of-way for transit facility with roadway.
- a. Volume from Imperial County Circulation and Scenic Highways Element Manual (Dec. 2003).
- b. Volume from Caltrans, Imperial County, or Linscott Law & Greenspan, Engineers counts.
- c. Volumes from Caltrans CalexGP+ Model and adjusted higher in some cases.
- d. A 0.5%, 1.0%, or 2.0% annual growth rate was applied to the Year 2025 volumes to obtain Year 2050 volumes.
- e. Capacity based on the Imperial County Classification Table (depending on the Year 2050 volume amount).

Appendix H

Count Data

PM ΑM Split % 48.7% 60.0% 40.0% 51.3% 39.8% 60.2% **Peak Hour** 09:30 05:15 05:30 16:15 14:15 16:15 Volume P.H.F. 0.52 0.73 0.79 0.80 0.84 0.75

CITY: IMPERIAL VALLEY

0.85

0.86

0.84

0.83

0.68

Volume

P.H.F.

0.68

PROJECT: PTD13-0426-01

CITY: IMPERIAL VALLEY

	BROCKMAI AM Period		IN LTO	SB		EB	WB		PM Period	NB		SB		EB W	B	
1																
		0														
1011-15 0																
11-15			0		2			2			14		15			29
10.150																
11-19																
			1		2			3			10		20			30
			- '					<u> </u>			10		20			30
1																
Decay																
1			4	0	2			6			17		19			36
1	03:00	2		0					15:00	8		3				
1	03:15	2		1					15:15	8		3				
04-00																
04130 1 1 1 1 1 1615 2 1 1 1 1 1615 2 1 1 1 1 1614 1 1 1 1614 1 1 1 1 1 1 1 1			5		1			6			25		12			37
04-45																
04.45																
1			6		5			11			83		16			00
10			<u> </u>		<u> </u>						0.0		10			//
1																
124																
06:15			12		112			124			94		11			105
06:30 6	06:00	1		45					18:00	11		3				
06.45	06:15	4		26					18:15	6		0				
19:00																
19:15 5			18		86			104			21		7			28
07:30																
107:45																
08:00			18		23			<i>/</i> 11			6		1			7
1			10		23			71								,
08:30 3																
No. No.																
09:15 6		3	14	4	25			39		0	3	2	2			5
09:30 9 2 21:30 0	09:00	2		4					21:00	2		1				
10:00	09:15	6		4					21:15	1		0				
10:00																
10:15 8			21		17			38			3		2			5
10:30																
10:45 2 14 4 13 27 22:45 0 2 1 1 3 11:00 10 5 23:00 0 0 11:15 5 5 5 23:15 0 1 11:30 3 21 2 17 38 23:45 0 0 0 2 Total Vol. 134 305 439 278 108 386																
11:00 10 5 23:00 0 0 0 1 11:15 5 5 5 23:15 0 0 1 11:30 3 5 23:30 0 0 0 2 2 2 2 2 2			14		13			27			2		1			3
11:15 5 5 5 23:15 0 1 11:30 3 5 23:30 0 1 11:45 3 21 2 17 38 23:45 0 0 0 2 2 2 2 2 2 2													•			<u>_</u>
11:30 3 5 3 21 2 17 38 23:30 0 1 3 3 21 2 17 38 23:45 0 0 0 0 2 2 2 Total Vol. 134 305 439 278 108 386 NB SB EB WB Combined 412 413 825																
11:45 3 21 2 17 38 23:45 0 0 0 2 2 Total Vol. 134 305 439 278 108 108 Daily Totals B WB Combined BEB NB SB EB WB Combined Combined 																
NB SB Daily Totals EB WB Combined Combined Combined EB 412 413 825 PM Split % 30.5% 69.5% 53.2% 72.0% 28.0% 46.8% Peak Hour 10:15 05:30 16:30 12:45 16:30 Volume 24 155 170 129 21 145		3	21	2	17			38		0	0	0	2			2
NB SB EB WB Combined 412 413 825 1412	Total Vol.		134		305			439			278		108			386
AM PM Split % 30.5% 69.5% 53.2% 72.0% 28.0% 46.8% Peak Hour 10:15 05:30 05:30 16:30 12:45 16:30 Volume 24 155 170 129 21 145														Daily Totals EB	WB	Combined
Split % 30.5% 69.5% 53.2% 72.0% 28.0% 46.8% Peak Hour 10:15 05:30 05:30 16:30 12:45 16:30 Volume 24 155 170 129 21 145						.					412		413			825
Peak Hour 10:15 05:30 05:30 16:30 12:45 16:30 Volume 24 155 170 129 21 145	Cnli+ 0/		20 504		40 F0/	AM		E2 20/	-		72.004		20.00/	PM		14 00/
Volume 24 155 170 129 21 145)				

0.87

0.79

0.58

0.60

0.86

P.H.F.

0.68

PROJECT: PTD13-0426-01

ΑM PM Split % 33.4% 53.5% 71.2% 28.8% 46.5% 66.6% **Peak Hour** 05:15 05:30 05:30 16:15 15:45 16:15 Volume P.H.F. 0.58 0.78 0.73 0.78 0.67 0.73

				494	464		958
			AM			PM	
Split %	19.4%	80.6%	49.0%	82.4%	17.6%		51.0%
Peak Hour	06:30	05:15	05:15	16:15	16:00		16:15
Volume	24	227	234	204	19		223
P.H.F.	0.67	0.64	0.63	0.62	0.48		0.61

	FORRESTER	

AM Period NB	SB	EB		WB			PM Period NB	SB	EB		WB		
00:00		3		0			12:00		3		6		
00:15		0		0			12:15		5		2		
00:30		2		0			12:30		3		3		
00:45		1	6	0	0	6	12:45		7	18	11	22	40
01:00		3		0			13:00		4		1		
01:15		3		0			13:15		7		13		
01:30		3		0			13:30		4		6		
01:45		2	11	1	1	12	13:45		9	24	3	23	47
02:00		13		0			14:00		6		11		
02:15		5		0			14:15		8		13		
02:30		1		0			14:30		9		12		
02:45		0	19	1	1	20	14:45		10	33	13	49	82
03:00		0		0			15:00		7		9		
03:15		2		0			15:15		9		9		
03:30		0		0			15:30		11		7		
03:45		0	2	1	1	3	15:45		10	37	10	35	72
04:00		3		0			16:00		18		7		
04:15		2		2			16:15		44		9		
04:30		1		1			16:30		21		8		
04:45		1	7	0	3	10	16:45		55	138	4	28	166
05:00		1		1			17:00		41		6		
05:15		6		4			17:15		20		11		
05:30		1		35			17:30		18		5		
05:45		4	12	51	91	103	17:45		11	90	4	26	116
06:00		1		38			18:00		5		2		
06:15		5		15			18:15		2		4		
06:30		7		18			18:30		4		2		
06:45		5	18	12	83	101	18:45		5	16	7	15	31
07:00		10		7			19:00		4		5		
07:15		8		8			19:15		4		5		
07:30		7		18			19:30		3		4		
07:45		15	40	8	41	81	19:45		2	13	2	16	29
08:00		17		13			20:00		3		2		
08:15		15		7			20:15		2		0		
08:30		13		11			20:30		1		2		
08:45		6	51	4	35	86	20:45		2	8	0	4	12
09:00		12		41			21:00		3		3		
09:15		5		12			21:15		3		0		
09:30		10		8			21:30		1		0		
09:45		4	31	3	64	95	21:45		3	10	4	7	17
10:00		13		4			22:00		2		0		
10:15		8		10			22:15		0		1		
10:30		7		6			22:30		2		0		
10:45		7	35	5	25	60	22:45		0	4	0	1	5
11:00		6		12			23:00		1		0		
11:15		6		7			23:15		5		0		
11:30		22		7			23:30		6		2		
11:45		15	49	6	32	81	23:45		0	12	0	2	14

NB SB EB WB Combined

684 605 1289

AM

PM

Combined

684 605 1289

Split %	42.7%	57.3% 51.0%	63.9%	36.1%	49.0%
Peak Hour	07:45	05:30 05:30	16:15	14:00	16:15
Volume P.H.F.	60 0.88	139 150 0.68	161 0.73	49 0.94	188 0.80

CITY: IMPERIAL VALLEY

LA BRUCH		BTN N				_								
AM Period			SB		<u>EB</u> W	В	PM Period	NB		SB		EB WE	3	
00:00	1		0				12:00	16		9				
00:15	2		0				12:15	6		6				
00:30	0	,	0	1		7	12:30	17	F0	9	20			00
00:45	3	6	1	1		7	12:45	13	52	14	38			90
01:00	0		4				13:00	8		5				
01:15	1		2				13:15	10		10				
01:30	0		1			0	13:30	10	0.0	7	0.0			74
01:45	0	1	1	8		9	13:45	10	38	11	33			71
02:00	0		1				14:00	15		12				
02:15	0		0				14:15	15		11				
02:30	1	4	0	1		7	14:30	8	E1	18	40			100
02:45	5	6	0	1			14:45	13	51	8	49			100
03:00	1		1				15:00	20		7				
03:15	1		0				15:15	20		24				
03:30	1	4	0	1		Г	15:30	23	70	32	77			15/
03:45	1	4	0	1		5	15:45	16	79	14	77			156
04:00	8		1				16:00	18		15				
04:15	1		0				16:15	22		18				
04:30 04:45	1 1	11	1 3	5		16	16:30 16:45	24 31	95	20 21	74			169
		- 11		3		10			95		74			109
05:00	1		3				17:00	27		13				
05:15	3		6				17:15	33		18				
05:30 05:45	6 7	17	28 25	62		79	17:30 17:45	24 21	105	18 16	65			170
		17		02		19			105		00			170
06:00 06:15	5 15		55 21				18:00 18:15	16		15 17				
06:15	15 12		21 30				18:15	20 12		13				
06:45	15	47	22	128		175	18:45	26	74	10	55			129
		77		120		173			74	9	- 55			127
07:00 07:15	18 28		15 11				19:00 19:15	15 13		9				
07.15	21		12				19:15	8		8				
07:45	43	110	21	59		169	19:45	7	43	9	35			78
08:00	50		33				20:00	8		1				
08:00	19		27				20:00	9		6				
08:30	11		13				20:30	4		2				
08:45	9	89	9	82		171	20:45	1	22	8	17			39
09:00	10		8				21:00	2		1				-
09:15	13		12				21:15	5		1				
09:30	10		5				21:30	3		1				
09:45	15	48	16	41		89	21:45	2	12	2	5			17
10:00	12		11				22:00	4		4				
10:00	9		12				22:15	3		0				
10:30	12		23				22:30	2		0				
10:45	12	45	8	54		99	22:45	2	11	0	4			15
11:00	16		8				23:00	4		1				
11:15	13		13				23:15	1		2				
11:30	16		10				23:30	0		0				
11:45	12	57	10	41		98	23:45	2	7	2	5			12
Total Vol.		441		483	·	924	·		589		457			1046
Total Vol.		771		403		724			307		437	Daily Tatala		1040
									NB		SB	Daily Totals EB	WB	Combined
									1030		940			1970
					AM				. 550		, 10	PM		.,,,
Split %		47.7%	ı	52.3%	7 1141	46.9%			56.3%	ı	43.7%			53.1%
Peak Hour		07:15		05:45		07:30			16:30		15:15			16:30
Volume		142		131		226			115		85 0.66			187
P.H.F.		0.71		0.60		0.68			0.88		0.66			0.90

PROJECT: PTD13-0426-01

				610	139		1347
			AM			PM	
Split %	43.2%	56.8%	47.3%	60.4%	39.6%		52.7%
Peak Hour	07:30	05:45	05:45	16:30	15:15		16:30
Volume P.H.F.	93 0.66	140 0.69	163 0.74	112 0.90	66 0.63		152 0.73

CITY: IMPERIAL VALLEY

0.70

0.77

0.86

P.H.F.

0.71

0.77

0.78

PROJECT: PTD13-0426-01

CITY: IMPERIAL VALLEY

0.85

0.59

0.88

P.H.F.

0.76

0.86

0.72

PROJECT: PTD13-0426-01

CHE EB 0 0 0 0 0 0 0 0 0 0 0 0 0 0 1	0 0	WB 0 0 0 0 0 0 1 0 0 0	0	1	PM Period NB 12:00 12:15 12:30 12:45 13:00 13:15 13:30	SB	EB 0 7 3 3 3 1 4	13	WB 1 2 0 1 2	4
0 0 0 0 0 0 0 0 0 0 0 0	0	0 0 0 0 0 0 0 1		1	12:15 12:30 12:45 13:00 13:15		7 3 3 1 4	13	2 0 1	4
0 0 0 0 0 0 0 0 0 0	0	0 0 0 0 0 1		11	12:30 12:45 13:00 13:15		3 3 1 4	13	0 1	4
0 0 0 0 0 0 0 0 0	0	0 0 0 0 1 0 0		1	12:45 13:00 13:15		3 1 4	13	1	4
0 0 0 0 0 0 0 0 0	0	0 0 0 1 0		1	13:00 13:15		1 4	13		4
0 0 0 0 0 0 0 0		0 0 1 0 0	1	1	13:15		4		າ	7
0 0 0 0 0 0 0		0 1 0 0	11	1					_	
0 0 0 0 0		1 0 0	1	1	13:30				0	
0 0 0 0 0		0	1	1			2		8	
0 0 0 0 0	0	0			13:45		4	11	0	10
0 0 0 0 0	0				14:00		4		2	
0 0 0 0	0	0			14:15		5		7	
0 0 0	0				14:30		1		1	
0		0	0		14:45		1	11	0	10
0		0			15:00		1		1	
		0			15:15		3		0	
1		0			15:30		9		2	
-	1	0	0	1	15:45		1	14	0	3
0		0			16:00		1		2	
0		2			16:15		3		2	
0		0			16:30		1		0	
0	0	0	2	2	16:45		1	6	2	6
0		1			17:00		2		0	
0		2			17:15		2		1	
1		1			17:30		1		1	
1	2	3	7	9	17:45		0	5	0	2
1		2			18:00		1		1	
1		1			18:15		1		0	
1		1			18:30		1		1	
4	7	4	8	15	18:45		0	3	1	3
0		2			19:00		0		1	
1					19:15				0	
2		3			19:30		0		0	
0	3	0	7	10	19:45		1	3	1	2
0		0			20:00		0		0	
0										
0		2			20:30		0		0	
0	0	1	3	3	20:45		0	0	0	0
4		1			21:00		1		0	
1										
5										
3	13	2	6	19			0	2	4	11
3		0					0		0	
2		2			22:30		0		0	
2	7	2	6	13			0	0	0	0
		2					0		0	
2		0					0		0	
3	11	1	4	15	23:45		0	0	0	0
	4.4		4.4					/ 0		E1
	44		44	88						51
	1 2 0 0 0 0 0 0 4 1 1 5 3 3 0 2 2 2 6 6 0 2	1 2 0 3 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	1	1	1	1 2 19:15 2 3 19:30 0 3 0 7 10 19:45 0 0 20:00 0 0 20:15 0 2 20:30 0 0 1 3 3 20:45 4 1 21:00 1	1 2 19:15 2 3 19:30 0 3 0 7 10 19:45 0 0 20:00 0 0 20:15 0 0 20:30 0 0 1 3 3 20:45 4 1 21:00 21:15 5 3 21:30 21:30 3 13 2 6 19 21:45 3 0 22:00 22:15 2 2 22:30 2 7 2 6 13 22:45 6 2 23:00 0 1 23:15 23:30 3 11 1 4 15 23:45	1 2 19:15 2 2 3 19:30 0 0 3 0 7 10 19:45 1 0 0 0 20:00 0 0 0 0 20:15 0 0 0 0 0 0 2 20:30 0	1 2 19:15 2 2 3 19:30 0 0 3 0 7 10 19:45 1 3 0 0 0 20:00 1 1 1 0 1 </td <td>1 2 19:15 2 0 2 3 19:30 0 0 0 3 0 7 10 19:45 1 3 1 0 0 0 20:00 0<!--</td--></td>	1 2 19:15 2 0 2 3 19:30 0 0 0 3 0 7 10 19:45 1 3 1 0 0 0 20:00 0 </td

Daily Totals EB SB NB WB Combined 95 112 207 ΑM PM 50.0% **42.5%** Split % 50.0% 57.1% 42.9% 57.5% Peak Hour 09:00 09:00 13:30 13:30 06:45 13:30 Volume 13 11 19 15 17 32 P.H.F. 0.65 0.69 0.59 0.75 0.53 0.67

17

21

21

17

12

13

119

CITY: IMPERIAL VALLEY

WEDNESDAY, APRIL 24	, 2013				CITY:	IMPERIAL	VALLEY		Р	KOJE(JI:	PIL	113-0426-	01
KUBLER BTN BROCKMA	N & FERRELL													
AM Period NB S	SB EB		WB			PM Period	NB	SB		EB		WB		
00:00	0		0			12:00				2		3		
00:15	0		0			12:15				0		1		
00:30	0		0			12:30				1		1		
00:45	0	0	0	0		12:45				3	6	3	8	14
01:00	0		0			13:00				1		2		
01:15	0		0			13:15				0		6		
01:30	0		0			13:30				4		2		
01:45	0	0	0	0		13:45				2	7	6	16	23
02:00	0		0			14:00				 1		4		
02:00	0		0			14:00				5		2		
02:30	0		0			14:13				3		9		
02:45	0	0	0	0		14:45					12	2	17	29
		- 0		- 0							12		17	27
03:00	0		0			15:00				4		7		
03:15	0		1			15:15				4		6		
03:30	0	4	0	4	0	15:30				1		5	04	0.5
03:45	1	1	0	1	2	15:45					14	3	21	35
04:00	0		0			16:00				5		1		
04:15	0		0			16:15				1		4		
04:30	0		0			16:30				2		0		
04:45	0	0	3	3	3	16:45				2	10	0	5	15
05:00	0		0			17:00				1		1		
05:15	0		0			17:15				2		2		
05:30	1		0			17:30				1		1		
05:45	0	1	1	1	2	17:45				3	7	0	4	11
06:00	0		2			18:00				2		0		
06:15	1		1			18:15				3		2		
06:30	0		1			18:30				5		0		
06:45	2	3	2	6	9	18:45				0	10	2	4	14
07:00	1		0			19:00				1		0		
07:15	0		0			19:15				0		1		
07:30	4		0			19:30				0		0		
07:45	1	6	1	1	7	19:45					2	0	1	3
08:00	1		0		-	20:00				1		0		
08:15	1		1			20:00				0		0		
08:30	1		6			20:15				0		1		
08:45	8	11	4	11	22	20:30					1	3	4	5
		- 11		- 11	22								4	<u> </u>
09:00	5		1			21:00				0		1		
09:15	6		3			21:15				0		0		
09:30	3	4.	0	-	04	21:30				0	•	1	0	
09:45	2	16	1	5	21	21:45					0	0	2	2
10:00	2		3			22:00				0		1		
10:15	1		3			22:15				0		0		
10:30	4		3			22:30				0		0		
10:45	1	8	3	12	20	22:45					0	0	1	1
11:00	1		4			23:00				2		0		
11:15	1		0			23:15				0		0		
11:30	3		3			23:30				1		1		
11:45	5	10	4	11	21	23:45				0	3	0	1	4
Total Vol.		56		51	107						72		84	156
. Jiai voi.		30		31	.57								J-1	130
								IB	SB	Dai	i ly To EB	otals	WB	Combined
								10	JU					
											128		135	263
0 111 01		AM			10 =5:						PM		F0 000	FC 05:
Split %		52.3%		47.7%	40.7%					46	5.2%)	53.8%	59.3%
Peak Hour		08:45		08:30	08:30					1	4:15		14:30	14:30
Volume		22		14	34						15		24	38
P.H.F.		0.69		0.58	0.71						0.75		0.67	0.79
						TECHNICA	LDATA							

PROJECT: PTD13-0426-01

CITY: IMPERIAL VALLEY

0.79

0.83

0.63

P.H.F.

0.74

0.75

CALTRANS 2011 DATA

									Ahead	
					Back Peak	Back Peak		Ahead	Peak	Ahead
Dist	Route	CO	Postmile	Description	Hour	Month	Back AADT	Peak Hour	Month	AADT
11	98	IMP	1.518	IMPERIAL HIGHWAY	170	1600	1450	180	1750	1650
11	98	IMP	22.197	DREW/SUNSET BLVD	220	1950	1800	200	1950	1800
11	98	IMP	27.21	FERRELL RD	200	1950	1800	260	2600	2400
11	98	IMP	28.74	CLARK RD	260	2600	2400	360	3750	3650
11	98	IMP	30.27	DOGWOOD RD	360	3750	3650	680	8800	8100

CALTRANS 2011 DATA

											Ahead	
							Back Peak	Back Peak		Ahead	Peak	Ahead
Dist	Route	CO		Postmile		Description	Hour	Month	Back AADT	Peak Hour	Month	AADT
11	8	IMP	R	23.48	DUNAWAY RD		1750	14000	11900	1750	13300	12000
11	8	IMP	R	29.933	DREW RD		1750	13300	12000	1950	14900	13600
11	8	IMP	R	33.991	FORRESTER RD		1950	14900	13600	2100	18000	16700
11	8	IMP	R	36.973	IMPERIAL AVE		2100	18000	16700	3750	34000	31000

PREPARED BY: PACIFIC TECHNICAL DATA

<u>DATE:</u> 4/24/13 WEDNESDAY LOCATION: NORTH & SOUTH:

IMPERIAL VALLEY FORRESTER

PROJECT #:

PTD13-0426-01

LOCATION #: 1

1

1-WAY STOP (WB)

WEDNESDAY

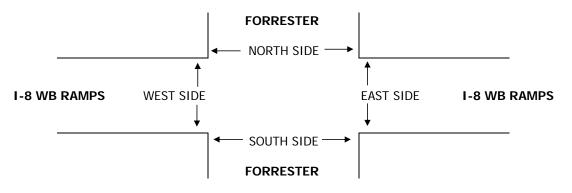
EAST & WEST: I-8 WB RAMPS

CONTROL:

NOTES:

AM
PM
N
N
E
OTHER
OTHER

					COLUTIVO UND				OTHER V					
		NC	ORTHBOU	ND	SO	OUTHBOU	ND	E	ASTBOUN	ID	W	/ESTBOUN	1D	
			FORRESTER			FORRESTER			I-8 WB RAMPS	5		I-8 WB RAMPS	S	
		NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
	LANES:	0	1	Χ	X	1	0	X	X	Χ	1	0	1	
	5:30 AM	1	4			21	20				22	0	23	91
	5:45 AM	1	6			31	16				45	0	26	125
	6:00 AM	2	5			27	10				24	0	27	95
	6:15 AM	0	12			21	20				10	0	34	97
	6:30 AM	1	7			18	18				15	0	34	93
	6:45 AM	1	8			15	10				10	0	27	71
	7:00 AM	2	13			14	17				8	0	21	75
ΑM	7:15 AM	1	7			7	11				10	0	26	62
¥	VOLUMES	9	62	0	0	154	122	0	0	0	144	0	218	709
	APPROACH %	13%	87%	0%	0%	56%	44%	0%	0%	0%	40%	0%	60%	
	APP/DEPART	71	/	280	276	/	298	0	/	0	362	/	131	0
	BEGIN PEAK HR		5:45 AM											
	VOLUMES	4	30	0	0	97	64	0	0	0	94	0	121	410
	APPROACH %	12%	88%	0%	0%	60%	40%	0%	0%	0%	44%	0%	56%	
	PEAK HR FACTOR		0.708			0.856			0.000			0.757		0.820
	APP/DEPART	34	/	151	161	/	191	0	/	0	215	/	68	0
	4:00 PM	1	26			21	4				8	0	10	70
	4:15 PM	2	23			42	7				3	0	16	93
	4:30 PM	2	42			32	11				11	0	17	115
	4:45 PM	8	35			38	7				9	0	7	104
	5:00 PM	10	31			31	5				5	0	9	91
	5:15 PM	7	27			27	4				4	0	12	81
	5:30 PM	8	22			28	6				8	0	5	77
PΜ	5:45 PM	7	13			22	2				9	0	4	57
Ъ	VOLUMES	45	219	0	0	241	46	0	0	0	57	0	80	688
	APPROACH %	17%	83%	0%	0%	84%	16%	0%	0%	0%	42%	0%	58%	
	APP/DEPART	264	/	299	287	/	298	0	/	0	137	/	91	0
1	BEGIN PEAK HR		4:15 PM											
	VOLUMES	22	131	0	0	143	30	0	0	0	28	0	49	403
	APPROACH %	14%	86%	0%	0%	83%	17%	0%	0%	0%	36%	0%	64%	
1	PEAK HR FACTOR		0.869			0.883			0.000			0.688		0.876
	APP/DEPART	153	/	180	173	/	171	0	/	0	77	/	52	0



PREPARED BY: PACIFIC TECHNICAL DATA

<u>DATE:</u> 4/24/13 WEDNESDAY LOCATION: IMPERIAL VALLEY NORTH & SOUTH: FORRESTER

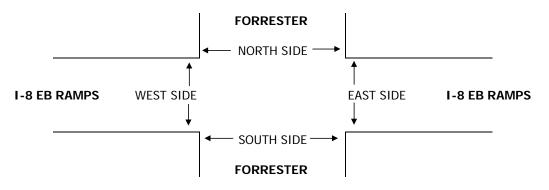
RIAL VALLEY PROJECT #:

LOCATION #: 2

PTD13-0426-01

EAST & WEST: I-8 EB RAMPS CONTROL: 1-WAY STOP (EB)

		NC	ORTHBOU	ND	SC	UTHBOU	ND	E,	ASTBOUN	1D	V	/ESTBOUN	ID	
			FORRESTER			FORRESTER			I-8 EB RAMPS	S		I-8 EB RAMPS		
		NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
	LANES:	X	1	0	0	1	X	1	0	1	X	X	Χ	
	5:30 AM		4	0	8	37		3	0	0				52
	5:45 AM		2	3	12	60		5	0	1				83
	6:00 AM		4	2	10	40		4	0	0				60
	6:15 AM		5	1	15	15		7	0	1				44
	6:30 AM		4	2	15	17		4	0	0				42
	6:45 AM		4	6	12	14		5	0	0				41
	7:00 AM		7	4	12	10		8	0	1				42
ΑĀ	7:15 AM		4	7	10	9		5	0	1				36
₹	VOLUMES	0	34	25	94	202	0	41	0	4	0	0	0	400
	APPROACH %	0%	58%	42%	32%	68%	0%	91%	0%	9%	0%	0%	0%	
	APP/DEPART	59	/	75	296	/	206	45	/	119	0	/	0	0
	BEGIN PEAK HR		5:30 AM											
	VOLUMES	0	15	6	45	152	0	19	0	2	0	0	0	239
	APPROACH %	0%	71%	29%	23%	77%	0%	90%	0%	10%	0%	0%	0%	
	PEAK HR FACTOR		0.875			0.684			0.656			0.000		0.720
	APP/DEPART	21	/	34	197	/	154	21	/	51	0	/	0	0
	4:00 PM		10	7	22	11		17	1	0				68
	4:15 PM		15	16	23	15		19	0	0				88
	4:30 PM		19	10	25	11		18	0	0				83
	4:45 PM		33	20	34	10		22	0	0				119
	5:00 PM		26	20	33	9		14	0	0				102
	5:15 PM		17	15	19	9		15	0	0				75
	5:30 PM		11	7	32	15		16	0	0				81
₽	5:45 PM		9	8	26	7		10	0	0				60
ᇫ	VOLUMES	0	140	103	214	87	0	131	1	0	0	0	0	676
	APPROACH %	0%	58%	42%	71%	29%	0%	99%	1%	0%	0%	0%	0%	
	APP/DEPART	243	/	271	301	/	87	132	/	318	0	/	0	0
	BEGIN PEAK HR		4:15 PM											
	VOLUMES	0	93	66	115	45	0	73	0	0	0	0	0	392
	APPROACH %	0%	58%	42%	72%	28%	0%	100%	0%	0%	0%	0%	0%	
1	PEAK HR FACTOR		0.750			0.909			0.830			0.000		0.824
	APP/DEPART	159	/	166	160	/	45	73	/	181	0	/	0	0



PREPARED BY: PACIFIC TECHNICAL DATA

DATE: 4/24/13 WEDNESDAY

LOCATION: NORTH & SOUTH:

EAST & WEST:

IMPERIAL VALLEY **FORRESTER**

MCCABE

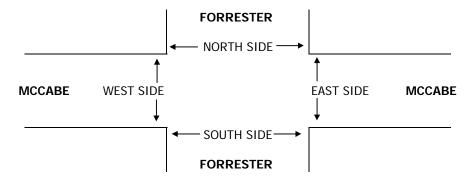
PROJECT #: LOCATION #:

PTD13-0426-01

CONTROL: 2-WAY STOP (EW)

NOTES:	AM		A	
	PM		N	
	MD	⋖ W		E►
	OTHER		S	
	OTHER		▼	

		NORTHBOUND		SOUTHBOUND		ND	E,	ASTBOUN	ID	W	/ESTBOUN	ND		
			FORRESTER			FORRESTER			MCCABE			MCCABE		
		NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
	LANES:	0	1	0	0	1	0	0	1	0	0	1	0	
	5:30 AM	0	1	0	3	0	42	0	1	0	0	7	3	57
	5:45 AM	0	0	0	4	0	48	1	3	0	0	12	4	72
	6:00 AM	0	0	0	5	0	24	1	0	0	0	7	2	39
	6:15 AM	0	0	0	5	0	11	2	2	0	1	4	0	25
	6:30 AM	0	1	0	2	0	3	3	1	1	0	7	1	19
	6:45 AM	1	0	0	4	1	8	1	1	0	1	3	2	22
	7:00 AM	0	0	0	1	0	4	2	6	0	0	3	4	20
₽M	7:15 AM	1	1	0	5	1	5	3	8	1	0	3	4	32
A	VOLUMES	2	3	0	29	2	145	13	22	2	2	46	20	286
	APPROACH %	40%	60%	0%	16%	1%	82%	35%	59%	5%	3%	68%	29%	
	APP/DEPART	5	/	36	176	/	6	37	/	51	68	/	193	0
	BEGIN PEAK HR		5:30 AM											
	VOLUMES	0	1	0	17	0	125	4	6	0	1	30	9	193
	APPROACH %	0%	100%	0%	12%	0%	88%	40%	60%	0%	3%	75%	23%	
	DEAL EASTOR													
	PEAK HR FACTOR		0.250			0.683			0.625			0.625		0.670
	APP/DEPART	1	/	14	142	/	1	10	/	23	40	/	155	0
	APP/DEPART 4:00 PM	0	0	0	6	0	1	9	3	0	0	6	5	0 30
	APP/DEPART 4:00 PM 4:15 PM	0	0 0	0	6	0 0	1 4	9 28	/ 3 10	0 1	0	6 2	5 3	0 30 51
	APP/DEPART 4:00 PM 4:15 PM 4:30 PM	0 0 1	/ 0 0 0	0 0 1	6 3 4	/ 0 0 0	1 4 3	9 28 20	/ 3 10 2	0	0 0 0	/ 6 2 4	5 3 3	0 30 51 38
	APP/DEPART 4:00 PM 4:15 PM 4:30 PM 4:45 PM	0 0 1 0	/ 0 0 0 0	0 0 1 0	6 3 4 2	/ 0 0 0	1 4 3 3	9 28 20 49	/ 3 10 2 7	0 1 0 0	0 0 0 0	/ 6 2 4 0	5 3 3 2	0 30 51 38 63
	APP/DEPART 4:00 PM 4:15 PM 4:30 PM 4:45 PM 5:00 PM	0 0 1	/ 0 0 0	0 0 1	6 3 4 2 4	/ 0 0 0 0	1 4 3 3	9 28 20 49 30	/ 3 10 2 7 8	0 1 0	0 0 0 0	/ 6 2 4 0	5 3 3 2 2	0 30 51 38 63 48
	APP/DEPART 4:00 PM 4:15 PM 4:30 PM 4:45 PM 5:00 PM 5:15 PM	0 0 1 0 0	/ 0 0 0 0 1	0 0 1 0 0	6 3 4 2 4 2	/ 0 0 0 0 0	1 4 3 3 1 2	9 28 20 49 30 12	/ 3 10 2 7 8 7	0 1 0 0 0	0 0 0 0 0	/ 6 2 4 0 2 7	5 3 3 2 2 2	0 30 51 38 63 48
	APP/DEPART 4:00 PM 4:15 PM 4:30 PM 4:45 PM 5:00 PM 5:15 PM 5:30 PM	0 0 1 0 0 0	0 0 0 0 1 0	0 0 1 0 0 0	6 3 4 2 4 2 2	/ 0 0 0 0 0	1 4 3 3 1 2	9 28 20 49 30 12	/ 3 10 2 7 8 7 4	0 1 0 0 0 0	0 0 0 0 0 0	/ 6 2 4 0 2 7	5 3 3 2 2 12 0	0 30 51 38 63 48 42
M	APP/DEPART 4:00 PM 4:15 PM 4:30 PM 4:45 PM 5:00 PM 5:15 PM 5:30 PM 5:45 PM	0 0 1 0 0 0	/ 0 0 0 0 1 0 0	0 0 1 0 0 0 0	6 3 4 2 4 2 2 1	/ 0 0 0 0 0 0	1 4 3 3 1 2 2	9 28 20 49 30 12 9	/ 3 10 2 7 8 7 4	0 1 0 0 0 0 0	0 0 0 0 0 0 0	/ 6 2 4 0 2 7 1 3	5 3 2 2 12 0 2	0 30 51 38 63 48 42 19
PM	APP/DEPART 4:00 PM 4:15 PM 4:30 PM 4:45 PM 5:00 PM 5:15 PM 5:30 PM 5:45 PM VOLUMES	0 0 1 0 0 0 0 0	/ 0 0 0 0 1 0 0 0	0 0 1 0 0 0 0 0	6 3 4 2 4 2 2 1 2	/ 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	1 4 3 3 1 2 2 1 17	9 28 20 49 30 12 9 9	/ 3 10 2 7 8 7 4 1	0 1 0 0 0 0 0 0	0 0 0 0 0 0 0 1	/ 6 2 4 0 2 7 1 3 25	5 3 2 2 12 0 2	0 30 51 38 63 48 42
PM	APP/DEPART 4:00 PM 4:15 PM 4:30 PM 4:45 PM 5:00 PM 5:15 PM 5:30 PM 5:45 PM VOLUMES APPROACH %	0 0 1 0 0 0 0 0 0 1 33%	/ 0 0 0 0 1 0 0	0 0 1 0 0 0 0 0 0 0 1 33%	6 3 4 2 4 2 2 1 2 1 24 59%	/ 0 0 0 0 0 0	1 4 3 3 1 2 2 1 17 41%	9 28 20 49 30 12 9 9	/ 3 10 2 7 8 7 4	0 1 0 0 0 0 0 0 1 2 1%	0 0 0 0 0 0 0 1 0	/ 6 2 4 0 2 7 1 3	5 3 2 2 12 0 2 29 53%	0 30 51 38 63 48 42 19 18
PM	APP/DEPART 4:00 PM 4:15 PM 4:30 PM 4:45 PM 5:00 PM 5:15 PM 5:30 PM 5:45 PM VOLUMES APPROACH % APP/DEPART	0 0 1 0 0 0 0 0	/ 0 0 0 0 1 0 0 0 1 33%	0 0 1 0 0 0 0 0	6 3 4 2 4 2 2 1 2	/ 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	1 4 3 3 1 2 2 1 17	9 28 20 49 30 12 9 9	/ 3 10 2 7 8 7 4 1	0 1 0 0 0 0 0 0	0 0 0 0 0 0 0 1	/ 6 2 4 0 2 7 1 3 25	5 3 2 2 12 0 2	0 30 51 38 63 48 42 19
PM	APP/DEPART 4:00 PM 4:15 PM 4:30 PM 4:45 PM 5:00 PM 5:15 PM 5:30 PM 5:45 PM VOLUMES APPROACH % APP/DEPART BEGIN PEAK HR	0 0 1 0 0 0 0 0 0 1 33% 3	/ 0 0 0 0 1 0 0 0 1 33% /	0 0 1 0 0 0 0 0 0 1 33%	6 3 4 2 4 2 2 1 24 59% 41	/ 0 0 0 0 0 0 0 0 0 0 0 0 0	1 4 3 3 1 2 2 1 17 41% 3	9 28 20 49 30 12 9 9 166 79% 210	/ 3 10 2 7 8 7 4 1 42 20%	0 1 0 0 0 0 0 1 2 1%	0 0 0 0 0 0 1 0 1 2% 55	/ 6 2 4 0 2 7 1 3 25 45% /	5 3 2 2 12 0 2 29 53% 43	0 30 51 38 63 48 42 19 18 309
PM	APP/DEPART 4:00 PM 4:15 PM 4:30 PM 4:45 PM 5:00 PM 5:15 PM 5:30 PM 5:45 PM VOLUMES APPROACH % APP/DEPART BEGIN PEAK HR VOLUMES	0 0 1 0 0 0 0 0 0 1 33% 3	/ 0 0 0 1 0 0 0 1 33% / 4:15 PM	0 0 1 0 0 0 0 0 0 1 33% 196	6 3 4 2 4 2 2 1 24 59% 41	/ 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	1 4 3 3 1 2 2 1 17 41% 3	9 28 20 49 30 12 9 9 166 79% 210	/ 3 10 2 7 8 7 4 1 42 20% /	0 1 0 0 0 0 0 1 2 1% 67	0 0 0 0 0 0 1 0 1 2% 55	/ 6 2 4 0 2 7 1 3 25 45% /	5 3 2 2 12 0 2 29 53% 43	0 30 51 38 63 48 42 19 18
Md	APP/DEPART 4:00 PM 4:15 PM 4:30 PM 4:45 PM 5:00 PM 5:15 PM 5:30 PM 5:45 PM VOLUMES APPROACH % APP/DEPART BEGIN PEAK HR VOLUMES APPROACH %	0 0 1 0 0 0 0 0 0 1 33% 3	/ 0 0 0 0 1 0 0 0 1 33% / 4:15 PM 1 33%	0 0 1 0 0 0 0 0 0 1 33%	6 3 4 2 4 2 2 1 24 59% 41	/ 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	1 4 3 3 1 2 2 1 17 41% 3	9 28 20 49 30 12 9 9 166 79% 210	/ 3 10 2 7 8 7 4 1 42 20% /	0 1 0 0 0 0 0 1 2 1%	0 0 0 0 0 0 1 0 1 2% 55	/ 6 2 4 0 2 7 1 3 25 45% / 8 44%	5 3 2 2 12 0 2 29 53% 43	0 30 51 38 63 48 42 19 18 309
PM	APP/DEPART 4:00 PM 4:15 PM 4:30 PM 4:45 PM 5:00 PM 5:15 PM 5:30 PM 5:45 PM VOLUMES APPROACH % APP/DEPART BEGIN PEAK HR VOLUMES	0 0 1 0 0 0 0 0 0 1 33% 3	/ 0 0 0 1 0 0 0 1 33% / 4:15 PM	0 0 1 0 0 0 0 0 0 1 33% 196	6 3 4 2 4 2 2 1 24 59% 41	/ 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	1 4 3 3 1 2 2 1 17 41% 3	9 28 20 49 30 12 9 9 166 79% 210	/ 3 10 2 7 8 7 4 1 42 20% /	0 1 0 0 0 0 0 1 2 1% 67	0 0 0 0 0 0 1 0 1 2% 55	/ 6 2 4 0 2 7 1 3 25 45% /	5 3 2 2 12 0 2 29 53% 43	0 30 51 38 63 48 42 19 18 309



PREPARED BY: PACIFIC TECHNICAL DATA

DATE: 4/24/13 WEDNESDAY LOCATION: **IMPERIAL VALLEY**

LYONS

PROJECT #:

PTD13-0426-01

NORTH & SOUTH: **BROCKMAN**

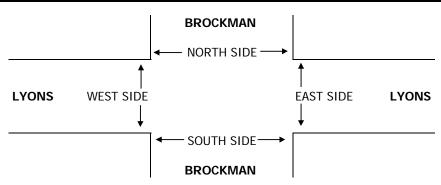
EAST & WEST:

LOCATION #: CONTROL:

2-WAY STOP (EW)

NOTES: \blacktriangle Ν **⋖**W E► S ▼

		NC	RTHBOU	ND	SOUTHBOUND			E,	ASTBOUN	ID	W	ESTBOUN	۷D	
			BROCKMAN			BROCKMAN			LYONS			LYONS		
		NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
	LANES:	0	1	0	0	1	0	0	1	0	0	1	0	
	5:30 AM	0	5	0	0	48	0	0	1	0	0	0	0	54
	5:45 AM	0	5	1	0	46	1	0	0	0	2	1	0	56
	6:00 AM	0	2	0	0	42	2	0	1	0	1	1	0	49
	6:15 AM	2	3	0	0	21	0	0	1	0	0	0	0	27
	6:30 AM	0	4	1	0	12	0	1	3	0	1	1	0	23
	6:45 AM	1	4	0	0	6	0	0	0	0	0	0	2	13
	7:00 AM	0	3	0	1	4	0	0	0	0	2	0	0	10
ΑM	7:15 AM	0	5	0	0	6	0	1	0	0	2	0	0	14
A	VOLUMES	3	31	2	1	185	3	2	6	0	8	3	2	246
	APPROACH %	8%	86%	6%	1%	98%	2%	25%	75%	0%	62%	23%	15%	
	APP/DEPART	36	/	35	189	/	193	8	/	9	13	/	9	0
	BEGIN PEAK HR		5:30 AM											
	VOLUMES	2	15	1	0	157	3	0	3	0	3	2	0	186
	APPROACH %	11%	83%	6%	0%	98%	2%	0%	100%	0%	60%	40%	0%	
	PEAK HR FACTOR		0.750			0.833			0.750			0.417		0.830
	APP/DEPART	18	/	15	160	/	160	3	/	4	5	/	7	0
	4:00 PM	0	5	0	0	5	0	2	0	0	1	1	0	14
	4:15 PM	0	21	0	1	0	0	0	2	0	1	0	0	25
	4:30 PM	0	28	0	0	8	1	0	0	0	0	0	0	37
	4:45 PM	0	36	1	0	1	1	0	1	0	0	1	1	42
	5:00 PM	0	52	2	0	2	1	0	0	0	0	0	0	57
	5:15 PM	0	19	2	0	5	0	0	0	0	0	1	0	27
	5:30 PM	0	10	0	0	2	0	0	0	0	0	0	0	12
Μ	5:45 PM	0	13	1	0	2	0	0	0	0	0	0	1	17
Ъ	VOLUMES	0	184	6	1	25	3	2	3	0	2	3	2	231
	APPROACH %	0%	97%	3%	3%	86%	10%	40%	60%	0%	29%	43%	29%	
	APP/DEPART	190	/	188	29	/	27	5	/	10	7	/	6	0
	BEGIN PEAK HR		4:30 PM											
	VOLUMES	0	135	5	0	16	3	0	1	0	0	2	1	163
	APPROACH %	0%	96%	4%	0%	84%	16%	0%	100%	0%	0%	67%	33%	
	PEAK HR FACTOR		0.648			0.528			0.250			0.375		0.715
	APP/DEPART	140	/	136	19	/	16	1	/	6	3	/	5	0



PREPARED BY: PACIFIC TECHNICAL DATA

DATE: 4/24/13 WEDNESDAY LOCATION: **IMPERIAL VALLEY**

PROJECT #:

PTD13-0426-01

NORTH & SOUTH: **BROCKMAN**

LOCATION #: CONTROL:

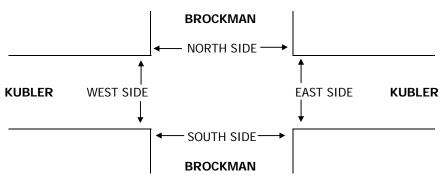
2-WAY STOP (EW)

EAST & WEST: NOTES:

KUBLER

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		NC	RTHBOU	ND	SC	UTHBOU	ND	E,	ASTBOUN	I D	W	ESTBOUN	ND	
			BROCKMAN			BROCKMAN			KUBLER			KUBLER		
		NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
	LANES:	0	1	0	0	1	0	0	1	0	0	1	0	
	5:30 AM	3	5	0	1	46	0	0	0	0	0	0	0	55
	5:45 AM	0	2	0	0	46	0	0	0	0	0	1	0	49
	6:00 AM	0	8	0	0	43	0	0	0	0	0	0	0	51
	6:15 AM	0	3	0	1	12	0	0	0	0	1	1	0	18
	6:30 AM	3	3	0	0	8	0	0	1	0	0	0	1	16
	6:45 AM	3	1	0	0	5	1	3	0	2	0	1	1	17
	7:00 AM	1	4	0	1	6	0	0	0	1	0	0	0	13
AM	7:15 AM	0	3	0	0	4	0	0	0	3	0	0	0	10
A	VOLUMES	10	29	0	3	170	1	3	1	6	1	3	2	229
	APPROACH %	26%	74%	0%	2%	98%	1%	30%	10%	60%	17%	50%	33%	
	APP/DEPART	39	/	34	174	/	177	10	/	4	6	/	14	0
	BEGIN PEAK HR		5:30 AM											
	VOLUMES	3	18	0	2	147	0	0	0	0	1	2	0	173
	APPROACH %	14%	86%	0%	1%	99%	0%	0%	0%	0%	33%	67%	0%	
	PEAK HR FACTOR		0.656			0.793			0.000			0.375		0.786
	APP/DEPART	21	/	18	149	/	148	0	/	2	3	/	5	0
	4:00 PM	0	5	5	0	4	0	0	0	0	1	0	0	15
	4:15 PM	0	31	0	0	3	0	1	0	1	0	0	3	39
	4:30 PM	0	15	1	2	2	0	1	0	2	0	0	0	23
	4:45 PM	0	56	1	1	0	0	1	0	0	0	0	0	59
	5:00 PM	0	34	2	0	1	0	1	0	0	0	0	1	39
	5:15 PM	0	10	0	1	2	0	4	0	0	1	0	0	18
	5:30 PM	0	9	0	1	0	1	1	1	1	0	0	0	14
PM	5:45 PM	0	8	0	0	1	0	0	0	0	0	0	0	9
Ь	VOLUMES	0	168	9	5	13	1	9	1	4	2	0	4	216
	APPROACH %	0%	95%	5%	26%	68%	5%	64%	7%	29%	33%	0%	67%	
	APP/DEPART	177	/	181	19	/	19	14	/	15	6	/	1	0
	BEGIN PEAK HR		4:15 PM											
	VOLUMES	0	136	4	3	6	0	4	0	3	0	0	4	160
	APPROACH %	0%	97%	3%	33%	67%	0%	57%	0%	43%	0%	0%	100%	
	PEAK HR FACTOR		0.614			0.563			0.583			0.333		0.678
	APP/DEPART	140	/	144	9	/	9	7	/	7	4	/	0	0



PREPARED BY: PACIFIC TECHNICAL DATA

DATE: 4/24/13 WEDNESDAY

LOCATION: NORTH & SOUTH: **IMPERIAL VALLEY BROCKMAN**

PROJECT #:

PTD13-0426-01

LOCATION #:

CONTROL:

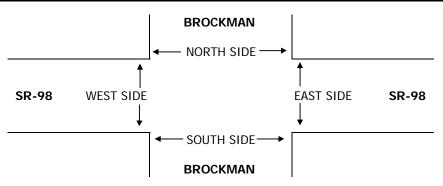
2-WAY STOP (NS)

EAST & WEST: NOTES:

SR-98

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		NC	RTHBOU	ND	SC	UTHBOU	ND	E.	ASTBOUN	ID	W	ESTBOUI	ND	
			BROCKMAN			BROCKMAN			SR-98			SR-98		
		NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
	LANES:	0	1	0	0	1	0	0	1	0	0	1	0	
	5:30 AM	0	0	0	0	35	12	0	5	1	26	74	5	158
	5:45 AM	1	2	1	0	40	9	0	5	10	35	33	3	139
	6:00 AM	0	1	1	0	37	6	1	13	4	44	40	3	150
	6:15 AM	2	1	2	2	9	4	2	11	2	10	23	1	69
	6:30 AM	3	7	0	1	4	4	0	6	1	2	25	3	56
	6:45 AM	4	4	0	3	2	2	0	5	3	4	16	0	43
	7:00 AM	1	1	0	3	2	3	0	11	3	0	8	3	35
AM	7:15 AM	1	3	1	0	6	3	0	6	0	0	14	3	37
⋖	VOLUMES	12	19	5	9	135	43	3	62	24	121	233	21	687
	APPROACH %	33%	53%	14%	5%	72%	23%	3%	70%	27%	32%	62%	6%	
	APP/DEPART	36	/	43	187	/	280	89	/	76	375	/	288	0
	BEGIN PEAK HR		5:30 AM											
	VOLUMES	3	4	4	2	121	31	3	34	17	115	170	12	516
	APPROACH %	27%	36%	36%	1%	79%	20%	6%	63%	31%	39%	57%	4%	
	PEAK HR FACTOR		0.550			0.786			0.750			0.707		0.816
	APP/DEPART	11	/	19	154	/	253	54	/	40	297	/	204	0
	4:00 PM	8	5	5	3	2	2	8	29	0	0	12	0	74
	4:15 PM	1	5	6	1	2	1	25	57	1	0	4	0	103
	4:30 PM	2	11	16	1	6	0	4	36	4	0	9	3	92
	4:45 PM	6	61	26	0	0	0	1	24	1	0	5	0	124
	5:00 PM	5	29	44	0	2	0	6	23	0	0	11	0	120
	5:15 PM	2	6	8	2	0	2	3	41	0	0	9	0	73
	5:30 PM	1	8	5	0	1	0	2	17	2	1	10	0	47
PM	5:45 PM	2	9	3	1	0	0	0	29	2	0	11	0	57
Д	VOLUMES	27	134	113	8	13	5	49	256	10	1	71	3	690
	APPROACH %	10%	49%	41%	31%	50%	19%	16%	81%	3%	1%	95%	4%	
	APP/DEPART	274	/	186	26	/	24	315	/	377	75	/	103	0
	BEGIN PEAK HR		4:15 PM											
	VOLUMES	14	106	92	2	10	1	36	140	6	0	29	3	439
	APPROACH %	7%	50%	43%	15%	77%	8%	20%	77%	3%	0%	91%	9%	
	PEAK HR FACTOR		0.570			0.464			0.548			0.667		0.885
	APP/DEPART	212	/	145	13	/	16	182	/	234	32	/	44	0



PREPARED BY: PACIFIC TECHNICAL DATA

DATE: 4/24/13 WEDNESDAY

LOCATION: IMPERIAL VALLEY NORTH & SOUTH:

PROJECT #: **BROCKMAN**

LOCATION #:

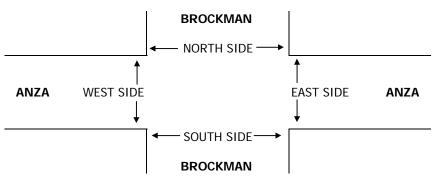
PTD13-0426-01

EAST & WEST: **ANZA**

CONTROL: 2-WAY STOP (NS)

NOTES:	AM		A	
	PM		N	
	MD	⋖ W		E►
	OTHER		S	
	OTHER		▼	

		NC	RTHBOU	ND	SOUTHBOUND		E,	ASTBOUN	ID	W	'ESTBOUN	ND .		
			BROCKMAN			BROCKMAN			ANZA			ANZA		
		NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
	LANES:	0	1	0	0	1	0	0	1	0	0	1	0	
	5:30 AM	0	0	0	0	0	2	0	0	0	0	5	0	7
	5:45 AM	0	0	0	2	0	0	0	1	0	0	6	1	10
	6:00 AM	0	0	0	0	0	2	0	2	0	0	2	2	8
	6:15 AM	0	0	0	0	0	0	0	1	0	0	2	1	4
	6:30 AM	0	0	0	0	0	3	1	0	0	0	3	1	8
	6:45 AM	0	0	0	0	0	3	0	0	0	0	1	3	7
	7:00 AM	0	0	0	1	0	0	1	1	0	0	3	1	7
AM	7:15 AM	0	0	0	0	0	0	0	1	0	0	1	1	3
⋖	VOLUMES	0	0	0	3	0	10	2	6	0	0	23	10	54
	APPROACH %	0%	0%	0%	23%	0%	77%	25%	75%	0%	0%	70%	30%	
	APP/DEPART	0	/	12	13	/	0	8	/	9	33	/	33	0
	BEGIN PEAK HR		5:45 AM											
	VOLUMES	0	0	0	2	0	5	1	4	0	0	13	5	30
	APPROACH %	0%	0%	0%	29%	0%	71%	20%	80%	0%	0%	72%	28%	
	PEAK HR FACTOR		0.000			0.583			0.625			0.643		0.750
	APP/DEPART	0	/	6	7	/	0	5	/	6	18	/	18	0
	4:00 PM	0	0	0	3	0	0	0	0	0	0	1	0	4
	4:15 PM	0	0	0	0	0	1	2	5	0	0	2	5	15
	4:30 PM	0	0	0	2	0	0	0	5	0	0	0	0	7
	4:45 PM	0	0	0	4	0	0	0	0	0	0	0	0	4
	5:00 PM	0	0	0	5	0	0	1	1	0	0	4	0	11
	5:15 PM	0	0	0	0	0	1	0	4	0	0	0	2	7
	5:30 PM	0	0	0	1	0	0	1	5	0	0	2	0	9
ΡM	5:45 PM	0	0	0	0	0	0	3	1	0	0	0	0	4
□	VOLUMES	0	0	0	15	0	2	7	21	0	0	9	7	61
	APPROACH %	0%	0%	0%	88%	0%	12%	25%	75%	0%	0%	56%	44%	
	APP/DEPART	0	/	14	17	/	0	28	/	36	16	/	11	0
	BEGIN PEAK HR		4:15 PM											
	VOLUMES	0	0	0	11	0	1	3	11	0	0	6	5	37
	APPROACH %	0%	0%	0%	92%	0%	8%	21%	79%	0%	0%	55%	45%	
	PEAK HR FACTOR		0.000			0.600			0.500			0.393		0.617
	APP/DEPART	0	/	8	12	/	0	14	/	22	11	/	7	0



PREPARED BY: PACIFIC TECHNICAL DATA

DATE: 4/24/13 WEDNESDAY LOCATION: **IMPERIAL VALLEY**

PROJECT #:

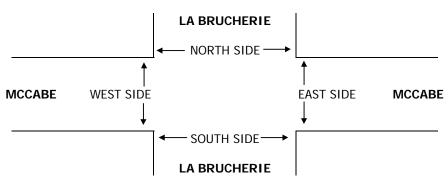
PTD13-0426-01

NORTH & SOUTH: LA BRUCHERIE LOCATION #: EAST & WEST: **MCCABE**

CONTROL: 4-WAY STOP

NOTES:	AM	
	PM N	
	MD ◀ W	E►
	OTHER S	
	OTHER ▼	

		NC	RTHBOU	ND	SC	UTHBOU	ND	E,	ASTBOUN	ID	W	ESTBOUN	ND	
		1	LA BRUCHERII	E		LA BRUCHERI	E		MCCABE			MCCABE		
		NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
	LANES:	0	1	0	0	1	0	0	1	0	0	1	0	
	5:30 AM	3	3	0	5	16	2	0	1	1	5	4	9	49
	5:45 AM	2	3	1	6	18	1	3	4	2	6	9	12	67
	6:00 AM	1	4	1	6	42	1	1	2	4	4	9	8	83
	6:15 AM	2	12	0	7	18	0	1	3	3	5	4	9	64
	6:30 AM	2	10	0	9	23	1	0	9	5	5	8	9	81
	6:45 AM	0	15	1	13	13	4	1	12	3	2	8	14	86
	7:00 AM	2	10	3	12	11	2	3	10	2	2	12	18	87
ΑM	7:15 AM	3	25	3	19	9	3	2	19	1	0	20	23	127
A	VOLUMES	15	82	9	77	150	14	11	60	21	29	74	102	644
	APPROACH %	14%	77%	8%	32%	62%	6%	12%	65%	23%	14%	36%	50%	
	APP/DEPART	106	/	195	241	/	200	92	/	146	205	/	103	0
	BEGIN PEAK HR		6:30 AM											
	VOLUMES	7	60	7	53	56	10	6	50	11	9	48	64	381
	APPROACH %	9%	81%	9%	45%	47%	8%	9%	75%	16%	7%	40%	53%	
	PEAK HR FACTOR		0.597			0.902			0.761			0.703		0.750
	APP/DEPART	74	/	130	119	/	76	67	/	110	121	/	65	0
	4:00 PM	4	10	1	21	9	4	9	67	3	2	28	19	177
	4:15 PM	2	20	0	19	13	1	7	14	6	0	10	18	110
	4:30 PM	6	13	4	17	19	3	4	12	2	0	6	21	107
	4:45 PM	3	29	3	30	14	4	3	8	0	0	13	25	132
	5:00 PM	2	21	4	24	13	1	2	19	0	0	25	40	151
	5:15 PM	5	26	2	22	16	2	1	6	0	1	14	17	112
	5:30 PM	3	15	7	21	12	1	0	5	2	1	15	31	113
ΡM	5:45 PM	4	15	2	19	17	4	2	5	0	1	15	21	105
Б	VOLUMES	29	149	23	173	113	20	28	136	13	5	126	192	1,007
	APPROACH %	14%	74%	11%	57%	37%	7%	16%	77%	7%	2%	39%	59%	
	APP/DEPART	201	/	369	306	/	131	177	/	332	323	/	175	0
	BEGIN PEAK HR		4:00 PM											
	VOLUMES	15	72	8	87	55	12	23	101	11	2	57	83	526
	APPROACH %	16%	76%	8%	56%	36%	8%	17%	75%	8%	1%	40%	58%	
	PEAK HR FACTOR		0.679			0.802			0.427			0.724		0.743
	APP/DEPART	95	/	178	154	/	68	135	/	196	142	/	84	0



PREPARED BY: PACIFIC TECHNICAL DATA

DATE: 4/24/13 WEDNESDAY

NOTES:

LOCATION: **IMPERIAL VALLEY** NORTH & SOUTH:

LA BRUCHERIE

PROJECT #:

PTD13-0426-01

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E►

LOCATION #:

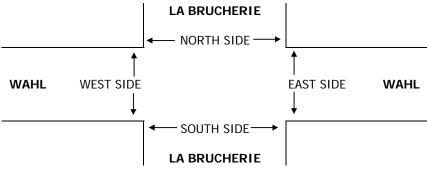
2-WAY STOP (NS)

⋖W

EAST & WEST: **WAHL**

CONTROL:

											OTHER		▼	
			RTHBOU			UTHBOU LA BRUCHERII		E,	ASTBOUN WAHL	ND	W	ESTBOUN WAHL	ND	
		NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
	LANES:	0	1	0	0	1	0	0	1	0	0	1	0	
	5:30 AM	1	2	0	0	19	1	1	3	0	2	4	0	33
	5:45 AM	5	3	0	2	23	1	0	3	2	1	4	0	44
	6:00 AM	1	3	0	0	52	1	0	1	4	2	2	0	66
	6:15 AM	1	4	0	0	29	2	0	1	3	0	3	1	44
	6:30 AM	0	4	0	4	27	2	2	3	3	0	4	0	49
	6:45 AM	0	10	0	0	9	2	1	5	1	2	3	0	33
	7:00 AM	1	1	1	4	11	3	0	3	0	0	3	0	27
ΑM	7:15 AM	0	10	0	2	4	0	0	3	2	0	4	1	26
A	VOLUMES	9	37	1	12	174	12	4	22	15	7	27	2	322
	APPROACH %	19%	79%	2%	6%	88%	6%	10%	54%	37%	19%	75%	6%	
	APP/DEPART	47	/	43	198	/	196	41	/	35	36	/	48	0
	BEGIN PEAK HR		5:45 AM											
	VOLUMES	7	14	0	6	131	6	2	8	12	3	13	1	203
	APPROACH %	33%	67%	0%	4%	92%	4%	9%	36%	55%	18%	76%	6%	
	PEAK HR FACTOR		0.656			0.675			0.688			0.850		0.769
	APP/DEPART	21	/	17	143	/	146	22	/	14	17	/	26	0
	4:00 PM	2	4	1	4	6	2	0	8	3	3	2	1	36
	4:15 PM	2	14	0	3	3	0	0	5	4	0	1	1	33
	4:30 PM	0	17	0	6	10	0	1	0	0	0	1	1	36
	4:45 PM	3	23	3	3	8	0	1	4	1	0	2	0	48
	5:00 PM	0	24	1	5	5	1	1	2	1	1	1	1	43
	5:15 PM	3	37	3	3	7	0	0	6	0	0	1	1	61
	5:30 PM	2	10	2	4	3	1	0	1	2	0	1	1	27
PM	5:45 PM	1	11	0	3	11	0	0	5	0	0	0	0	31
Ы	VOLUMES	13	140	10	31	53	4	3	31	11	4	9	6	315
	APPROACH %	8%	86%	6%	35%	60%	5%	7%	69%	24%	21%	47%	32%	
	APP/DEPART	163	/	149	88	/	68	45	/	72	19	/	26	0
	BEGIN PEAK HR		4:30 PM											
	VOLUMES	6	101	7	17	30	1	3	12	2	1	5	3	188
	APPROACH %	5%	89%	6%	35%	63%	2%	18%	71%	12%	11%	56%	33%	
	PEAK HR FACTOR		0.663			0.750			0.708			0.750		0.770
	APP/DEPART	114	/	107	48	/	33	17	/	36	9	/	12	0



PREPARED BY: PACIFIC TECHNICAL DATA

DATE: 4/24/13 LOCATION: **IMPERIAL VALLEY** PROJECT #:

PTD13-0426-01

NORTH & SOUTH: **FERRELL**

LOCATION #:

10 2-WAY STOP (EW)

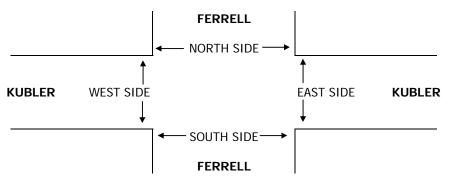
WEDNESDAY NOTES:

EAST & WEST: **KUBLER**

CONTROL:

 \blacktriangle Ν **⋖**W E► S

		NC	ORTHBOU	ND	SC	UTHBOU	ND	E	ASTBOUN	ID	W	ESTBOUN	ND	
			FERRELL			FERRELL			KUBLER			KUBLER		
		NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
	LANES:	0	1	0	0	1	0	0	1	0	0	1	0	
	5:30 AM	0	2	0	0	19	1	1	0	0	0	1	1	25
	5:45 AM	0	8	0	1	24	0	0	0	0	0	0	0	33
	6:00 AM	3	5	0	1	35	1	0	0	0	1	0	1	47
	6:15 AM	2	3	0	1	40	3	0	0	0	0	0	0	49
	6:30 AM	3	2	0	1	23	3	0	1	0	0	0	0	33
	6:45 AM	7	7	1	0	15	5	1	0	1	0	1	0	38
	7:00 AM	1	4	1	0	7	1	1	0	1	0	1	0	17
ΑM	7:15 AM	1	6	0	3	11	0	2	0	3	2	0	0	28
₹	VOLUMES	17	37	2	7	174	14	5	1	5	3	3	2	270
	APPROACH %	30%	66%	4%	4%	89%	7%	45%	9%	45%	38%	38%	25%	
	APP/DEPART	56	/	44	195	/	182	11	/	10	8	/	34	0
	BEGIN PEAK HR		6:00 AM											
	VOLUMES	15	17	1	3	113	12	1	1	1	1	1	1	167
	APPROACH %	45%	52%	3%	2%	88%	9%	33%	33%	33%	33%	33%	33%	
	PEAK HR FACTOR		0.550			0.727			0.375			0.375		0.852
	APP/DEPART	33	/	19	128	/	115	3	/	5	3	/	28	0
	4:00 PM	3	8	0	0	5	2	1	1	3	0	1	0	24
	4:15 PM	1	10	0	0	8	5	0	1	0	0	0	0	25
	4:30 PM	2	16	0	0	3	0	1	0	4	1	1	0	28
	4:45 PM	1	30	0	0	10	0	0	1	0	2	0	0	44
	5:00 PM	7	23	1	0	11	0	2	2	1	0	0	2	49
	5:15 PM	4	29	0	1	5	0	4	0	1	0	0	0	44
	5:30 PM	1	20	2	0	6	0	6	0	2	0	0	0	37
₽	5:45 PM	0	11	1	0	5	0	2	0	0	0	0	0	19
۵	VOLUMES	19	147	4	1	53	7	16	5	11	3	2	2	270
	APPROACH %	11%	86%	2%	2%	87%	11%	50%	16%	34%	43%	29%	29%	
	APP/DEPART	170	/	165	61	/	67	32	/	10	7	/	28	0
	BEGIN PEAK HR		4:45 PM											
	VOLUMES	13	102	3	1	32	0	12	3	4	2	0	2	174
	APPROACH %	11%	86%	3%	3%	97%	0%	63%	16%	21%	50%	0%	50%	
	PEAK HR FACTOR		0.894			0.750			0.594			0.500		0.888
	APP/DEPART	118	/	116	33	/	38	19	/	7	4	/	13	0



PREPARED BY: PACIFIC TECHNICAL DATA

DATE: 4/24/13 WEDNESDAY LOCATION: **IMPERIAL VALLEY**

SR-98

PROJECT #:

PTD13-0426-01

NORTH & SOUTH: **FARRELL**

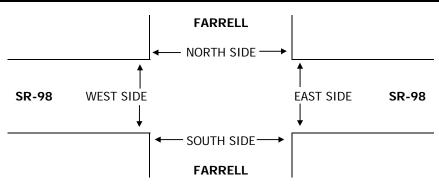
EAST & WEST:

LOCATION #: CONTROL:

2-WAY STOP (NS)

NOTES: \blacktriangle Ν **⋖**W E► S

		NC	RTHBOU	ND	SC	UTHBOU	ND	E/	ASTBOUN	ID	W	ESTBOUN	۱D	
			FARRELL			FARRELL			SR-98			SR-98		
		NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
	LANES:	0	1	0	0	1	0	0	1	0	0	1	0	
	5:30 AM	1	1	0	0	9	12	0	4	0	6	104	6	143
	5:45 AM	2	1	0	0	14	8	0	4	0	26	58	6	119
	6:00 AM	1	1	0	2	25	17	2	9	2	15	66	5	145
	6:15 AM	0	5	1	5	27	6	0	13	3	15	14	3	92
	6:30 AM	0	8	0	8	23	3	2	5	1	8	28	4	90
	6:45 AM	1	4	0	8	9	3	2	8	0	7	14	8	64
	7:00 AM	0	2	1	0	8	0	1	6	0	5	14	4	41
ΑM	7:15 AM	0	4	1	6	10	0	1	6	1	4	12	6	51
₹	VOLUMES	5	26	3	29	125	49	8	55	7	86	310	42	745
	APPROACH %	15%	76%	9%	14%	62%	24%	11%	79%	10%	20%	71%	10%	
	APP/DEPART	34	/	76	203	/	218	70	/	87	438	/	364	0
	BEGIN PEAK HR		5:30 AM											
	VOLUMES	4	8	1	7	75	43	2	30	5	62	242	20	499
	APPROACH %	31%	62%	8%	6%	60%	34%	5%	81%	14%	19%	75%	6%	
	PEAK HR FACTOR		0.542			0.710			0.578			0.698		0.860
	APP/DEPART	13	/	30	125	/	142	37	/	38	324	/	289	0
	4:00 PM	0	3	1	3	4	1	2	12	0	2	12	2	42
	4:15 PM	1	8	1	6	3	0	7	45	0	0	6	4	81
	4:30 PM	2	12	7	5	7	0	4	53	0	1	6	3	100
	4:45 PM	0	17	6	9	8	1	14	45	0	0	8	5	113
	5:00 PM	0	6	10	3	4	1	11	43	1	0	10	4	93
	5:15 PM	1	26	10	5	3	1	2	56	0	10	8	1	123
	5:30 PM	0	11	7	5	1	0	2	39	0	0	10	0	75
Μ	5:45 PM	0	7	5	7	1	0	4	19	1	0	11	2	57
ᆸ	VOLUMES	4	90	47	43	31	4	46	312	2	13	71	21	684
	APPROACH %	3%	64%	33%	55%	40%	5%	13%	87%	1%	12%	68%	20%	
	APP/DEPART	141	/	157	78	/	46	360	/	402	105	/	79	0
	BEGIN PEAK HR		4:30 PM											
	VOLUMES	3	61	33	22	22	3	31	197	1	11	32	13	429
	APPROACH %	3%	63%	34%	47%	47%	6%	14%	86%	0%	20%	57%	23%	
	PEAK HR FACTOR		0.655			0.653			0.970			0.737		0.872
	APP/DEPART	97	/	105	47	/	34	229	/	252	56	/	38	0



PREPARED BY: PACIFIC TECHNICAL DATA

DATE: 4/24/13 WEDNESDAY LOCATION: IMPERIAL VALLEY

PROJECT #:

PTD13-0426-01

NORTH & SOUTH: EAST & WEST:

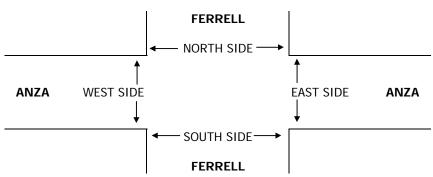
FERRELL ANZA

LOCATION #: CONTROL:

12 2-WAY STOP (NS)

NOTES:	AM	A	
	PM	N	
	MD ⋖ W	•	E►
	OTHER	S	
	OTHER	\blacksquare	

		NC	ORTHBOU	ND	SO	UTHBOU	ND	E,	ASTBOUN	ID	W	ESTBOUN	ND .	
			FERRELL			FERRELL			ANZA			ANZA		
		NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
	LANES:	0	1	0	0	1	0	0	1	0	0	1	0	
	5:30 AM	0	0	0	1	0	2	1	0	0	0	1	0	5
	5:45 AM	0	0	0	1	0	4	1	1	0	0	5	2	14
	6:00 AM	0	0	0	1	0	1	1	0	0	0	2	0	5
	6:15 AM	0	0	0	3	0	3	2	0	0	0	1	1	10
	6:30 AM	0	0	0	4	1	3	1	0	0	0	2	1	12
	6:45 AM	0	1	0	3	1	7	0	0	0	0	1	2	15
	7:00 AM	0	1	0	1	0	3	2	1	0	0	0	0	8
AM	7:15 AM	0	0	0	7	0	5	2	0	0	0	1	3	18
Α	VOLUMES	0	2	0	21	2	28	10	2	0	0	13	9	87
	APPROACH %	0%	100%	0%	41%	4%	55%	83%	17%	0%	0%	59%	41%	
	APP/DEPART	2	/	21	51	/	2	12	/	23	22	/	41	0
	BEGIN PEAK HR		6:30 AM											
	VOLUMES	0	2	0	15	2	18	5	1	0	0	4	6	53
	APPROACH %	0%	100%	0%	43%	6%	51%	83%	17%	0%	0%	40%	60%	
	PEAK HR FACTOR		0.500			0.729			0.500			0.625		0.736
	APP/DEPART	2	/	13	35	/	2	6	/	16	10	/	22	0
	4:00 PM	0	0	0	2	0	1	1	1	0	0	1	2	8
	4:15 PM	0	0	0	2	0	3	9	2	0	0	0	2	18
	4:30 PM	0	0	0	3	0	0	5	5	0	0	0	0	13
	4:45 PM	0	0	0	4	0	1	2	2	0	0	2	0	11
	5:00 PM	0	1	0	2	0	4	2	7	1	0	1	1	19
	5:15 PM	0	0	0	0	0	0	2	2	0	0	3	7	14
	5:30 PM	0	0	0	3	0	1	5	1	0	0	0	2	12
PM	5:45 PM	0	1	0	3	0	0	1	1	0	0	1	0	7
Ь	VOLUMES	0	2	0	19	0	10	27	21	1	0	8	14	102
	APPROACH %	0%	100%	0%	66%	0%	34%	55%	43%	2%	0%	36%	64%	
	APP/DEPART	2	/	43	29	/	1	49	/	40	22	/	18	0
	BEGIN PEAK HR		4:15 PM											
	VOLUMES	0	1	0	11	0	8	18	16	1	0	3	3	61
	APPROACH %	0%	100%	0%	58%	0%	42%	51%	46%	3%	0%	50%	50%	
	PEAK HR FACTOR		0.250			0.792			0.795			0.750		0.803
1	APP/DEPART	1	/	22	19	/	1	35	/	27	6	/	11	0



PEAK HOUR VOLUME DATA

Peak hour volume data consists of hourly volume relationships and data location. The hourly volumes are expressed as a percentage of the Annual Average Daily Traffic (AADT). The percentages are shown for both the AM and the PM peak periods.

The principle data described here are the K factor, the D factor and their product (KD). The K factor is the percentage of AADT during the peak hour for both directions of travel. The D factor is the percentage of the peak hour travel in the peak direction. KD multiplied with the AADT gives the one way peak period directional flow rate or the design hourly volume (DHV). The design hourly volume is used for either Operational Analysis or Design Analysis. Refer to the 2000 Highway Capacity Manual for more details.

Following is a glossary of terms used in this listing of peak hour volume data:

Dir Indicates direction of travel for peak volume

AADT Annual Average Daily Traffic in vehicles per day (vpd).

AM Peak Represents the morning peak period for traffic analysis

CS Control Station Number, Caltrans identification number for

monitoring site.

CO County abbreviation used by Caltrans

D factor. The percentage of traffic in the peak direction during the

peak hour. Values in this book are derived by dividing the measured

PHV by the sum of both directions of travel during the peak hour.

DAY Day of week for the peak volume.

DDHV The directional design hour volume, in vehicles per hour (vph)

DDHV=AADTxKxD. See equation (8-1) on page 8-11 of the 2000

Highway Capacity Manual.

DI Caltrans has twelve transportation districts statewide. This

abbreviation identifies the district in which the count station is

located.

HR The ending time for the peak hour volume listed. The volume

observed fro 1 to 2 would be recorded as 2.

K The percentage of the AADT in both directions during the peak hour.

Values in this table are derived by dividing the measured 2-way PHV

by the AADT.

KD The product of K and D. The percentage of AADT in the peak direction during the peak hour. Values in this table are derived by

dividing the measured 1-way PHV by the AADT.

LEG For traffic counting purposes, a highway intersection or interchange

is assigned two legs according to increasing postmiles (route direction) and with a postmile reference at the center of the intersection or interchange. The volume of traffic on each leg is denoted by an A, B or O. A = ahead leg, B = back leg, and O – traffic volume being same for both back and ahead legs.

MNTH The month that the peak volume occurred.

PHV Peak Hour Volume in the peak direction. A one way volume in

vehicles per hour (vph) as used here. The PHV is analogous to the

DDHV as used for design purposes.

PM The Post Mile is the mileage measured from the county line, or from

the beginning of a route. Each postmile along a route in a county is

a unique location on the state highway system.

PM Peak Represents the afternoon peak period for traffic analysis.

PRE The postmile may have a prefix like R, T, L, M, etc. When a length of

highway is changed due to construction or realigment, new postmile values are assigned. To distinguish the new values from the old, an

alpha code is prefixed to the new postmile.

RTE The state highway route number

YR The year when the count was made. Traffic counting is on a 3-year

cycle.

LATEST TRAFFIC YEAR SELECTED

PEAK HOUR VOLUME DATA

									AM	PEAK							PM	PEAK				
DI	RTE	co	PRE	PM	CS	LEG YR	Dir	1 WAY PHV	% K	% D	% KD	HR	DAY	MNTH D	ir	1 WAY PHV	% K	% D	% KD	HR	DAY	MNTH
11	008	SD	L	1.213	958	A 08	E	4637	7.47	61.45	4.59	7	TUE	FEB	W	4604	8.33	54.73	4.56	17	FRI	AUG
11	800	SD		.946	804	A 08	W	8170	7.41	57.07	4.23	7	THU	SEP	E	8446	8.02	54.48	4.37	16	TUE	MAR
11	800	SD		5.638	953	в 08	W	11617	7.43	64.73	4.81	7	TUE	APR	E	10959	7.96	56.96	4.53	15	THU	DEC
11	800	SD		8.336	807	в 08	W	11072	8.06	60.93	4.91	7	THU	NOV	E	10737	8.02	59.36	4.76	15	WED	OCT
11	800	SD		8.336	808	A 08	W	10170	7.6	67.39	5.12	7	THU	MAY	E	9780	7.99	61.61	4.92	16	FRI	JAN
11	800	SD		11.76	810	в 08	W	8307	6.82	63.17	4.31	7	THU	JAN	E	9011	8.24	56.73	4.67	16	WED	FEB
11	800	SD		14.59	806	в 07	W	8456	6.87	59.41	4.08	7	THU	OCT	E	9132	8.15	54.13	4.41	15	THU	DEC
11	800	SD	R	18.73	824	в 08	W	4555	7.07	69.67	4.93	7	TUE	OCT	E	4273	8.06	57.38	4.62	15	TUE	NOV
11	800	SD	R	20.04	888	в 08	W	3944	7.07	69.41	4.9	7	TUE	MAR	E	3787	8.05	58.53	4.71	17	FRI	APR
11	800	SD	R	23.64	979	0 08	W	2444	7.79	55.9	4.35	12	FRI	DEC	W	2926	8.57	60.81	5.21	17	WED	NOV
11	800	SD	R	37.83	811	A 08	E	1143	8.94	64.36	5.76	10	FRI	NOV	W	1404	11.46	61.69	7.07	15	WED	DEC
11	800	SD	R	51.98	621	в 08	E	999	11.26	56.73	6.39	11	THU	NOV	W	1284	12.29	66.81	8.21	14	MON	FEB
11	800	SD	R	65.90	981	A 08	E	1001	12.07	59.55	7.19	10	WED	DEC	E	1189	14.5	58.86	8.53	16	SUN	JUL
11	800	IMP	R	10.29	993	в 08	W	984	11.35	61.85	7.02	11	MON	FEB	W	1180	12.22	68.89	8.42	15	TUE	JAN
11	800	IMP	R	10.29	994	A 08	E	914	14.57	51.55	7.51	12	MON	MAY	W	1079	12.69	69.84	8.87	15	TUE	JAN
11	800	IMP	R	23.48	624	A 08	W	872	9.63	73.84	7.11	9	FRI	JUL	W	1038	15.17	55.81	8.46	15	MON	MAY
11	800	IMP	R	36.97	982	в 08	E	1034	10.76	53	5.7	12	SAT	DEC	W	1215	10.94	61.24	6.7	15	SAT	NOV
11	800	IMP	R	40.94	638	в 08	W	1401	8.35	53.37	4.46	12	MON	MAY	E	1805	9.17	62.63	5.74	18	FRI	MAY
11	800	IMP	R	53.50	964	A 08	E	909	12.78	61.21	7.82	10	SAT	DEC	W	1018	15.25	57.42	8.76	13	SAT	NOV
11	800	IMP	R	96.55	995	в 08	E	1276	12.1	54.39	6.58	12	FRI	FEB	E	1300	10.71	62.65	6.71	13	MON	SEP
11	800	IMP	R	96.99	988	в 08	E	1097	11.54	56.58	6.53	12	MON	JAN	E	1173	11.9	58.71	6.98	15	MON	FEB
05	009	SCR		.63	681	A 08	S	380	8.29	91.79	7.61	8	TUE	DEC	S	390	8.27	94.43	7.81	17	MON	DEC
05	009	SCR		8.11	430	в 08	S	1364	8.35	78.89	6.58	7	THU	MAR	N	1250	9.09	66.38	6.03	17	TUE	DEC
05	009	SCR		13.04	169	в 08	N	731	9.14	64.92	5.93	10	WED	DEC	N	643	8.85	58.99	5.22	17	MON	DEC
05	009	SCR		27.09	49	в 08	N	294	12.23	97.35	11.91	7	MON	JUN	S	233	11.06	85.35	9.44	17	WED	SEP
04	009	SCL		7.09	170	A 07	S	456	10.67	61.13	6.52	11	SAT	JUL	N	537	9.69	79.2	7.68	22	SAT	JUL
04	009	SCL		11.45	171	в 07	N	1613	7.59	60.8	4.62	8	WED	OCT	N	1841	8.84	59.64	5.27	15	TUE	JAN
07	010	LA		18.41	456	в 08	W	819	11.39	93.81	10.69	9	FRI	DEC	E	580	9.9	76.42	7.57	15	FRI	JUL
07	010	LA		19.71	783	0 08	W	868	11.22	92.34	10.36	9	THU	OCT	E	569	8.93	76.07	6.79	17	THU	NOV
07	010	LA		24.31	785	A 08	W	1498	6.78	86.74	5.88	9	WED	MAR	E	1523	8.2	72.98	5.98	15	WED	MAR
07	010	LA	R	3.89	402	в 06	W	7499	7.61	52.15	3.97	7	WED	SEP	E	6834	6.82	53.07	3.62	14	WED	MAY
07	010	LA		24.32	721	A 08	E	7451	6.26	53.18	3.33	12	SAT	SEP	E	7695	6.01	57.18	3.43	16	TUE	AUG
07	010	LA		30.3	429	A 08	W	7633	6.41	55.24	3.54	10	SAT	MAR	E	7707	6.31	56.63	3.57	14	WED	MAR

2007

Annual Average Daily Truck Traffic on the California State Highway System

Compiled by
Traffic Data Branch
Division of Traffic Operations

State of California
Business, Transportation and Housing Agency
Department of Transportation

Prepared in cooperation with the U.S. Department of Transportation Federal Highway Administration

SEPTEMBER 2008

L POST E RTEXDIST CNTY MILE G DESCRIPTION	AADT	TRUCK T AADT % TOTAL	RUCK TOT VEH		4			% 2	TRUCK By Ax 3			EAL 2-WAY (1000)	YEAR VER/ EST
0087 11 IMP R10.01 A JCT. RTE. 9	8 12200	1696	13.9	607	78	39	972	35.8	4.6	2.3	57.3	369	05E
008 11 IMP R23.48 A DUNAWAY ROA	D 12300	1931	15.7	583	90	39	1219	30.18	4.68	2.03	63.12	455	08V
008 11 IMP R29.933 B DREW ROAD	12300	1998 1	6.24	592	93	37	1277	29.63	4.63	1.85	63.89	475	05E
00 & 11 IMP R37.972 B JCT. RTE. 8	6 32500	3478	10.7	L120	191	77	2090	32.2	5.5	2.2	60.1	789	05E
008 11 IMP R37.972 A JCT. RTE. 8	6 34500	3509 1	0.17	L131	192	77	2109	32.24	5.46	2.19	60.11	796	05E
008 11 IMP R40.944 B JCT. RTE. 1	11 31500	2844	9.03	684	178	52	1930	24.04	6.27	1.83	67.85	714	08V
008 11 IMP R40.944 A JCT. RTE. 1	11 14600	3358	23	860	222	87	2189	25.6	6.6	2.6	65.2	819	00E
0084 11 IMP R53.497 B JCT. RTE. 1	15 NORTH 11000	3300	30	845	218	86	2152	25.6	6.6	2.6	65.2	805	00E
00 g 11 IMP R53.497 A JCT. RTE. 1	15 NORTH 11600	3074	26.5	787	203	80	2004	25.6	6.6	2.6	65.2	749	00E
00 11 IMP R65.752 B EAST JCT. R	TE. 98 WEST 11600	3074	26.5	787	203	80	2004	25.6	6.6	2.6	65.2	749	00E
008 11 IMP R65.752 A EAST JCT. R	TE. 98 WEST 14000	3360	24	860	222	87	2191	25.6	6.6	2.6	65.2	819	00E
008 11 IMP R96.546 B 4TH AVENUE	19400	1505	7.76	279	110	78	1038	18.51	7.31	5.21	68.96	389	06V
008 11 IMP R96.986 B ARIZONA STA	TE LINE 16800	3259	19.4	834	215	85	2125	25.6	6.6	2.6	65.2	795	00E

Appendix I

Existing Intersection LOS Calculations

AM Existing
1: Forrester Road & I-8 WB Ramp

HCM Unsignalized Intersection Capacity Analysis

						-		.		-		
	•	-	•	•	•	•	1	Ť	~	-	¥	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
Lane Configurations					ર્ન	7		ર્ન			î,	
Volume (veh/h)	0	0	0	94	0	121	4	30	0	0	97	64
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	0	102	0	132	4	33	0	0	105	70
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)						2						
Median type								None			None	
Median storage veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	247	182	140	182	216	33	175			33		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	247	182	140	182	216	33	175			33		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	87	100	87	100			100		
cM capacity (veh/h)	616	710	908	778	679	1041	1401			1579		
Direction, Lane #	WB 1	NB 1	SB 1									
Volume Total	234	37	175									
Volume Left	102	4	0									
Volume Right	132	0	70									
cSH	1780	1401	1700									
Volume to Capacity	0.13	0.00	0.10									
Queue Length 95th (ft)	11	0	0									
Control Delay (s)	9.6	0.9	0.0									
Lane LOS	Α	Α										
Approach Delay (s)	9.6	0.9	0.0									
Approach LOS	Α											
Intersection Summary												
Average Delay			5.1									
Intersection Capacity Utiliza	ation		20.9%	IC	U Level	of Service			Α			
Analysis Period (min)			15									

LOS Engineering, Inc. Synchro

AM Existing

HCM Unsignalized Intersection Capacity Analysis

	•	-	•	•	←	•	4	†	~	-	ļ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBI
Lane Configurations		4	7					ĵ.			ની	
Volume (veh/h)	19	Ö	2	0	0	0	0	15	6	45	152	(
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	21	0	2	0	0	0	0	16	7	49	165	(
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)			2									
Median type								None			None	
Median storage veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	283	286	165	284	283	20	165			23		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	283	286	165	284	283	20	165			23		
C, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	97	100	100	100	100	100	100			97		
cM capacity (veh/h)	654	604	879	651	607	1058	1413			1592		
Direction, Lane #	EB 1	NB 1	SB 1									
Volume Total	23	23	214									
Volume Left	21	0	49									
Volume Right	2	7	0									
cSH	723	1700	1592									
Volume to Capacity	0.03	0.01	0.03									
Queue Length 95th (ft)	2	0	2									
Control Delay (s)	10.5	0.0	1.9									
Lane LOS	В		Α									
Approach Delay (s)	10.5	0.0	1.9									
Approach LOS	В											
Intersection Summary												
Average Delay			2.5									
Intersection Capacity Utiliza	tion		27.2%	IC	U Level o	of Service			Α			
Analysis Period (min)			15									

AM Existing
3: Forrester Road & McCabe Rd

HCM Unsignalized Intersection Capacity Analysis

J. I Ollestel Road		DC IXG										
	•	-	•	•	•	•	^	†		-	↓	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
Lane Configurations		43-			4			4			44	
Volume (veh/h)	4	6	0	1	30	9	0	1	0	17	0	125
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	4	7	0	1	33	10	0	1	0	18	0	136
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	42			7			191	60	7	55	55	38
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	42			7			191	60	7	55	55	38
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			100			100	100	100	98	100	87
cM capacity (veh/h)	1567			1614			666	828	1076	939	833	1035
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	11	43	1	154								
Volume Left	4	1	0	18								
Volume Right	0	10	0	136								
cSH	1567	1614	828	1022								
Volume to Capacity	0.00	0.00	0.00	0.15								
Queue Length 95th (ft)	0	0	0	13								
Control Delay (s)	2.9	0.2	9.4	9.1								
Lane LOS	Α	Α	Α	Α								
Approach Delay (s)	2.9	0.2	9.4	9.1								
Approach LOS			Α	Α								
Intersection Summary												
Average Delay			7.0									
Intersection Capacity Utiliza	ation		25.3%	IC	CU Level o	of Service			Α			
Analysis Period (min)			15									

LOS Engineering, Inc. Synchro AM Existing

	•	_	`	_	•	•	•	†	<i>></i>	\	1	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			43-			4			4	
Volume (veh/h)	0	3	0	3	2	0	2	15	1	0	157	3
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	3	0	3	2	0	2	16	1	0	171	3
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	195	194	172	195	195	17	174			17		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	195	194	172	195	195	17	174			17		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	100	100	100	100			100		
cM capacity (veh/h)	762	700	871	761	699	1062	1403			1600		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	3	5	20	174								
Volume Left	0	3	2	0								
Volume Right	0	0	1	3								
cSH	700	735	1403	1600								
Volume to Capacity	0.00	0.01	0.00	0.00								
Queue Length 95th (ft)	0	1	0	0								
Control Delay (s)	10.2	9.9	0.9	0.0								
Lane LOS	В	Α	Α									
Approach Delay (s)	10.2	9.9	0.9	0.0								
Approach LOS	В	Α										
Intersection Summary												
Average Delay			0.5									
Intersection Capacity Utilizati	ion		18.4%	IC	U Level	of Service			Α			
Analysis Period (min)			15									

AM Existing 5: Brockman Rd & Kubler Rd

HCM Unsignalized Intersection Capacity Analysis

	•	-	•	•	•	•	4	†	1	-	↓	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
Lane Configurations		44			44			43-			4	
Volume (veh/h)	0	0	0	1	2	0	3	18	0	2	147	
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.9
Hourly flow rate (vph)	0	0	0	1	2	0	3	20	0	2	160	
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	191	190	160	190	190	20	160			20		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	191	190	160	190	190	20	160			20		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	100	100	100	100			100		
cM capacity (veh/h)	765	702	885	768	702	1058	1419			1597		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	0	3	23	162								
Volume Left	0	1	3	2								
Volume Right	0	0	0	0								
cSH	1700	723	1419	1597								
Volume to Capacity	0.01	0.00	0.00	0.00								
Queue Length 95th (ft)	0	0	0	0								
Control Delay (s)	0.0	10.0	1.1	0.1								
Lane LOS	Α	В	Α	Α								
Approach Delay (s)	0.0	10.0	1.1	0.1								
Approach LOS	Α	В										
Intersection Summary												
Average Delay			0.4									
Intersection Capacity Utiliza	ition		18.0%	IC	CU Level of	of Service			Α			
Analysis Period (min)			15									

LOS Engineering, Inc. Synchro AM Existing

6: Broc	kman Rd	& SF	₹-98
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HCM Unsignalized Intersection Capacity Analysis

	•	-	•	•	←	•	4	†	~	-	ļ	1
Movement	EBL	EBT	EBR	WBL	WBT V	VBR	NBL	NBT	NBR	SBL	SBT	SBI
Lane Configurations		44			4			43-			4	
Volume (veh/h)	3	34	17	115	170	12	3	4	4	2	121	3
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	3	37	18	125	185	13	3	4	4	2	132	3.
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	198			55			593	501	46	501	503	19
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	198			55			593	501	46	501	503	19
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			92			99	99	100	100	70	96
cM capacity (veh/h)	1375			1549			288	433	1023	445	432	850
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	59	323	12	167								
Volume Left	3	125	3	2								
Volume Right	18	123	4	34								
cSH	1375	1549	467	479								
Volume to Capacity	0.00	0.08	0.03	0.35								
Queue Length 95th (ft)	0.00	7	0.03	39								
Control Delay (s)	0.4	3.3	12.9	16.5								
Lane LOS	0.4 A	3.3 A	12.9 B	10.5 C								
		3.3	12.9	16.5								
Approach Delay (s)	0.4	3.3		16.5 C								
Approach LOS			В	C								
Intersection Summary												
Average Delay			7.2									
Intersection Capacity Utiliza	tion		37.8%	IC	U Level of S	ervice			Α			
Analysis Period (min)			15									

AM Existing
7: Brockman Rd & Anza Rd

HCM Unsignalized Intersection Capacity Analysis

	•	-	•	•	←	•	•	†	~	-	ļ	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Volume (veh/h)	1	4	0	0	13	5	0	0	0	2	0	5
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	1	4	0	0	14	5	0	0	0	2	0	5
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	20			4			29	26	4	23	23	17
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	20			4			29	26	4	23	23	17
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			100			100	100	100	100	100	99
cM capacity (veh/h)	1597			1617			975	867	1079	988	869	1062
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	5	20	0	8								
Volume Left	1	0	0	2								
Volume Right	0	5	0	5								
cSH	1597	1617	1700	1040								
Volume to Capacity	0.00	0.00	0.00	0.01								
Queue Length 95th (ft)	0	0	0	1								
Control Delay (s)	1.5	0.0	0.0	8.5								
Lane LOS	Α		Α	Α								
Approach Delay (s)	1.5	0.0	0.0	8.5								
Approach LOS			Α	Α								
Intersection Summary												
Average Delay			2.2									
Intersection Capacity Utiliza	tion		13.3%	IC	U Level	of Service			Α			
Analysis Period (min)			15									

LOS Engineering, Inc. Synchro

AM Existing 8: La Brucherie Rd & McCabe Rd

HCM Unsignalized Intersection Capacity Analysis

	۶	→	*	•	←	•	•	†	<i>></i>	/	↓	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Sign Control		Stop			Stop			Stop			Stop	
Volume (vph)	6	50	11	9	48	64	7	60	7	53	56	10
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	7	54	12	10	52	70	8	65	8	58	61	11
Direction, Lane #	EB1	WB 1	NB 1	SB 1								
Volume Total (vph)	73	132	80	129								
Volume Left (vph)	7	10	8	58								
Volume Right (vph)	12	70	8	11								
Hadj (s)	-0.05	-0.27	0.00	0.07								
Departure Headway (s)	4.5	4.2	4.5	4.5								
Degree Utilization, x	0.09	0.15	0.10	0.16								
Capacity (veh/h)	756	802	752	749								
Control Delay (s)	7.9	8.0	8.0	8.4								
Approach Delay (s)	7.9	8.0	8.0	8.4								
Approach LOS	Α	Α	Α	Α								
Intersection Summary												
Delay			8.1									
HCM Level of Service			Α									
Intersection Capacity Utilizat	ion		28.5%	IC	:U Level o	f Service			Α			
Analysis Period (min)			15									

AM Existing 9: La Brucherie Rd & Wahl Rd

HCM Unsignalized Intersection Capacity Analysis

		-	•	•	-	•	•	T	-	>	Į.	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			44			4	
Volume (veh/h)	2	8	12	3	13	1	7	14	0	6	131	6
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	2	9	13	3	14	1	8	15	0	7	142	7
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	15			22			118	41	15	48	47	15
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	15			22			118	41	15	48	47	15
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			100			99	98	100	99	83	99
cM capacity (veh/h)	1603			1594			739	848	1064	937	841	1065
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	24	18	23	155								
Volume Left	2	3	8	7								
Volume Right	13	1	0	7								
cSH	1603	1594	808	853								
Volume to Capacity	0.00	0.00	0.03	0.18								
Queue Length 95th (ft)	0	0	2	17								
Control Delay (s)	0.7	1.3	9.6	10.2								
Lane LOS	Α	Α	Α	В								
Approach Delay (s)	0.7	1.3	9.6	10.2								
Approach LOS			Α	В								
Intersection Summary												
Average Delay			8.3									
Intersection Capacity Utilizat	tion		17.7%	IC	U Level o	f Service			Α			
Analysis Period (min)			15									

LOS Engineering, Inc. Synchro AM Existing

	•	→	•	•	←	•	4	†	-	-	Ţ	
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	
Lane Configurations		4			43-			43-			4	_
Volume (veh/h)	1	1	1	1	1	1	15	17	1	3	113	
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	1	1	1	1	1	1	16	18	1	3	123	
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (ft)											436	
pX, platoon unblocked												
vC, conflicting volume	189	188	129	189	194	19	136			20		
vC1, stage 1 conf vol	107	100	127	107	.,,	.,	100					
vC2, stage 2 conf vol												
vCu, unblocked vol	189	188	129	189	194	19	136			20		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)		0.0	0.2		0.0	O.L						
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	100	100	100	99			100		
cM capacity (veh/h)	762	697	920	761	692	1059	1448			1597		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1	072	1007	1110			1077		
Volume Total	3	3	36	139								
Volume Left	1	1	16	3								
Volume Right	1	1	1	13								
cSH	782	810	1448	1597								
Volume to Capacity	0.00	0.00	0.01	0.00								
Queue Length 95th (ft)	0.00	0.00	1	0.00								
Control Delay (s)	9.6	9.5	3.5	0.2								
Lane LOS	7.0 A	Α.	3.5 A	Α.2								
Approach Delay (s)	9.6	9.5	3.5	0.2								
Approach LOS	7.0 A	7.5 A	3.3	0.2								
Intersection Summary												
Average Delay			1.2									
Intersection Capacity Utiliza	ation		20.0%	IC	U Level	of Service			Α			
Analysis Period (min)			15									